

APPENDIX A

City of Fairfax Scoping Agreement



SCOPE OF WORK MEETING FORM

Information on the Project

Traffic Impact Analysis Base Assumptions

PAUL VI REDEVELOPMENT
CITY OF FAIRFAX, VIRGINIA
December 29, 2016

Contact Information

Consultant Name: Tele: E-mail:	Christopher Turnbull - Wells + Associates, Inc. 703-917-6620 cturnbull@wellsandassociates.com
Developer/Owner Name: Tele: E-mail:	Enrico C. Cecchi – Patrick Rhodes IDI Group Companies 703-558-7348 ececchi@idigroup.com

Project Information

Project Name:	Paul VI Redevelopment	Locality/County:	City of Fairfax
Project Location: (Attach regional and site specific location map)	The project is generally located south of Fairfax Boulevard, between Main Street and Chain Bridge Road. See Attachment 1 for the site location.		
Submission Type	Comp Plan <input type="checkbox"/>	Rezoning <input checked="" type="checkbox"/> (SUP)	Site Plan <input type="checkbox"/> Subd Plat <input type="checkbox"/>
Project Description: (Including details on the land use, acreage, phasing, access location, etc. Attach additional sheet if necessary)	The Applicant is proposing to redevelop the property with 575 residential units to include active adult, condominiums, townhomes, and multifamily units. Twenty thousand (20,000) square feet of commercial and community space is also proposed. The conceptual development plan is provided as Attachment 2 .		
Proposed Use(s): (Check all that apply; attach additional pages as necessary)	Residential <input type="checkbox"/>	Commercial <input type="checkbox"/>	Mixed Use <input checked="" type="checkbox"/> Other <input type="checkbox"/>
(See Attachment – 3)	Residential Uses(s) Number of Units: <u>575</u> ITE LU Code(s): <u>220, 230, & 251</u> Commercial Use(s) ITE LU Code(s): 820 Square Ft or Other Variable: <u>10,000</u> _____	Other Use(s) ITE LU Code(s): _____ _____ _____ Independent Variable(s): _____ _____ _____	

Total Peak Hour Trip Projection:	Less than 100 <input type="checkbox"/>	100 – 499 <input checked="" type="checkbox"/>	500 – 999 <input type="checkbox"/>	1,000 or more <input type="checkbox"/>
Traffic Impact Analysis Assumptions				
Study Period	Existing Year: 2017	Build-out Year: 2027	Design Year: n/a	
Study Area Boundaries	North: Fairfax Boulevard (US Route 50)	South: Cedar Avenue		
	East: McLean Avenue	West: Oak Street		
External Factors That Could Affect Project (Planned road improvements, other nearby developments)	<ul style="list-style-type: none"> • Novus Fairfax Gateway redevelopment • Mount Vineyard (Oak Knolls) • Kamp Washington intersection improvements • Fairfax Boulevard at Chain Bridge Road intersection improvements 			
Consistency With Comprehensive Plan (Land use, transportation plan)	A change in land use from Institutional was not anticipated with the latest Comp. Plan Update. The current C-2 Commercial/R-2 zoning would permit the proposed land use via a rezoning. The roadway network is consistent with the intent of the City Transportation Plan.			
Available Traffic Data (Historical, forecasts)	<p>VDOT historical traffic count data indicates:</p> <p><u>2015 VDOT Average Annual Daily Traffic (AADT):</u> Fairfax Boulevard (US Route 50): 35,000 vpd (Main Street to Chain Bridge Road)</p> <p><u>2014 VDOT Average Annual Daily Traffic (AADT):</u> Fairfax Boulevard (US Route 50): 36,000 vpd (Main Street to Chain Bridge Road)</p> <p><u>2013 VDOT Average Annual Daily Traffic (AADT):</u> Fairfax Boulevard (US Route 50): 36,000 vpd (Main Street to Chain Bridge Road)</p> <p>Future Forecasts will be developed</p>			
Trip Distribution (Pending data from existing traffic counts) (See Attachment 4)	From the West: 35%		From the Northeast: 50%	
	From the North: 0%		From the Southeast: 15%	
Annual Vehicle Trip Growth Rate:	1% or per VDOT AADT counts	Peak Period for Study (check all that apply)	<input checked="" type="checkbox"/> AM <input checked="" type="checkbox"/> PM <input type="checkbox"/> SAT	
		Peak Hour of the Generator	N/A	
Study Intersections and/or Road Segments (See Attachment 4)	1. Fairfax Boulevard/Main Street		6. Walnut Street/Cedar Avenue	
	2. Fairfax Boulevard/Fairchester Drive, Walnut Street		7. Oak Street/Cedar Avenue	
	3. Fairfax Boulevard/Meredith Drive, Oak Street		8. McLean Avenue/Cedar Avenue	
	4. Fairfax Boulevard/Shopping Center Entrance – Future Site Entrance		9. Chain Bridge Road/Cedar Avenue	
	5. Fairfax Boulevard /McLean Avenue/ Warwick Avenue			
Trip Adjustment Factors	Internal allowance: <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No Reduction: _____% trips		Pass-by allowance: <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No Reduction: _____% trips	

Software Methodology	<input checked="" type="checkbox"/> Synchro <input type="checkbox"/> HCS (v.2000/+) <input type="checkbox"/> aaSIDRA <input type="checkbox"/> CORSIM <input type="checkbox"/> Other _____
Traffic Signal Proposed or Affected (Analysis software to be used, progression speed, cycle length)	Project anticipates adding 4 th leg to Fairfax Drive/Shopping Center signal. Capacity analyses will be based on Synchro (version 9.1).
Improvement(s) Assumed or to be Considered	A full-access entrance is proposed as noted above along with a frontage road or slow lane as envisioned in the Master Plan for Fairfax Boulevard.
Background Traffic Studies Considered	<ul style="list-style-type: none"> • Novus Fairfax Gateway Traffic Impact Analysis • Mount Vineyard (Oak Knolls) Traffic Impact Study
Plan Submission	<input type="checkbox"/> Master Development Plan (MDP) <input checked="" type="checkbox"/> Generalized Development Plan (GDP) <input type="checkbox"/> Preliminary/Sketch Plan <input type="checkbox"/> Other Plan type (Final Site, Subd. Plan)
Additional Issues to be Addressed	<input type="checkbox"/> Queuing analysis <input type="checkbox"/> Actuation/Coordination <input type="checkbox"/> Weaving analysis <input type="checkbox"/> Merge analysis <input type="checkbox"/> Bike/Ped Accommodations <input type="checkbox"/> Intersection(s) <input type="checkbox"/> TDM Measures <input type="checkbox"/> Other _____

NOTES on ASSUMPTIONS:

1. Synchro 9.1 will be used to conduct capacity analysis with peak hour factors measured in the field for existing conditions ($0.85 < PHF < 0.92$). Under background and total future conditions a PHF of 0.92 will be used for all movements.
2. Existing Synchro (signal timing) files to be provided by the city.

SCOPE OF WORK MEETING

ADDITIONS TO THE REQUIRED ELEMENTS, CHANGES TO THE METHODOLOGY OR STANDARD ASSUMPTIONS, AND SIGNATURE PAGE

Any additions to the Required Elements or changes to the Methodology or Standard Assumptions due to special circumstances that are approved by the City of Fairfax:

AGREED:  DATE: 12/29/2016
Consultant

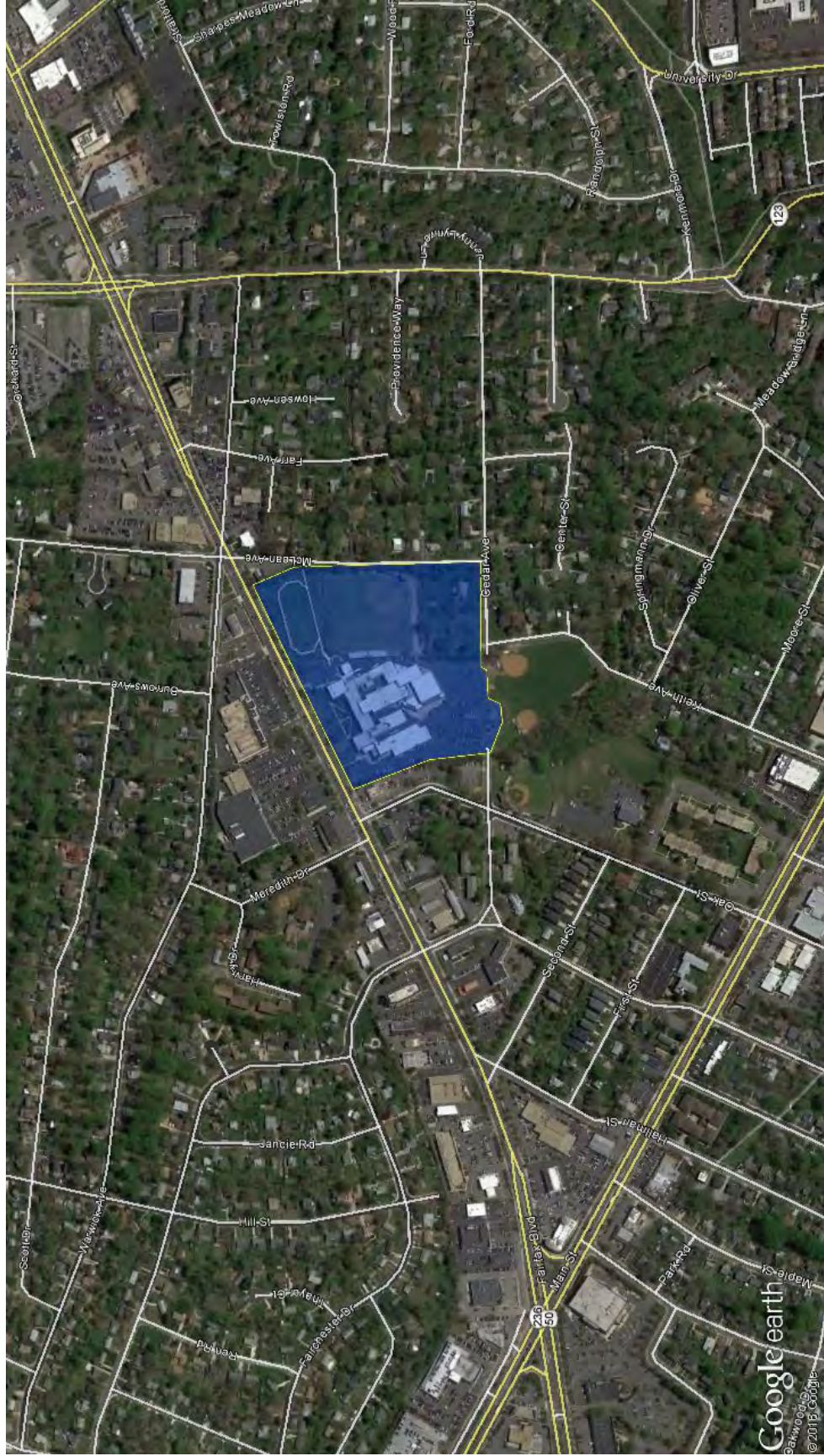
PRINT NAME: Christopher Turnbull
Consultant

SIGNED:  DATE: January 12, 2017

PRINT NAME: Wendy Sanford

- Attachments:
- Attachment 1 - Site Location
 - Attachment 2 – Conceptual Development Plans – Paul VI Scheme B
 - Attachment 3 – Trip Generation
 - Attachment 4 – Study Intersections and Site Trip Distribution Percentages

Attachment 1 – Site Location



Attachment 2 - Conceptual Development Plan



SCHEME B

Paul VI Development | Fairfax, Virginia

THE IDI GROUP COMPANIES

SLANA | SOME, LLC

W3 REAL ASSOCIATES

THORNTON TOMASETTI

christopher greenleaf

streetsense

July 26, 2016

Attachment 3

Paul VI Redevelopment - Scheme B Site Trip Generation Comparison

Development	ITE Land Use Code ¹	Amount	Units	AM Peak Hour			PM School Peak (2:45-3:45)			PM Peak Hour		Average Daily Trips	
				In	Out	Total	In	Out	Total	In	Out		Total
Existing													
Private High School ^{2,3}			Actual Trips	676	311	987	157	397	554	37	74	111	3,270
Current Zoning⁴													
Retail	820	132,500	SF	79	48	127	318	359	677	347	376	723	8,154
Proposed⁴													
Apartments	220	214	DU	22	87	109	58	45	102	88	47	135	1,420
Condominiums/Townhomes	230	327	DU	23	110	133	37	32	69	107	52	159	1,803
Senior Housing	251	34	DU	13	23	36	37	35	73	12	8	20	181
Subtotal Residential		575	DU	58	220	278	132	111	243	207	107	314	3,404
Local Serving Retail	820	10,000	SF	6	4	10	59	67	126	61	67	128	1,520
Comparison													
		Total Proposed Trips		64	224	288	191	178	369	268	174	442	4,924
		Actual vs. Current Zoning		-597	-263	-860	161	-38	123	310	302	612	4,884
		Actual vs. Proposed		-612	-87	-699	34	-219	-185	231	100	331	1,654

Notes:

- 1 Institute of Transportation Engineer's (ITE), Trip Generation Manual, 9th Edition
- 2 Based on traffic counts completed on February 3, 2016.
- 3 Actual ADT estimated based on ITE ADT and PM school peak ratio.
- 4 PM School Peak trips based on residential and retail diurnal rates compiled from ITE and Wells + Associates files.

Attachment 4 - Study Intersections and Site Trip Distribution Percentages



APPENDIX B

Existing Traffic Volumes

Wells + Associates, Inc.

McLean, Virginia

Turning Movement Count - All Vehicles

Project: Paul VI Development, W+A Job No: 6709, Intersection: Fairfax Blvd. & Meredith Dr., Location: Fairfax County, VA, Date: 2/3/2016, Day: Wednesday, Weather: rain, Counted By: Dzemo & Whitney, Input By: agan, Southbound Road: Meredith Drive, Northbound Road: Oak Street, Westbound Road: Fairfax Boulevard - 50, Eastbound Road: Fairfax Boulevard - 50

Table with 16 columns: Time Period, Southbound Meredith Drive (Right, Thru, Left, Total, PHF), Westbound Fairfax Boulevard - 50 (Right, Thru, Left, Total, PHF), Northbound Oak Street (Right, Thru, Left, Total, PHF), Eastbound Fairfax Boulevard - 50 (Right, Thru, Left, Total, PHF), North & East South & West, Total. Rows include AM 15 Minute Volumes from 6:00 AM to 8:45 AM and a Total row.

Table with 16 columns: Time Period, Southbound Meredith Drive (Right, Thru, Left, Total, PHF), Westbound Fairfax Boulevard - 50 (Right, Thru, Left, Total, PHF), Northbound Oak Street (Right, Thru, Left, Total, PHF), Eastbound Fairfax Boulevard - 50 (Right, Thru, Left, Total, PHF), North & East South & West, Total. Rows include AM One Hour Volumes from 6:00 AM to 8:00 AM and a Total row.

Table with 16 columns: Time Period, Southbound Meredith Drive (Right, Thru, Left, Total, PHF), Westbound Fairfax Boulevard - 50 (Right, Thru, Left, Total, PHF), Northbound Oak Street (Right, Thru, Left, Total, PHF), Eastbound Fairfax Boulevard - 50 (Right, Thru, Left, Total, PHF), North & East South & West, Total. Rows include PM 15 Minute Volumes from 2:00 PM to 6:00 PM and a Total row.

Table with 16 columns: Time Period, Southbound Meredith Drive (Right, Thru, Left, Total, PHF), Westbound Fairfax Boulevard - 50 (Right, Thru, Left, Total, PHF), Northbound Oak Street (Right, Thru, Left, Total, PHF), Eastbound Fairfax Boulevard - 50 (Right, Thru, Left, Total, PHF), North & East South & West, Total. Rows include PM One Hour Volumes from 2:00 PM to 6:00 PM and a Total row.

Wells + Associates, Inc.

McLean, Virginia

Turning Movement Count - All Vehicles

Project: Paul VI Development, W+A Job No: 6709, Intersection: Fairfax Blvd. & McLean Ave., Location: Fairfax County, VA, Date: 2/3/2016, Day: Wednesday, Weather: rain, Counted By: Tyler, Austin & Salih, Input By: agan, Southbound Road: McLean Avenue, Northbound Road: McLean Avenue, Westbound Road: Fairfax Boulevard - 50, Eastbound Road: Fairfax Boulevard - 50

Table with 18 columns: Time Period, Southbound McLean Avenue (Right, Thru, Left, Total, PHF), Westbound Fairfax Boulevard - 50 (Right, Thru, Left, Total, PHF), Northbound McLean Avenue (Right, Thru, Left, Total, PHF), Eastbound Fairfax Boulevard - 50 (Right, Thru, Left, Total, PHF), North & South, East & West, Total. Rows include 15 AM Minute Volumes and a Total row.

Table with 18 columns: Time Period, Southbound McLean Avenue (Right, Thru, Left, Total, PHF), Westbound Fairfax Boulevard - 50 (Right, Thru, Left, Total, PHF), Northbound McLean Avenue (Right, Thru, Left, Total, PHF), Eastbound Fairfax Boulevard - 50 (Right, Thru, Left, Total, PHF), North & South, East & West, Total. Rows include 1 AM One Hour Volumes and a Total row.

Table with 18 columns: Time Period, Southbound McLean Avenue (Right, Thru, Left, Total, PHF), Westbound Fairfax Boulevard - 50 (Right, Thru, Left, Total, PHF), Northbound McLean Avenue (Right, Thru, Left, Total, PHF), Eastbound Fairfax Boulevard - 50 (Right, Thru, Left, Total, PHF), North & South, East & West, Total. Rows include 1 PM 15 Minute Volumes and a Total row.

Table with 18 columns: Time Period, Southbound McLean Avenue (Right, Thru, Left, Total, PHF), Westbound Fairfax Boulevard - 50 (Right, Thru, Left, Total, PHF), Northbound McLean Avenue (Right, Thru, Left, Total, PHF), Eastbound Fairfax Boulevard - 50 (Right, Thru, Left, Total, PHF), North & South, East & West, Total. Rows include 1 PM One Hour Volumes and a Total row.

Wells + Associates, Inc.

McLean, Virginia

Turning Movement Count - All Vehicles

PROJECT: Paul VI Development	DATE: 2/3/2016	SOUTHBOUND ROAD: McLean Avenue
W+A JOB NO: 6709	DAY: Wednesday	NORTHBOUND ROAD: McLean Avenue
INTERSECTION: McLean Ave. & Warwick Ave.	WEATHER: rain	WESTBOUND ROAD: Warwick Avenue
LOCATION: Fairfax County,VA	COUNTED BY: Salih & Laura	EASTBOUND ROAD: Warwick Avenue
INPUTED BY: agan		

Time Period	Southbound McLean Avenue					Westbound Warwick Avenue					Northbound McLean Avenue					Eastbound Warwick Avenue					North South	East & West	Total
	Right	Thru	Left	Total	PHF	Right	Thru	Left	Total	PHF	Right	Thru	Left	Total	PHF	Right	Thru	Left	Total	PHF			
AM 15 Minute Volumes																							
6:00 AM - 6:15 AM	1	0	0	1		0	0	0	0		0	0	0	0		0	0	0	0		1	0	1
6:15 AM - 6:30 AM	0	0	0	0		1	0	0	1		0	0	0	0		0	0	0	0		0	1	1
6:30 AM - 6:45 AM	0	0	0	0		0	0	0	0		0	0	0	0		0	0	0	0		0	0	0
6:45 AM - 7:00 AM	0	0	0	0		0	0	1	1		0	0	0	0		0	0	0	0		0	1	1
7:00 AM - 7:15 AM	0	0	1	1		1	0	0	1		0	0	0	0		0	0	1	1		1	2	3
7:15 AM - 7:30 AM	1	0	1	2		0	0	0	0		1	0	0	1		0	0	2	2		3	2	5
7:30 AM - 7:45 AM	0	0	1	1		2	0	0	2		0	0	0	0		0	0	1	1		1	3	4
7:45 AM - 8:00 AM	1	0	0	1		1	0	0	1		0	0	0	0		0	0	0	0		1	1	2
8:00 AM - 8:15 AM	2	0	2	4		2	0	0	2		0	0	2	2		0	0	1	1		6	3	9
8:15 AM - 8:30 AM	1	0	0	1		2	0	0	2		0	0	0	0		0	0	1	1		1	3	4
8:30 AM - 8:45 AM	0	0	0	0		4	0	1	5		0	0	1	1		0	0	0	0		1	5	6
8:45 AM - 9:00 AM	0	0	1	1		2	0	2	4		0	0	0	0		1	0	0	1		1	5	6
Total	6	0	6	12		15	0	4	19		1	0	3	4		1	0	6	7		16	26	42
AM One Hour Volumes																							
6:00 AM - 7:00 AM	1	0	0	1	0.25	1	0	1	2	0.50	0	0	0	0	0.00	0	0	0	0	0.00	1	2	3
6:15 AM - 7:15 AM	0	0	1	1	0.25	2	0	1	3	0.75	0	0	0	0	0.00	0	0	1	1	0.25	1	4	5
6:30 AM - 7:30 AM	1	0	2	3	0.38	1	0	1	2	0.50	1	0	0	1	0.25	0	0	3	3	0.38	4	5	9
6:45 AM - 7:45 AM	1	0	3	4	0.50	3	0	1	4	0.50	1	0	0	1	0.25	0	0	4	4	0.50	5	8	13
7:00 AM - 8:00 AM	2	0	3	5	0.63	4	0	1	4	0.50	1	0	0	1	0.25	0	0	4	4	0.50	6	8	14
7:15 AM - 8:15 AM	4	0	4	8	0.50	5	0	1	5	0.63	1	0	2	3	0.38	0	0	4	4	0.50	11	9	20
7:30 AM - 8:30 AM	4	0	3	7	0.44	7	0	0	7	0.88	0	0	2	2	0.25	0	0	3	3	0.75	9	10	19
7:45 AM - 8:45 AM	4	0	2	6	0.38	9	0	1	10	0.50	0	0	3	3	0.38	0	0	2	2	0.50	9	12	21
8:00 AM - 9:00 AM	3	0	3	6	0.38	10	0	3	13	0.65	0	0	3	3	0.38	1	0	2	3	0.75	9	16	25
PM 15 Minute Volumes																							
2:00 PM - 2:15 PM	1	0	1	2		2	0	0	2		0	0	0	0		1	0	0	1		2	3	5
2:15 PM - 2:30 PM	0	0	0	0		1	0	0	1		0	0	0	0		0	0	2	2		0	3	3
2:30 PM - 2:45 PM	0	0	1	1		1	0	0	1		0	0	1	1		0	0	0	0		2	1	3
2:45 PM - 3:00 PM	0	0	0	0		2	0	1	3		0	0	2	2		0	0	1	1		2	4	6
3:00 PM - 3:15 PM	0	0	0	0		3	0	0	3		0	0	0	0		3	0	0	3		0	6	6
3:15 PM - 3:30 PM	1	0	3	4		3	0	0	3		0	0	3	3		0	0	2	2		7	5	12
3:30 PM - 3:45 PM	0	0	4	4		2	0	0	2		0	0	3	3		0	0	1	1		7	3	10
3:45 PM - 4:00 PM	1	0	0	1		1	0	0	1		0	0	0	0		0	0	0	0		1	1	2
4:00 PM - 4:15 PM	0	0	0	0		2	0	0	2		2	0	3	5		1	0	0	1		5	3	8
4:15 PM - 4:30 PM	0	0	0	0		3	0	0	3		0	0	5	5		1	0	0	1		5	4	9
4:30 PM - 4:45 PM	0	0	0	0		3	0	0	3		0	0	2	2		1	0	4	5		2	8	10
4:45 PM - 5:00 PM	1	0	0	1		3	0	0	3		0	0	6	6		0	0	0	0		7	3	10
5:00 PM - 5:15 PM	1	0	1	2		3	0	1	4		0	0	3	3		2	0	1	3		5	7	12
5:15 PM - 5:30 PM	0	0	1	1		5	0	0	5		0	0	4	4		0	0	1	1		5	6	11
5:30 PM - 5:45 PM	0	0	1	1		4	0	0	4		0	0	1	1		1	0	2	3		2	7	9
5:45 PM - 6:00 PM	0	0	0	0		2	0	0	2		0	0	5	5		0	0	2	2		5	4	9
6:00 PM - 6:15 PM	0	0	0	0		4	0	0	4		0	0	0	0		0	0	1	1		0	5	5
6:15 PM - 6:30 PM	0	0	1	1		1	0	0	1		0	0	1	1		0	0	1	1		2	2	4
6:30 PM - 6:45 PM	0	0	1	1		4	0	0	4		0	0	3	3		0	0	1	1		4	5	9
6:45 PM - 7:00 PM	0	0	0	0		3	0	0	3		0	0	1	1		0	0	0	0		1	3	4
Total	2	0	5	7		37	0	1	38		2	0	34	36		6	0	13	19		43	57	100
PM One Hour Volumes																							
2:00 PM - 3:00 PM	1	0	2	3	0.38	6	0	1	7	0.58	0	0	3	3	0.38	1	0	3	4	0.50	6	11	17
2:15 PM - 3:15 PM	0	0	1	1	0.25	7	0	1	8	0.67	0	0	3	3	0.38	3	0	3	6	0.50	4	14	18
2:30 PM - 3:30 PM	1	0	4	5	0.31	9	0	1	10	0.83	0	0	6	6	0.50	3	0	3	6	0.50	11	16	27
2:45 PM - 3:45 PM	1	0	7	8	0.50	10	0	1	11	0.92	0	0	8	8	0.67	3	0	4	7	0.58	16	18	34
3:00 PM - 4:00 PM	2	0	7	9	0.56	9	0	0	9	0.75	0	0	6	6	0.50	3	0	3	6	0.50	15	15	30
3:15 PM - 4:15 PM	2	0	7	9	0.56	8	0	0	8	0.67	2	0	9	11	0.55	1	0	3	4	0.50	20	12	32
3:30 PM - 4:30 PM	1	0	4	5	0.31	8	0	0	8	0.67	2	0	11	13	0.65	2	0	1	3	0.75	18	11	29
3:45 PM - 4:45 PM	1	0	0	1	0.25	9	0	0	9	0.75	2	0	10	12	0.60	3	0	4	7	0.35	13	16	29
4:00 PM - 5:00 PM	1	0	0	1	0.25	11	0	0	11	0.92	2	0	16	18	0.75	3	0	4	7	0.35	19	18	37
4:15 PM - 5:15 PM	2	0	1	3	0.38	12	0	1	13	0.81	0	0	16	16	0.67	4	0	5	9	0.45	19	22	41
4:30 PM - 5:30 PM	2	0	2	4	0.50	14	0	1	15	0.75	0	0	15	15	0.63	3	0	6	9	0.45	19	24	43
4:45 PM - 5:45 PM	2	0	3	5	0.63	15	0	1	16	0.80	0	0	14	14	0.58	3	0	4	7	0.58	19	23	42
5:00 PM - 6:00 PM	1	0	3	4	0.50	14	0	1	15	0.75	0	0	13	13	0.65	3	0	6	9	0.75	17	24	41
5:15 PM - 6:15 PM	0	0	2	2	0.50	15	0	0	15	0.75	0	0	10	10	0.50	1	0	6	7	0.58	12	22	34
5:30 PM - 6:30 PM	0	0	2	2	0.50	11	0	0	11	0.69	0	0	7	7	0.35	1	0	6	7	0.58	9	18	27
5:45 PM - 6:45 PM	0	0	2	2	0.50	11	0	0	11	0.69	0	0	9	9	0.45	0	0	5	5	0.63	11	16	27
6:00 PM - 7:00 PM	0	0	2	2	0.50	12	0	0	12	0.75	0	0	5	5	0.42	0	0	3	3	0.75	7	15	22

Wells + Associates, Inc.

McLean, Virginia

Turning Movement Count - All Vehicles

PROJECT: Paul VI Development	DATE: 2/3/2016	SOUTHBOUND ROAD: Warwick Avenue
W+A JOB NO: 6709	DAY: Wednesday	NORTHBOUND ROAD: Warwick Avenue
INTERSECTION: Fairfax Blvd. & Warwick Ave.	WEATHER: rain	WESTBOUND ROAD: Fairfax Boulevard - 50
LOCATION: Fairfax County,VA	COUNTED BY: Tyler,Sali,Austin & Laura	EASTBOUND ROAD: Fairfax Boulevard - 50
	INPUTED BY: agan	

Time Period	Southbound Warwick Avenue					Westbound Fairfax Boulevard - 50					Northbound Warwick Avenue					Eastbound Fairfax Boulevard - 50					North South	East & West	Total
	Right	Thru	Left	Total	PHF	Right	Thru	Left	Total	PHF	Right	Thru	Left	Total	PHF	Right	Thru	Left	Total	PHF			
AM 15 Minute Volumes																							
6:00 AM - 6:15 AM	0	3	2	5		3	0	0	3		0	1	1	2		2	0	1	3		7	6	13
6:15 AM - 6:30 AM	0	3	3	6		1	0	0	1		0	0	2	2		2	0	0	2		8	3	11
6:30 AM - 6:45 AM	0	0	11	11		5	0	0	5		1	1	1	3		2	0	0	2		14	7	21
6:45 AM - 7:00 AM	0	2	15	17		4	0	0	4		0	0	2	2		0	0	0	0		19	4	23
7:00 AM - 7:15 AM	0	6	13	19		19	0	0	19		3	0	2	5		7	0	0	7		24	26	50
7:15 AM - 7:30 AM	0	5	16	21		18	0	1	19		2	0	1	3		10	0	0	10		24	29	53
7:30 AM - 7:45 AM	1	8	30	39		11	0	1	12		1	1	2	4		12	0	0	12		43	24	67
7:45 AM - 8:00 AM	0	7	36	43		19	0	0	19		1	2	4	7		4	0	0	4		50	23	73
8:00 AM - 8:15 AM	1	8	16	25		17	0	0	17		1	2	5	8		4	0	0	4		33	21	54
8:15 AM - 8:30 AM	0	3	16	19		14	0	0	14		0	1	3	4		6	0	0	6		23	20	43
8:30 AM - 8:45 AM	0	2	15	17		6	0	1	7		1	2	2	5		2	0	0	2		22	9	31
8:45 AM - 9:00 AM	0	5	14	19		26	0	2	28		0	1	2	3		10	0	0	10		22	38	60
Total	2	52	187	241		143	0	5	148		10	11	27	48		61	0	1	62		289	210	499
AM One Hour Volumes																							
6:00 AM - 7:00 AM	0	8	31	39	0.57	13	0	0	13	0.65	1	2	6	9	0.75	6	0	1	7	0.58	48	20	68
6:15 AM - 7:15 AM	0	11	42	53	0.70	29	0	0	29	0.38	4	1	7	12	0.60	11	0	0	11	0.39	65	40	105
6:30 AM - 7:30 AM	0	13	55	68	0.81	46	0	1	47	0.62	6	1	6	13	0.65	19	0	0	19	0.48	81	66	147
6:45 AM - 7:45 AM	1	21	74	96	0.62	52	0	2	54	0.71	6	1	7	14	0.70	29	0	0	29	0.60	110	83	193
7:00 AM - 8:00 AM	1	26	95	122	0.71	67	0	2	69	0.91	7	3	9	19	0.68	33	0	0	33	0.69	141	102	243
7:15 AM - 8:15 AM	2	28	98	128	0.74	65	0	2	67	0.88	5	5	12	22	0.69	30	0	0	30	0.63	150	97	247
7:30 AM - 8:30 AM	2	26	98	126	0.73	61	0	1	62	0.82	3	6	14	23	0.72	26	0	0	26	0.54	149	88	237
7:45 AM - 8:45 AM	1	20	83	104	0.60	56	0	1	57	0.75	3	7	14	24	0.75	16	0	0	16	0.67	128	73	201
8:00 AM - 9:00 AM	1	18	61	80	0.80	63	0	3	66	0.59	2	6	12	20	0.63	22	0	0	22	0.55	100	88	188
PM 15 Minute Volumes																							
2:00 PM - 2:15 PM	0	7	12	19		15	0	1	16		0	2	1	3		7	0	1	8		22	24	46
2:15 PM - 2:30 PM	0	3	16	19		33	0	2	35		1	4	0	5		14	0	3	17		24	52	76
2:30 PM - 2:45 PM	0	2	19	21		7	0	1	8		3	5	2	10		13	0	2	15		31	23	54
2:45 PM - 3:00 PM	0	5	16	21		10	0	1	11		7	7	1	15		12	0	0	12		36	23	59
3:00 PM - 3:15 PM	0	3	23	26		20	0	1	21		1	9	0	10		16	0	3	19		36	40	76
3:15 PM - 3:30 PM	0	6	17	23		18	0	0	18		4	4	2	10		11	0	1	12		33	30	63
3:30 PM - 3:45 PM	0	5	17	22		20	0	1	21		3	6	1	10		12	0	1	13		32	34	66
3:45 PM - 4:00 PM	0	6	14	20		18	0	2	20		3	9	0	12		16	0	0	16		32	36	68
4:00 PM - 4:15 PM	0	4	10	14		20	0	1	21		1	8	1	10		10	0	0	10		24	31	55
4:15 PM - 4:30 PM	0	10	15	25		23	0	0	23		1	10	0	11		11	0	0	11		36	34	70
4:30 PM - 4:45 PM	2	7	14	23		22	0	3	25		2	7	2	11		13	0	11	24		34	49	83
4:45 PM - 5:00 PM	0	8	15	23		29	0	2	31		0	7	2	9		5	0	1	6		32	37	69
5:00 PM - 5:15 PM	0	6	17	23		19	0	0	19		2	6	0	8		4	0	0	4		31	23	54
5:15 PM - 5:30 PM	0	5	11	16		30	0	0	30		0	7	2	9		5	0	1	6		25	36	61
5:30 PM - 5:45 PM	0	6	21	27		24	0	0	24		1	9	0	10		13	0	0	13		37	37	74
5:45 PM - 6:00 PM	0	16	18	34		26	0	0	26		0	6	0	6		12	0	2	14		40	40	80
6:00 PM - 6:15 PM	0	6	12	18		27	0	0	27		0	9	1	10		8	0	1	9		28	36	64
6:15 PM - 6:30 PM	0	3	12	15		29	0	0	29		0	3	1	4		10	0	2	12		19	41	60
6:30 PM - 6:45 PM	0	10	13	23		22	0	2	24		1	5	0	6		6	0	6	12		29	36	65
6:45 PM - 7:00 PM	0	5	9	14		13	0	0	13		0	9	1	10		5	0	0	5		24	18	42
Total	2	86	167	255		284	0	8	292		8	86	10	104		102	0	24	126		359	418	777
PM One Hour Volumes																							
2:00 PM - 3:00 PM	0	17	63	80	0.95	65	0	5	70	0.50	11	18	4	33	0.55	46	0	6	52	0.76	113	122	235
2:15 PM - 3:15 PM	0	13	74	87	0.84	70	0	5	75	0.54	12	25	3	40	0.67	55	0	8	63	0.83	127	138	265
2:30 PM - 3:30 PM	0	16	75	91	0.88	55	0	3	58	0.69	15	25	5	45	0.75	52	0	6	58	0.76	136	116	252
2:45 PM - 3:45 PM	0	19	73	92	0.88	68	0	3	71	0.85	15	26	4	45	0.75	51	0	5	56	0.74	137	127	264
3:00 PM - 4:00 PM	0	20	71	91	0.88	76	0	4	80	0.95	11	28	3	42	0.88	55	0	5	60	0.79	133	140	273
3:15 PM - 4:15 PM	0	21	58	79	0.86	76	0	4	80	0.95	11	27	4	42	0.88	49	0	2	51	0.80	121	131	252
3:30 PM - 4:30 PM	0	25	56	81	0.81	81	0	4	85	0.92	8	33	2	43	0.90	49	0	1	50	0.78	124	135	259
3:45 PM - 4:45 PM	2	27	53	82	0.82	83	0	6	89	0.89	7	34	3	44	0.92	50	0	11	61	0.64	126	150	276
4:00 PM - 5:00 PM	2	29	54	85	0.85	94	0	6	100	0.81	4	32	5	41	0.93	39	0	12	51	0.53	126	151	277
4:15 PM - 5:15 PM	2	31	61	94	0.94	93	0	5	98	0.79	5	30	4	39	0.89	33	0	12	45	0.47	133	143	276
4:30 PM - 5:30 PM	2	26	57	85	0.92	100	0	5	105	0.85	4	27	6	37	0.84	27	0	13	40	0.42	122	145	267
4:45 PM - 5:45 PM	0	25	64	89	0.82	102	0	2	104	0.84	3	29	4	36	0.90	27	0	2	29	0.56	125	133	258
5:00 PM - 6:00 PM	0	33	67	100	0.74	99	0	0	99	0.83	3	28	2	33	0.83	34	0	3	37	0.66	133	136	269
5:15 PM - 6:15 PM	0	33	62	95	0.70	107	0	0	107	0.89	1	31	3	35	0.88	38	0	4	42	0.75	130	149	279
5:30 PM - 6:30 PM	0	31	63	94	0.69	106	0	0	106	0.91	1	27	2	30	0.75	43	0	5	48	0.86	124	154	278
5:45 PM - 6:45 PM	0	35	55	90	0.66	104	0	2	106	0.91	1	23	2	26	0.65	36	0	11	47	0.84	116	153	269
6:00 PM - 7:00 PM	0	24	46	70	0.76	91	0	2	93	0.80	1	26	3	30	0.75	29	0	9	38	0.79	100	131	231

Wells + Associates, Inc.

McLean, Virginia

Turning Movement Count - All Vehicles

PROJECT: Paul VI Development	DATE: 2/3/2016	SOUTHBOUND ROAD: Walnut Street
W+A JOB NO: 6709	DAY: Wednesday	NORTHBOUND ROAD: Walnut Street
INTERSECTION: Cedar Ave. & Walnut St.	WEATHER: rain	WESTBOUND ROAD: Cedar Avenue
LOCATION: Fairfax County,VA	COUNTED BY: Vanessa	EASTBOUND ROAD: Driveway
INPUTED BY: agan		

Time Period	Southbound Walnut Street					Westbound Cedar Avenue					Northbound Walnut Street					Eastbound Driveway					North & South	East & West	Total
	Right	Thru	Left	Total	PHF	Right	Thru	Left	Total	PHF	Right	Thru	Left	Total	PHF	Right	Thru	Left	Total	PHF			
AM 15 Minute Volumes																							
6:00 AM - 6:15 AM	0	1	2	3		0	0	0	0		0	5	0	5		0	0	1	1		8	1	9
6:15 AM - 6:30 AM	0	3	1	4		0	0	0	0		0	9	0	9		0	0	0	0		13	0	13
6:30 AM - 6:45 AM	0	5	3	8		0	0	2	2		8	10	0	18		0	0	0	0		26	2	28
6:45 AM - 7:00 AM	0	4	7	11		1	0	3	4		22	16	0	38		0	0	0	0		49	4	53
7:00 AM - 7:15 AM	0	12	26	38		7	0	2	9		62	18	0	80		0	0	0	0		118	9	127
7:15 AM - 7:30 AM	0	16	34	50		12	0	5	17		51	31	0	82		0	0	0	0		132	17	149
7:30 AM - 7:45 AM	2	13	12	27		23	0	7	30		26	29	0	55		0	0	0	0		82	30	112
7:45 AM - 8:00 AM	2	12	3	17		6	0	2	8		6	15	0	21		1	0	0	1		38	9	47
8:00 AM - 8:15 AM	0	11	6	17		3	0	1	4		8	34	0	42		0	0	0	0		59	4	63
8:15 AM - 8:30 AM	0	16	5	21		0	0	0	0		15	31	4	50		0	0	0	0		71	0	71
8:30 AM - 8:45 AM	0	22	4	26		3	0	1	4		14	20	1	35		0	0	0	0		61	4	65
8:45 AM - 9:00 AM	0	13	0	13		5	0	3	8		8	20	0	28		0	0	0	0		41	8	49
Total	4	128	103	235		60	0	26	86		220	238	5	463		1	0	1	2		698	88	786
AM One Hour Volumes																							
6:00 AM - 7:00 AM	0	13	13	26	0.59	1	0	5	6	0.38	30	40	0	70	0.46	0	0	1	1	0.25	96	7	103
6:15 AM - 7:15 AM	0	24	37	61	0.40	8	0	7	15	0.42	92	53	0	145	0.45	0	0	0	0	0.00	206	15	221
6:30 AM - 7:30 AM	0	37	70	107	0.54	20	0	12	32	0.47	143	75	0	218	0.66	0	0	0	0	0.00	325	32	357
6:45 AM - 7:45 AM	2	45	79	126	0.63	43	0	17	60	0.50	161	94	0	255	0.78	0	0	0	0	0.00	381	60	441
7:00 AM - 8:00 AM	4	53	75	132	0.66	48	0	16	64	0.53	145	93	0	238	0.73	1	0	0	1	0.25	370	65	435
7:15 AM - 8:15 AM	4	52	55	111	0.56	44	0	15	59	0.49	91	109	0	200	0.61	1	0	0	1	0.25	311	60	371
7:30 AM - 8:30 AM	4	52	26	82	0.76	32	0	10	42	0.35	55	109	4	168	0.76	1	0	0	1	0.25	250	43	293
7:45 AM - 8:45 AM	2	61	18	81	0.78	12	0	4	16	0.50	43	100	5	148	0.74	1	0	0	1	0.25	229	17	246
8:00 AM - 9:00 AM	0	62	15	77	0.74	11	0	5	16	0.50	45	105	5	155	0.78	0	0	0	0	0.00	232	16	248
PM 15 Minute Volumes																							
2:00 PM - 2:15 PM	0	18	1	19		6	0	4	10		5	13	0	18		1	1	0	2		37	12	49
2:15 PM - 2:30 PM	0	11	3	14		6	1	2	9		2	10	0	12		0	0	0	0		26	9	35
2:30 PM - 2:45 PM	2	17	5	24		7	0	1	8		7	23	0	30		1	0	0	1		54	9	63
2:45 PM - 3:00 PM	2	14	6	22		18	0	2	20		8	25	1	34		0	0	1	1		56	21	77
3:00 PM - 3:15 PM	0	18	5	23		60	0	20	80		5	18	0	23		1	0	1	2		46	82	128
3:15 PM - 3:30 PM	0	17	6	23		24	0	9	33		3	19	2	24		1	0	1	2		47	35	82
3:30 PM - 3:45 PM	2	17	5	24		14	0	6	20		1	13	2	16		0	0	0	0		40	20	60
3:45 PM - 4:00 PM	1	28	6	35		11	0	6	17		5	16	1	22		0	0	1	1		57	18	75
4:00 PM - 4:15 PM	1	26	2	29		10	0	3	13		0	15	0	15		2	0	0	2		44	15	59
4:15 PM - 4:30 PM	0	12	3	15		8	0	5	13		4	23	0	27		1	0	0	1		42	14	56
4:30 PM - 4:45 PM	0	18	1	19		4	0	2	6		4	24	0	28		1	0	2	3		47	9	56
4:45 PM - 5:00 PM	0	15	3	18		7	0	6	13		4	26	0	30		0	0	0	0		48	13	61
5:00 PM - 5:15 PM	0	17	6	23		8	0	4	12		1	16	1	18		0	0	1	1		41	13	54
5:15 PM - 5:30 PM	0	17	4	21		2	0	3	5		5	10	0	15		0	0	0	0		36	5	41
5:30 PM - 5:45 PM	0	20	3	23		4	0	6	10		0	15	0	15		0	0	0	0		38	10	48
5:45 PM - 6:00 PM	0	21	2	23		3	0	6	9		4	17	0	21		0	0	0	0		44	9	53
6:00 PM - 6:15 PM	1	14	4	19		5	0	8	13		3	17	0	20		1	0	1	2		39	15	54
6:15 PM - 6:30 PM	0	19	3	22		6	0	6	12		2	22	0	24		1	0	1	2		46	14	60
6:30 PM - 6:45 PM	0	19	4	23		1	0	6	7		6	17	0	23		0	0	0	0		46	7	53
6:45 PM - 7:00 PM	1	17	3	21		5	0	1	6		3	11	0	14		0	0	0	0		35	6	41
Total	3	215	38	256		63	0	56	119		36	213	1	250		6	0	5	11		506	130	636
PM One Hour Volumes																							
2:00 PM - 3:00 PM	4	60	15	79	0.82	37	1	9	47	0.59	22	71	1	94	0.69	2	1	1	4	0.50	173	51	224
2:15 PM - 3:15 PM	4	60	19	83	0.86	91	1	25	117	0.37	22	76	1	99	0.73	2	0	2	4	0.50	182	121	303
2:30 PM - 3:30 PM	4	66	22	92	0.96	109	0	32	141	0.44	23	85	3	111	0.82	3	0	3	6	0.75	203	147	350
2:45 PM - 3:45 PM	4	66	22	92	0.96	116	0	37	153	0.48	17	75	5	97	0.71	2	0	3	5	0.63	189	158	347
3:00 PM - 4:00 PM	3	80	22	105	0.75	109	0	41	150	0.47	14	66	5	85	0.89	2	0	3	5	0.63	190	155	345
3:15 PM - 4:15 PM	4	88	19	111	0.79	59	0	24	83	0.63	9	63	5	77	0.80	3	0	2	5	0.63	188	88	276
3:30 PM - 4:30 PM	4	83	16	103	0.74	43	0	20	63	0.79	10	67	3	80	0.74	3	0	1	4	0.50	183	67	250
3:45 PM - 4:45 PM	2	84	12	98	0.70	33	0	16	49	0.72	13	78	1	92	0.82	4	0	3	7	0.58	190	56	246
4:00 PM - 5:00 PM	1	71	9	81	0.70	29	0	16	45	0.87	12	88	0	100	0.83	4	0	2	6	0.50	181	51	232
4:15 PM - 5:15 PM	0	62	13	75	0.82	27	0	17	44	0.85	13	89	1	103	0.86	2	0	3	5	0.42	178	49	227
4:30 PM - 5:30 PM	0	67	14	81	0.88	21	0	15	36	0.69	14	76	1	91	0.76	1	0	3	4	0.33	172	40	212
4:45 PM - 5:45 PM	0	69	16	85	0.92	21	0	19	40	0.77	10	67	1	78	0.65	0	0	1	1	0.25	163	41	204
5:00 PM - 6:00 PM	0	75	15	90	0.98	17	0	19	36	0.75	10	58	1	69	0.82	0	0	1	1	0.25	159	37	196
5:15 PM - 6:15 PM	1	72	13	86	0.93	14	0	23	37	0.71	12	59	0	71	0.85	1	0	1	2	0.25	157	39	196
5:30 PM - 6:30 PM	1	74	12	87	0.95	18	0	26	44	0.85	9	71	0	80	0.83	2	0	2	4	0.50	167	48	215
5:45 PM - 6:45 PM	1	73	13	87	0.95	15	0	26	41	0.79	15	73	0	88	0.92	2	0	2	4	0.50	175	45	220
6:00 PM - 7:00 PM	2	69	14	85	0.92	17	0	21	38	0.73	14	67	0	81	0.84	2	0	2	4	0.50	166	42	208

Wells + Associates, Inc.

McLean, Virginia

Turning Movement Count - All Vehicles

PROJECT: Paul VI Development	DATE: 2/3/2016	SOUTHBOUND ROAD: Oak Street
W+A JOB NO: 6709	DAY: Wednesday	NORTHBOUND ROAD: Oak Street
INTERSECTION: Cedar Ave. & Oak St.	WEATHER: rain	WESTBOUND ROAD: Cedar Avenue
LOCATION: Fairfax County, VA	COUNTED BY: Geraldin	EASTBOUND ROAD: Cedar Avenue
INPUTED BY: agan		

Time Period	Southbound Oak Street				Westbound Cedar Avenue				Northbound Oak Street				Eastbound Cedar Avenue				North & South	East & West	Total				
	Right	Thru	Left	Total	PHF	Right	Thru	Left	Total	PHF	Right	Thru	Left	Total	PHF	Right				Thru	Left	Total	PHF
AM 15 Minute Volumes																							
6:00 AM - 6:15 AM	0	3	0	3		0	1	0	1		2	3	0	5		0	2	1	3		8	4	12
6:15 AM - 6:30 AM	0	2	3	5		1	0	1	2		1	6	0	7		0	0	0	0		12	2	14
6:30 AM - 6:45 AM	2	9	4	15		1	0	0	1		8	8	0	16		2	5	2	9		31	10	41
6:45 AM - 7:00 AM	3	7	17	27		8	3	4	15		22	13	0	35		2	21	2	25		62	40	102
7:00 AM - 7:15 AM	2	3	31	36		25	9	11	45		48	11	0	59		0	50	1	51		95	96	191
7:15 AM - 7:30 AM	1	4	71	76		62	17	17	96		66	1	3	70		2	68	1	71		146	167	313
7:30 AM - 7:45 AM	2	10	54	66		70	29	30	129		63	7	1	71		0	38	2	40		137	169	306
7:45 AM - 8:00 AM	0	21	2	23		10	8	7	25		1	7	1	9		1	5	6	12		32	37	69
8:00 AM - 8:15 AM	1	18	3	22		1	1	0	2		1	12	1	14		1	5	7	13		36	15	51
8:15 AM - 8:30 AM	0	13	10	23		2	0	0	2		4	15	0	19		2	10	7	19		42	21	63
8:30 AM - 8:45 AM	1	21	8	30		5	4	4	13		6	17	0	23		3	7	7	17		53	30	83
8:45 AM - 9:00 AM	3	21	3	27		4	2	5	11		7	18	1	26		1	4	6	11		53	22	75
Total	15	132	206	353		189	74	79	342		229	118	7	354		14	215	42	271		707	613	1320
AM One Hour Volumes																							
6:00 AM - 7:00 AM	5	21	24	50	0.46	10	4	5	19	0.32	33	30	0	63	0.45	4	28	5	37	0.37	113	56	169
6:15 AM - 7:15 AM	7	21	55	83	0.58	35	12	16	63	0.35	79	38	0	117	0.50	4	76	5	85	0.42	200	148	348
6:30 AM - 7:30 AM	8	23	123	154	0.51	96	29	32	157	0.41	144	33	3	180	0.64	6	144	6	156	0.55	334	313	647
6:45 AM - 7:45 AM	8	24	173	205	0.67	165	58	62	285	0.55	199	32	4	235	0.83	4	177	6	187	0.66	440	472	912
7:00 AM - 8:00 AM	5	38	158	201	0.66	167	63	65	295	0.57	178	26	5	209	0.74	3	161	10	174	0.61	410	469	879
7:15 AM - 8:15 AM	4	53	130	187	0.62	143	55	54	252	0.49	131	27	6	164	0.58	4	116	16	136	0.48	351	388	739
7:30 AM - 8:30 AM	3	62	69	134	0.51	83	38	37	158	0.31	69	41	3	113	0.40	4	58	22	84	0.53	247	242	489
7:45 AM - 8:45 AM	2	73	23	98	0.82	18	13	11	42	0.42	12	51	2	65	0.71	7	27	27	61	0.80	163	103	266
8:00 AM - 9:00 AM	5	73	24	102	0.85	12	7	9	28	0.54	18	62	2	82	0.79	7	26	27	60	0.79	184	88	272
PM 15 Minute Volumes																							
2:00 PM - 2:15 PM	3	29	3	35		8	6	2	16		2	14	0	16		1	1	4	6		51	22	73
2:15 PM - 2:30 PM	2	26	4	32		2	4	0	6		2	25	1	28		1	5	0	6		60	12	72
2:30 PM - 2:45 PM	3	16	11	30		2	3	3	8		3	17	2	22		0	7	3	10		52	18	70
2:45 PM - 3:00 PM	2	29	18	49		11	15	8	34		7	18	4	29		4	7	4	15		78	49	127
3:00 PM - 3:15 PM	3	32	13	48		46	63	33	142		7	20	1	28		3	5	1	9		76	151	227
3:15 PM - 3:30 PM	5	17	6	28		31	27	15	73		8	35	2	45		3	3	4	10		73	83	156
3:30 PM - 3:45 PM	3	16	9	28		23	16	16	55		7	11	1	19		2	2	1	5		47	60	107
3:45 PM - 4:00 PM	2	15	7	24		9	14	10	33		11	19	1	31		1	10	4	15		55	48	103
4:00 PM - 4:15 PM	0	18	9	27		10	12	13	35		5	21	2	28		1	0	0	1		55	36	91
4:15 PM - 4:30 PM	6	17	6	29		7	8	7	22		5	19	1	25		1	1	6	8		54	30	84
4:30 PM - 4:45 PM	1	17	3	21		11	3	2	16		3	18	1	22		1	2	2	5		43	21	64
4:45 PM - 5:00 PM	6	18	5	29		3	4	4	11		0	20	1	21		0	3	4	7		50	18	68
5:00 PM - 5:15 PM	4	12	5	21		5	3	2	10		2	22	5	29		3	1	2	6		50	16	66
5:15 PM - 5:30 PM	2	16	4	22		5	4	4	13		2	17	0	19		3	4	1	8		41	21	62
5:30 PM - 5:45 PM	5	15	7	27		7	3	1	11		1	18	4	23		2	2	0	4		50	15	65
5:45 PM - 6:00 PM	4	12	7	23		6	3	8	17		5	16	1	22		2	1	0	3		45	20	65
6:00 PM - 6:15 PM	8	19	5	32		5	3	4	12		3	15	3	21		3	5	1	9		53	21	74
6:15 PM - 6:30 PM	0	16	4	20		10	13	8	31		0	12	1	13		0	5	0	5		33	36	69
6:30 PM - 6:45 PM	2	15	7	24		8	6	7	21		3	14	0	17		2	1	3	6		41	27	68
6:45 PM - 7:00 PM	1	26	8	35		3	2	2	7		3	7	4	14		0	7	2	9		49	16	65
Total	39	201	70	310		80	64	62	206		32	199	23	254		18	32	21	71		564	277	841
PM One Hour Volumes																							
2:00 PM - 3:00 PM	10	100	36	146	0.74	23	28	13	64	0.47	14	74	7	95	0.82	6	20	11	37	0.62	241	101	342
2:15 PM - 3:15 PM	10	103	46	159	0.81	61	85	44	190	0.33	19	80	8	107	0.92	8	24	8	40	0.67	266	230	496
2:30 PM - 3:30 PM	13	94	48	155	0.79	90	108	59	257	0.45	25	90	9	124	0.69	10	22	12	44	0.73	279	301	580
2:45 PM - 3:45 PM	13	94	46	153	0.78	111	121	72	304	0.54	29	84	8	121	0.67	12	17	10	39	0.65	274	343	617
3:00 PM - 4:00 PM	13	80	35	128	0.67	109	120	74	303	0.53	33	85	5	123	0.68	9	20	10	39	0.65	251	342	593
3:15 PM - 4:15 PM	10	66	31	107	0.96	73	69	54	196	0.67	31	86	6	123	0.68	7	15	9	31	0.52	230	227	457
3:30 PM - 4:30 PM	11	66	31	108	0.93	49	50	46	145	0.66	28	70	5	103	0.83	5	13	11	29	0.48	211	174	385
3:45 PM - 4:45 PM	9	67	25	101	0.87	37	37	32	106	0.76	24	77	5	106	0.85	4	13	12	29	0.48	207	135	342
4:00 PM - 5:00 PM	13	70	23	106	0.91	31	27	26	84	0.60	13	78	5	96	0.86	3	6	12	21	0.66	202	105	307
4:15 PM - 5:15 PM	17	64	19	100	0.86	26	18	15	59	0.67	10	79	8	97	0.84	5	7	14	26	0.81	197	85	282
4:30 PM - 5:30 PM	13	63	17	93	0.80	24	14	12	50	0.78	7	77	7	91	0.78	7	10	9	26	0.81	184	76	260
4:45 PM - 5:45 PM	17	61	21	99	0.85	20	14	11	45	0.87	5	77	10	92	0.79	8	10	7	25	0.78	191	70	261
5:00 PM - 6:00 PM	15	55	23	93	0.86	23	13	15	51	0.75	10	73	10	93	0.80	10	8	3	21	0.66	186	72	258
5:15 PM - 6:15 PM	19	62	23	104	0.81	23	13	17	53	0.78	11	66	8	85	0.92	10	12	2	24	0.67	189	77	266
5:30 PM - 6:30 PM	17	62	23	102	0.80	28	22	21	71	0.57	9	61	9	79	0.86	7	13	1	21	0.58	181	92	273
5:45 PM - 6:45 PM	14	62	23	99	0.77	29	25	27	81	0.65	11	57	5	73	0.83	7	12	4	23	0.64	172	104	276
6:00 PM - 7:00 PM	11	76	24	111	0.79	26	24	21	71	0.57	9	48	8	65	0.77	5	18	6	29	0.81	176	100	276
























APPENDIX C

Existing Capacity Analysis Worksheets

HCM Signalized Intersection Capacity Analysis

1: Lee Highway & Fairfax Boulevard & Main Street
























Existing AM

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	474	845	17	341	689	36	9	954	732	43	350	227
Future Volume (vph)	474	845	17	341	689	36	9	954	732	43	350	227
Ideal Flow (vphp)	2500	2500	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	5.1	5.1		4.3	4.3	6.3	4.8	5.6	5.6	4.8	5.6	5.6
Lane Util. Factor	0.97	0.95		0.97	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00
Frt	1.00	1.00		1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85
Flt Protected	0.95	1.00		0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)	4430	4558		3335	3539	1509	1805	3539	1568	1703	3343	1524
Flt Permitted	0.95	1.00		0.95	1.00	1.00	0.43	1.00	1.00	0.07	1.00	1.00
Satd. Flow (perm)	4430	4558		3335	3539	1509	820	3539	1568	121	3343	1524
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	515	918	18	371	749	39	10	1037	796	47	380	247
RTOR Reduction (vph)	0	1	0	0	0	31	0	0	0	0	0	61
Lane Group Flow (vph)	515	935	0	371	749	8	10	1037	796	47	380	186
Heavy Vehicles (%)	4%	4%	0%	5%	2%	7%	0%	2%	3%	6%	8%	6%
Turn Type	Prot	NA		Prot	NA	Perm	pm+pt	NA	pt+ov	pm+pt	NA	pt+ov
Protected Phases	5	2		1	6		3	8	8	1	4	4
Permitted Phases						6	8			4		
Actuated Green, G (s)	59.9	59.9		37.8	37.8	37.8	64.5	57.4	95.2	64.5	57.4	124.4
Effective Green, g (s)	61.9	61.9		39.8	39.8	37.8	68.5	59.4	99.2	68.5	59.4	121.3
Actuated g/C Ratio	0.33	0.33		0.21	0.21	0.20	0.36	0.31	0.52	0.36	0.31	0.64
Clearance Time (s)	7.1	7.1		6.3	6.3	6.3	6.8	7.6		6.8	7.6	
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0		3.0	3.0	
Lane Grp Cap (vph)	1443	1484		698	741	300	342	1106	818	119	1045	972
v/s Ratio Prot	0.12	c0.21		0.11	c0.21		0.00	0.29	c0.51	c0.02	0.11	0.12
v/s Ratio Perm						0.01	0.01			0.12		
v/c Ratio	0.36	0.63		0.53	1.01	0.03	0.03	0.94	0.97	0.39	0.36	0.19
Uniform Delay, d1	48.9	54.3		66.8	75.1	61.3	46.5	63.5	44.1	82.3	50.6	14.1
Progression Factor	1.00	1.00		1.00	1.00	1.00	1.00	1.00	1.00	1.14	1.21	2.98
Incremental Delay, d2	0.7	2.0		2.9	35.8	0.2	0.0	15.7	25.6	2.1	1.0	0.4
Delay (s)	49.6	56.4		69.7	110.9	61.4	46.5	79.2	69.7	95.6	62.4	42.6
Level of Service	D	E		E	F	E	D	E	E	F	E	D
Approach Delay (s)		54.0			96.0			74.9			57.4	
Approach LOS		D			F			E			E	
Intersection Summary												
HCM 2000 Control Delay			71.5	HCM 2000 Level of Service				E				
HCM 2000 Volume to Capacity ratio			0.83									
Actuated Cycle Length (s)			190.0	Sum of lost time (s)				19.8				
Intersection Capacity Utilization			82.2%	ICU Level of Service				E				
Analysis Period (min)			15									
c Critical Lane Group												

HCM Signalized Intersection Capacity Analysis

1: Lee Highway & Fairfax Boulevard & Main Street
























Existing PM School

													
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations													
Traffic Volume (vph)	438	514	3	617	843	53	30	781	457	72	628	388	
Future Volume (vph)	438	514	3	617	843	53	30	781	457	72	628	388	
Ideal Flow (vphp)	2500	2500	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Total Lost time (s)	5.1	5.1		4.3	4.3	6.3	4.8	5.6	5.6	4.8	5.6	5.6	
Lane Util. Factor	0.97	0.95		0.97	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	
Frt	1.00	1.00		1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	
Flt Protected	0.95	1.00		0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	
Satd. Flow (prot)	4430	4565		3335	3539	1509	1805	3539	1568	1703	3343	1524	
Flt Permitted	0.95	1.00		0.95	1.00	1.00	0.25	1.00	1.00	0.17	1.00	1.00	
Satd. Flow (perm)	4430	4565		3335	3539	1509	474	3539	1568	296	3343	1524	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	476	559	3	671	916	58	33	849	497	78	683	422	
RTOR Reduction (vph)	0	0	0	0	0	40	0	0	0	0	0	44	
Lane Group Flow (vph)	476	562	0	671	916	18	33	849	497	78	683	378	
Heavy Vehicles (%)	4%	4%	0%	5%	2%	7%	0%	2%	3%	6%	8%	6%	
Turn Type	Prot	NA		Prot	NA	Perm	pm+pt	NA	pt+ov	pm+pt	NA	pt+ov	
Protected Phases	5	2		1	6		3	8	8	1	4	4	
Permitted Phases						6	8			4			
Actuated Green, G (s)	37.9	37.9		68.9	68.9	68.9	85.4	78.4	147.3	85.4	78.4	123.4	
Effective Green, g (s)	39.9	39.9		70.9	70.9	68.9	89.4	80.4	151.3	89.4	80.4	120.3	
Actuated g/C Ratio	0.18	0.18		0.32	0.32	0.31	0.41	0.37	0.69	0.41	0.37	0.55	
Clearance Time (s)	7.1	7.1		6.3	6.3	6.3	6.8	7.6		6.8	7.6		
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0		3.0	3.0		
Lane Grp Cap (vph)	803	827		1074	1140	472	247	1293	1078	177	1221	833	
v/s Ratio Prot	0.11	c0.12		0.20	c0.26		0.01	c0.24	0.32	c0.02	0.20	0.25	
v/s Ratio Perm						0.01	0.05			0.16			
v/c Ratio	0.59	0.68		0.62	0.80	0.04	0.13	0.66	0.46	0.44	0.56	0.45	
Uniform Delay, d1	82.6	84.1		63.3	68.2	52.5	60.8	58.3	15.7	76.5	55.7	30.0	
Progression Factor	1.00	1.00		1.00	1.00	1.00	1.00	1.00	1.00	0.91	0.97	1.13	
Incremental Delay, d2	3.2	4.5		2.7	6.0	0.2	0.2	2.6	1.4	1.6	1.7	1.7	
Delay (s)	85.8	88.6		66.0	74.2	52.7	61.0	60.9	17.1	71.4	55.6	35.6	
Level of Service	F	F		E	E	D	E	E	B	E	E	D	
Approach Delay (s)		87.3			70.1			45.1			49.5		
Approach LOS		F			E			D			D		
Intersection Summary													
HCM 2000 Control Delay			62.3									HCM 2000 Level of Service	E
HCM 2000 Volume to Capacity ratio			0.70										
Actuated Cycle Length (s)			220.0									Sum of lost time (s)	19.8
Intersection Capacity Utilization			76.7%									ICU Level of Service	D
Analysis Period (min)			15										
c Critical Lane Group													

HCM Signalized Intersection Capacity Analysis

1: Lee Highway & Fairfax Boulevard & Main Street

Existing PM

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	385	551	74	638	953	36	34	575	523	49	915	437
Future Volume (vph)	385	551	74	638	953	36	34	575	523	49	915	437
Ideal Flow (vphpl)	2500	2500	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	5.1	5.1		4.3	4.3	6.3	4.8	5.6	5.6	4.8	5.6	5.6
Lane Util. Factor	0.97	0.95		0.97	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00
Frt	1.00	0.98		1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85
Flt Protected	0.95	1.00		0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)	4430	4507		3335	3539	1509	1805	3539	1568	1703	3343	1524
Flt Permitted	0.95	1.00		0.95	1.00	1.00	0.10	1.00	1.00	0.28	1.00	1.00
Satd. Flow (perm)	4430	4507		3335	3539	1509	195	3539	1568	507	3343	1524
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	418	599	80	693	1036	39	37	625	568	53	995	475
RTOR Reduction (vph)	0	5	0	0	0	27	0	0	0	0	0	32
Lane Group Flow (vph)	418	674	0	693	1036	12	37	625	568	53	995	443
Heavy Vehicles (%)	4%	4%	0%	5%	2%	7%	0%	2%	3%	6%	8%	6%
Turn Type	Prot	NA		Prot	NA	Perm	pm+pt	NA	pt+ov	pm+pt	NA	pt+ov
Protected Phases	5	2		1	6		3	8	8	1	4	4
Permitted Phases						6	8			4		
Actuated Green, G (s)	37.9	37.9		68.9	68.9	68.9	85.4	78.4	147.3	85.4	78.4	123.4
Effective Green, g (s)	39.9	39.9		70.9	70.9	68.9	89.4	80.4	151.3	89.4	80.4	120.3
Actuated g/C Ratio	0.18	0.18		0.32	0.32	0.31	0.41	0.37	0.69	0.41	0.37	0.55
Clearance Time (s)	7.1	7.1		6.3	6.3	6.3	6.8	7.6		6.8	7.6	
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0		3.0	3.0	
Lane Grp Cap (vph)	803	817		1074	1140	472	145	1293	1078	254	1221	833
v/s Ratio Prot	0.09	c0.15		0.21	c0.29		c0.01	0.18	0.36	0.01	c0.30	0.29
v/s Ratio Perm						0.01	0.09			0.08		
v/c Ratio	0.52	0.83		0.65	0.91	0.03	0.26	0.48	0.53	0.21	0.81	0.53
Uniform Delay, d1	81.4	86.7		63.8	71.5	52.3	81.0	53.8	16.8	59.7	63.1	31.9
Progression Factor	1.00	1.00		1.00	1.00	1.00	1.00	1.00	1.00	1.25	1.19	1.56
Incremental Delay, d2	2.4	9.3		3.0	12.1	0.1	0.9	1.3	1.8	0.4	5.4	2.2
Delay (s)	83.8	96.0		66.8	83.6	52.4	81.9	55.1	18.7	75.1	80.8	51.9
Level of Service	F	F		E	F	D	F	E	B	E	F	D
Approach Delay (s)		91.3			76.3			39.1			71.6	
Approach LOS		F			E			D			E	
Intersection Summary												
HCM 2000 Control Delay			69.8	HCM 2000 Level of Service				E				
HCM 2000 Volume to Capacity ratio			0.82									
Actuated Cycle Length (s)			220.0	Sum of lost time (s)				19.8				
Intersection Capacity Utilization			82.3%	ICU Level of Service				E				
Analysis Period (min)			15									
c Critical Lane Group												

HCM Signalized Intersection Capacity Analysis

2: Walnut Street/Fairchester Drive & Fairfax Boulevard

Existing AM



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	15	1616	34	32	600	21	45	27	71	28	44	19
Future Volume (vph)	15	1616	34	32	600	21	45	27	71	28	44	19
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	5.6	5.6		5.6	5.6		4.5	4.5		4.5	4.5	
Lane Util. Factor	1.00	0.95		1.00	0.95		1.00	1.00		1.00	1.00	
Frt	1.00	1.00		1.00	0.99		1.00	0.89		1.00	0.95	
Flt Protected	0.95	1.00		0.95	1.00		0.95	1.00		0.95	1.00	
Satd. Flow (prot)	1671	3496		1805	3395		1805	1538		1752	1776	
Flt Permitted	0.38	1.00		0.09	1.00		0.64	1.00		0.40	1.00	
Satd. Flow (perm)	677	3496		165	3395		1210	1538		746	1776	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	16	1757	37	35	652	23	49	29	77	30	48	21
RTOR Reduction (vph)	0	0	0	0	1	0	0	61	0	0	9	0
Lane Group Flow (vph)	16	1794	0	35	674	0	49	45	0	30	60	0
Heavy Vehicles (%)	8%	3%	0%	0%	6%	0%	0%	5%	12%	3%	3%	0%
Turn Type	pm+pt	NA		pm+pt	NA		Perm	NA		Perm	NA	
Protected Phases	5	2		1	6			7			3	
Permitted Phases	2			6			7			3		
Actuated Green, G (s)	146.9	142.7		150.3	144.4		21.7	21.7		12.3	12.3	
Effective Green, g (s)	148.9	143.7		152.3	145.4		23.7	23.7		14.3	14.3	
Actuated g/C Ratio	0.78	0.76		0.80	0.77		0.12	0.12		0.08	0.08	
Clearance Time (s)	6.6	6.6		6.6	6.6		6.5	6.5		6.5	6.5	
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Lane Grp Cap (vph)	557	2644		191	2598		150	191		56	133	
v/s Ratio Prot	0.00	c0.51		c0.01	0.20			0.03			0.03	
v/s Ratio Perm	0.02			0.14			c0.04			c0.04		
v/c Ratio	0.03	0.68		0.18	0.26		0.33	0.23		0.54	0.45	
Uniform Delay, d1	4.5	11.6		10.7	6.5		75.9	75.0		84.7	84.1	
Progression Factor	0.37	0.62		1.00	0.74		1.00	1.00		1.00	1.00	
Incremental Delay, d2	0.0	1.1		0.4	0.2		1.3	0.6		9.5	2.4	
Delay (s)	1.7	8.3		11.1	5.1		77.1	75.6		94.2	86.5	
Level of Service	A	A		B	A		E	E		F	F	
Approach Delay (s)		8.2			5.3			76.1			88.8	
Approach LOS		A			A			E			F	

Intersection Summary

HCM 2000 Control Delay	14.1	HCM 2000 Level of Service	B
HCM 2000 Volume to Capacity ratio	0.64		
Actuated Cycle Length (s)	190.0	Sum of lost time (s)	20.7
Intersection Capacity Utilization	63.3%	ICU Level of Service	B
Analysis Period (min)	15		
c Critical Lane Group			

HCM Signalized Intersection Capacity Analysis

2: Walnut Street/Fairchester Drive & Fairfax Boulevard

Existing PM School



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	19	877	21	47	1015	19	106	54	45	44	28	12
Future Volume (vph)	19	877	21	47	1015	19	106	54	45	44	28	12
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	5.6	5.6		5.6	5.6		4.5	4.5		4.5	4.5	
Lane Util. Factor	1.00	0.95		1.00	0.95		1.00	1.00		1.00	1.00	
Frt	1.00	1.00		1.00	1.00		1.00	0.93		1.00	0.95	
Flt Protected	0.95	1.00		0.95	1.00		0.95	1.00		0.95	1.00	
Satd. Flow (prot)	1671	3495		1805	3400		1805	1637		1752	1777	
Flt Permitted	0.23	1.00		0.26	1.00		0.72	1.00		0.47	1.00	
Satd. Flow (perm)	397	3495		489	3400		1373	1637		872	1777	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	21	953	23	51	1103	21	115	59	49	48	30	13
RTOR Reduction (vph)	0	1	0	0	0	0	0	15	0	0	7	0
Lane Group Flow (vph)	21	975	0	51	1124	0	115	93	0	48	36	0
Heavy Vehicles (%)	8%	3%	0%	0%	6%	0%	0%	5%	12%	3%	3%	0%
Turn Type	pm+pt	NA		pm+pt	NA		Perm	NA		Perm	NA	
Protected Phases	5	2		1	6			7			3	
Permitted Phases	2			6			7			3		
Actuated Green, G (s)	166.0	161.7		172.2	164.8		31.2	31.2		21.8	21.8	
Effective Green, g (s)	168.0	162.7		174.2	165.8		33.2	33.2		23.8	23.8	
Actuated g/C Ratio	0.76	0.74		0.79	0.75		0.15	0.15		0.11	0.11	
Clearance Time (s)	6.6	6.6		6.6	6.6		6.5	6.5		6.5	6.5	
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Lane Grp Cap (vph)	333	2584		437	2562		207	247		94	192	
v/s Ratio Prot	0.00	0.28		c0.00	c0.33			0.06			0.02	
v/s Ratio Perm	0.05			0.09			c0.08			0.06		
v/c Ratio	0.06	0.38		0.12	0.44		0.56	0.38		0.51	0.19	
Uniform Delay, d1	7.0	10.4		5.9	10.0		86.6	84.1		92.6	89.3	
Progression Factor	0.07	0.10		0.30	0.21		1.00	1.00		1.00	1.00	
Incremental Delay, d2	0.1	0.3		0.1	0.5		3.2	1.0		4.6	0.5	
Delay (s)	0.6	1.4		1.9	2.6		89.8	85.0		97.2	89.8	
Level of Service	A	A		A	A		F	F		F	F	
Approach Delay (s)		1.4			2.6			87.5			93.7	
Approach LOS		A			A			F			F	

Intersection Summary			
HCM 2000 Control Delay	13.0	HCM 2000 Level of Service	B
HCM 2000 Volume to Capacity ratio	0.46		
Actuated Cycle Length (s)	220.0	Sum of lost time (s)	20.7
Intersection Capacity Utilization	60.0%	ICU Level of Service	B
Analysis Period (min)	15		
c Critical Lane Group			

HCM Signalized Intersection Capacity Analysis

2: Walnut Street/Fairchester Drive & Fairfax Boulevard

Existing PM



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	28	811	12	48	1297	11	41	29	63	33	25	15
Future Volume (vph)	28	811	12	48	1297	11	41	29	63	33	25	15
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	5.6	5.6		5.6	5.6		4.5	4.5		4.5	4.5	
Lane Util. Factor	1.00	0.95		1.00	0.95		1.00	1.00		1.00	1.00	
Frt	1.00	1.00		1.00	1.00		1.00	0.90		1.00	0.94	
Flt Protected	0.95	1.00		0.95	1.00		0.95	1.00		0.95	1.00	
Satd. Flow (prot)	1671	3499		1805	3403		1805	1554		1752	1761	
Flt Permitted	0.16	1.00		0.29	1.00		0.71	1.00		0.38	1.00	
Satd. Flow (perm)	279	3499		558	3403		1357	1554		702	1761	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	30	882	13	52	1410	12	45	32	68	36	27	16
RTOR Reduction (vph)	0	0	0	0	0	0	0	41	0	0	10	0
Lane Group Flow (vph)	30	895	0	52	1422	0	45	59	0	36	33	0
Heavy Vehicles (%)	8%	3%	0%	0%	6%	0%	0%	5%	12%	3%	3%	0%
Turn Type	pm+pt	NA		pm+pt	NA		Perm	NA		Perm	NA	
Protected Phases	5	2		1	6			7			3	
Permitted Phases	2			6			7			3		
Actuated Green, G (s)	175.8	170.1		179.0	171.7		22.9	22.9		13.5	13.5	
Effective Green, g (s)	177.8	171.1		181.0	172.7		24.9	24.9		15.5	15.5	
Actuated g/C Ratio	0.81	0.78		0.82	0.78		0.11	0.11		0.07	0.07	
Clearance Time (s)	6.6	6.6		6.6	6.6		6.5	6.5		6.5	6.5	
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Lane Grp Cap (vph)	267	2721		506	2671		153	175		49	124	
v/s Ratio Prot	0.00	0.26		c0.00	c0.42			c0.04			0.02	
v/s Ratio Perm	0.09			0.08			0.03			c0.05		
v/c Ratio	0.11	0.33		0.10	0.53		0.29	0.34		0.73	0.26	
Uniform Delay, d1	5.9	7.3		4.0	8.7		89.5	90.0		100.2	96.8	
Progression Factor	0.13	0.16		0.12	0.13		1.00	1.00		1.00	1.00	
Incremental Delay, d2	0.2	0.3		0.1	0.6		1.1	1.2		43.4	1.1	
Delay (s)	0.9	1.4		0.5	1.8		90.6	91.1		143.7	98.0	
Level of Service	A	A		A	A		F	F		F	F	
Approach Delay (s)		1.4			1.7			90.9			118.8	
Approach LOS		A			A			F			F	

Intersection Summary

HCM 2000 Control Delay	10.1	HCM 2000 Level of Service	B
HCM 2000 Volume to Capacity ratio	0.53		
Actuated Cycle Length (s)	220.0	Sum of lost time (s)	20.7
Intersection Capacity Utilization	57.2%	ICU Level of Service	B
Analysis Period (min)	15		
c Critical Lane Group			

HCM Signalized Intersection Capacity Analysis

3: Oak Street/Meredith Drive & Fairfax Boulevard

Existing AM



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	8	1607	87	128	647	2	97	1	148	32	2	13
Future Volume (vph)	8	1607	87	128	647	2	97	1	148	32	2	13
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	5.6	4.6		5.6	4.6			4.5	4.5		4.5	
Lane Util. Factor	1.00	0.95		1.00	0.95			1.00	1.00		1.00	
Frt	1.00	0.99		1.00	1.00			1.00	0.85		0.96	
Flt Protected	0.95	1.00		0.95	1.00			0.95	1.00		0.97	
Satd. Flow (prot)	1597	3483		1805	3405			1810	1615		1769	
Flt Permitted	0.38	1.00		0.03	1.00			0.95	1.00		0.97	
Satd. Flow (perm)	645	3483		65	3405			1810	1615		1769	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	9	1747	95	139	703	2	105	1	161	35	2	14
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	146	0	8	0
Lane Group Flow (vph)	9	1842	0	139	705	0	0	106	15	0	43	0
Heavy Vehicles (%)	13%	3%	0%	0%	6%	0%	0%	0%	0%	0%	0%	0%
Turn Type	pm+pt	NA		pm+pt	NA		Split	NA	Perm	Split	NA	
Protected Phases	5	2		1	6		4	4		7	7	
Permitted Phases	2			6					4			
Actuated Green, G (s)	117.0	114.2		136.9	127.5			15.6	15.6		8.9	
Effective Green, g (s)	119.0	116.2		137.9	129.5			17.6	17.6		10.9	
Actuated g/C Ratio	0.63	0.61		0.73	0.68			0.09	0.09		0.06	
Clearance Time (s)	6.6	6.6		6.6	6.6			6.5	6.5		6.5	
Vehicle Extension (s)	3.0	3.0		3.0	3.0			3.0	3.0		3.0	
Lane Grp Cap (vph)	423	2130		203	2320			167	149		101	
v/s Ratio Prot	0.00	c0.53		c0.06	0.21			c0.06			c0.02	
v/s Ratio Perm	0.01			0.44					0.01			
v/c Ratio	0.02	0.86		0.68	0.30			0.63	0.10		0.43	
Uniform Delay, d1	13.3	30.4		61.3	12.1			83.1	78.9		86.5	
Progression Factor	0.79	0.39		1.47	0.66			1.00	1.00		1.00	
Incremental Delay, d2	0.0	3.9		9.0	0.3			7.7	0.3		2.9	
Delay (s)	10.5	15.7		99.3	8.3			90.8	79.2		89.5	
Level of Service	B	B		F	A			F	E		F	
Approach Delay (s)		15.7			23.3			83.8			89.5	
Approach LOS		B			C			F			F	

Intersection Summary

HCM 2000 Control Delay	25.1	HCM 2000 Level of Service	C
HCM 2000 Volume to Capacity ratio	0.77		
Actuated Cycle Length (s)	190.0	Sum of lost time (s)	24.2
Intersection Capacity Utilization	75.9%	ICU Level of Service	D
Analysis Period (min)	15		
c Critical Lane Group			

HCM Signalized Intersection Capacity Analysis

3: Oak Street/Meredith Drive & Fairfax Boulevard

Existing PM School



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	23	835	42	91	1175	5	78	19	157	19	5	38
Future Volume (vph)	23	835	42	91	1175	5	78	19	157	19	5	38
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	5.6	4.6		5.6	4.6			4.5	4.5		4.5	
Lane Util. Factor	1.00	0.95		1.00	0.95			1.00	1.00		1.00	
Frt	1.00	0.99		1.00	1.00			1.00	0.85		0.92	
Flt Protected	0.95	1.00		0.95	1.00			0.96	1.00		0.98	
Satd. Flow (prot)	1597	3484		1805	3404			1827	1615		1716	
Flt Permitted	0.17	1.00		0.25	1.00			0.96	1.00		0.98	
Satd. Flow (perm)	285	3484		475	3404			1827	1615		1716	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	25	908	46	99	1277	5	85	21	171	21	5	41
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	157	0	26	0
Lane Group Flow (vph)	25	954	0	99	1282	0	0	106	14	0	41	0
Heavy Vehicles (%)	13%	3%	0%	0%	6%	0%	0%	0%	0%	0%	0%	0%
Turn Type	pm+pt	NA		pm+pt	NA		Split	NA	Perm	Split	NA	
Protected Phases	5	2		1	6		4	4		7	7	
Permitted Phases	2			6					4			
Actuated Green, G (s)	154.2	148.4		160.4	151.5			16.5	16.5		11.0	
Effective Green, g (s)	156.2	150.4		162.4	153.5			18.5	18.5		13.0	
Actuated g/C Ratio	0.71	0.68		0.74	0.70			0.08	0.08		0.06	
Clearance Time (s)	6.6	6.6		6.6	6.6			6.5	6.5		6.5	
Vehicle Extension (s)	3.0	3.0		3.0	3.0			3.0	3.0		3.0	
Lane Grp Cap (vph)	242	2381		410	2375			153	135		101	
v/s Ratio Prot	0.00	0.27		c0.01	c0.38			c0.06			c0.02	
v/s Ratio Perm	0.07			0.17					0.01			
v/c Ratio	0.10	0.40		0.24	0.54			0.69	0.11		0.40	
Uniform Delay, d1	11.9	15.2		9.8	16.1			98.0	93.1		99.8	
Progression Factor	0.31	0.25		0.63	0.41			1.00	1.00		1.00	
Incremental Delay, d2	0.2	0.5		0.3	0.8			12.7	0.3		2.6	
Delay (s)	3.9	4.3		6.5	7.5			110.7	93.5		102.4	
Level of Service	A	A		A	A			F	F		F	
Approach Delay (s)		4.3			7.4			100.1			102.4	
Approach LOS		A			A			F			F	

Intersection Summary

HCM 2000 Control Delay	18.1	HCM 2000 Level of Service	B
HCM 2000 Volume to Capacity ratio	0.53		
Actuated Cycle Length (s)	220.0	Sum of lost time (s)	24.2
Intersection Capacity Utilization	62.7%	ICU Level of Service	B
Analysis Period (min)	15		
c Critical Lane Group			

HCM Signalized Intersection Capacity Analysis

3: Oak Street/Meredith Drive & Fairfax Boulevard

Existing PM



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	14	789	18	76	1424	7	18	10	117	23	4	32
Future Volume (vph)	14	789	18	76	1424	7	18	10	117	23	4	32
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	5.6	4.6		5.6	4.6			4.5	4.5		4.5	
Lane Util. Factor	1.00	0.95		1.00	0.95			1.00	1.00		1.00	
Frt	1.00	1.00		1.00	1.00			1.00	0.85		0.93	
Flt Protected	0.95	1.00		0.95	1.00			0.97	1.00		0.98	
Satd. Flow (prot)	1597	3495		1805	3404			1841	1615		1726	
Flt Permitted	0.12	1.00		0.28	1.00			0.97	1.00		0.98	
Satd. Flow (perm)	205	3495		535	3404			1841	1615		1726	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	15	858	20	83	1548	8	20	11	127	25	4	35
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	120	0	21	0
Lane Group Flow (vph)	15	878	0	83	1556	0	0	31	7	0	43	0
Heavy Vehicles (%)	13%	3%	0%	0%	6%	0%	0%	0%	0%	0%	0%	0%
Turn Type	pm+pt	NA		pm+pt	NA		Split	NA	Perm	Split	NA	
Protected Phases	5	2		1	6		4	4		7	7	
Permitted Phases	2			6					4			
Actuated Green, G (s)	159.9	155.7		167.9	159.7			9.7	9.7		11.2	
Effective Green, g (s)	161.9	157.7		169.9	161.7			11.7	11.7		13.2	
Actuated g/C Ratio	0.74	0.72		0.77	0.73			0.05	0.05		0.06	
Clearance Time (s)	6.6	6.6		6.6	6.6			6.5	6.5		6.5	
Vehicle Extension (s)	3.0	3.0		3.0	3.0			3.0	3.0		3.0	
Lane Grp Cap (vph)	183	2505		466	2501			97	85		103	
v/s Ratio Prot	0.00	0.25		c0.01	c0.46			c0.02			c0.03	
v/s Ratio Perm	0.06			0.13					0.00			
v/c Ratio	0.08	0.35		0.18	0.62			0.32	0.08		0.42	
Uniform Delay, d1	11.6	11.8		7.1	14.2			100.3	99.0		99.7	
Progression Factor	0.33	0.25		0.94	0.51			1.00	1.00		1.00	
Incremental Delay, d2	0.2	0.4		0.2	1.0			1.9	0.4		2.8	
Delay (s)	4.0	3.3		6.9	8.3			102.2	99.4		102.5	
Level of Service	A	A		A	A			F	F		F	
Approach Delay (s)		3.3			8.3			100.0			102.5	
Approach LOS		A			A			F			F	

Intersection Summary

HCM 2000 Control Delay	14.1	HCM 2000 Level of Service	B
HCM 2000 Volume to Capacity ratio	0.57		
Actuated Cycle Length (s)	220.0	Sum of lost time (s)	24.2
Intersection Capacity Utilization	67.8%	ICU Level of Service	C
Analysis Period (min)	15		
c Critical Lane Group			

HCM Signalized Intersection Capacity Analysis

5: Fairfax Boulevard & Fairfax Shoppes Entrance

Existing AM



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (vph)	26	1708	731	9	11	18
Future Volume (vph)	26	1708	731	9	11	18
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	5.2	5.2	5.2		4.0	
Lane Util. Factor	1.00	0.95	0.95		1.00	
Frt	1.00	1.00	1.00		0.92	
Flt Protected	0.95	1.00	1.00		0.98	
Satd. Flow (prot)	1805	3505	3434		1708	
Flt Permitted	0.33	1.00	1.00		0.98	
Satd. Flow (perm)	619	3505	3434		1708	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	28	1857	795	10	12	20
RTOR Reduction (vph)	0	0	0	0	19	0
Lane Group Flow (vph)	28	1857	805	0	13	0
Heavy Vehicles (%)	0%	3%	5%	0%	0%	0%
Turn Type	pm+pt	NA	NA		Prot	
Protected Phases	1	6	2		4	
Permitted Phases	6					
Actuated Green, G (s)	167.4	167.4	155.6		10.4	
Effective Green, g (s)	168.4	168.4	156.6		12.4	
Actuated g/C Ratio	0.89	0.89	0.82		0.07	
Clearance Time (s)	6.2	6.2	6.2		6.0	
Vehicle Extension (s)	3.0	3.0	3.0		3.0	
Lane Grp Cap (vph)	589	3106	2830		111	
v/s Ratio Prot	0.00	c0.53	0.23		c0.01	
v/s Ratio Perm	0.04					
v/c Ratio	0.05	0.60	0.28		0.12	
Uniform Delay, d1	1.5	2.6	3.8		83.7	
Progression Factor	0.16	0.59	0.09		1.00	
Incremental Delay, d2	0.0	0.5	0.2		0.5	
Delay (s)	0.3	2.0	0.6		84.1	
Level of Service	A	A	A		F	
Approach Delay (s)		2.0	0.6		84.1	
Approach LOS		A	A		F	

Intersection Summary

HCM 2000 Control Delay	2.6	HCM 2000 Level of Service	A
HCM 2000 Volume to Capacity ratio	0.58		
Actuated Cycle Length (s)	190.0	Sum of lost time (s)	14.4
Intersection Capacity Utilization	63.2%	ICU Level of Service	B
Analysis Period (min)	15		
c Critical Lane Group			

HCM Signalized Intersection Capacity Analysis

5: Fairfax Boulevard & Fairfax Shoppes Entrance

Existing PM School



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (vph)	44	876	1160	29	57	57
Future Volume (vph)	44	876	1160	29	57	57
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	5.2	5.2	5.2		4.0	
Lane Util. Factor	1.00	0.95	0.95		1.00	
Frt	1.00	1.00	1.00		0.93	
Flt Protected	0.95	1.00	1.00		0.98	
Satd. Flow (prot)	1805	3505	3429		1729	
Flt Permitted	0.19	1.00	1.00		0.98	
Satd. Flow (perm)	356	3505	3429		1729	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	48	952	1261	32	62	62
RTOR Reduction (vph)	0	0	1	0	18	0
Lane Group Flow (vph)	48	952	1292	0	106	0
Heavy Vehicles (%)	0%	3%	5%	0%	0%	0%
Turn Type	pm+pt	NA	NA		Prot	
Protected Phases	1	6	2		4	
Permitted Phases	6					
Actuated Green, G (s)	189.0	189.0	174.0		18.8	
Effective Green, g (s)	190.0	190.0	175.0		20.8	
Actuated g/C Ratio	0.86	0.86	0.80		0.09	
Clearance Time (s)	6.2	6.2	6.2		6.0	
Vehicle Extension (s)	3.0	3.0	3.0		3.0	
Lane Grp Cap (vph)	372	3027	2727		163	
v/s Ratio Prot	0.01	c0.27	c0.38		c0.06	
v/s Ratio Perm	0.11					
v/c Ratio	0.13	0.31	0.47		0.65	
Uniform Delay, d1	7.0	2.8	7.4		96.1	
Progression Factor	0.20	0.26	0.10		1.00	
Incremental Delay, d2	0.1	0.3	0.5		8.6	
Delay (s)	1.5	1.0	1.2		104.7	
Level of Service	A	A	A		F	
Approach Delay (s)		1.0	1.2		104.7	
Approach LOS		A	A		F	

Intersection Summary

HCM 2000 Control Delay	6.4	HCM 2000 Level of Service	A
HCM 2000 Volume to Capacity ratio	0.49		
Actuated Cycle Length (s)	220.0	Sum of lost time (s)	14.4
Intersection Capacity Utilization	52.6%	ICU Level of Service	A
Analysis Period (min)	15		
c Critical Lane Group			

HCM Signalized Intersection Capacity Analysis

5: Fairfax Boulevard & Fairfax Shoppes Entrance

Existing PM



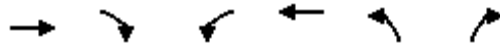
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (vph)	37	929	1383	29	64	31
Future Volume (vph)	37	929	1383	29	64	31
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	5.2	5.2	5.2		4.0	
Lane Util. Factor	1.00	0.95	0.95		1.00	
Frt	1.00	1.00	1.00		0.96	
Flt Protected	0.95	1.00	1.00		0.97	
Satd. Flow (prot)	1805	3505	3431		1757	
Flt Permitted	0.14	1.00	1.00		0.97	
Satd. Flow (perm)	268	3505	3431		1757	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	40	1010	1503	32	70	34
RTOR Reduction (vph)	0	0	0	0	9	0
Lane Group Flow (vph)	40	1010	1535	0	95	0
Heavy Vehicles (%)	0%	3%	5%	0%	0%	0%
Turn Type	pm+pt	NA	NA		Prot	
Protected Phases	1	6	2		4	
Permitted Phases	6					
Actuated Green, G (s)	190.5	190.5	177.3		17.3	
Effective Green, g (s)	191.5	191.5	178.3		19.3	
Actuated g/C Ratio	0.87	0.87	0.81		0.09	
Clearance Time (s)	6.2	6.2	6.2		6.0	
Vehicle Extension (s)	3.0	3.0	3.0		3.0	
Lane Grp Cap (vph)	289	3050	2780		154	
v/s Ratio Prot	0.01	c0.29	c0.45		c0.05	
v/s Ratio Perm	0.12					
v/c Ratio	0.14	0.33	0.55		0.62	
Uniform Delay, d1	8.5	2.6	7.2		96.8	
Progression Factor	0.32	0.40	0.01		1.00	
Incremental Delay, d2	0.2	0.3	0.4		7.1	
Delay (s)	2.9	1.3	0.5		103.9	
Level of Service	A	A	A		F	
Approach Delay (s)		1.4	0.5		103.9	
Approach LOS		A	A		F	

Intersection Summary			
HCM 2000 Control Delay	4.8	HCM 2000 Level of Service	A
HCM 2000 Volume to Capacity ratio	0.55		
Actuated Cycle Length (s)	220.0	Sum of lost time (s)	14.4
Intersection Capacity Utilization	55.2%	ICU Level of Service	B
Analysis Period (min)	15		
c Critical Lane Group			

HCM Unsignalized Intersection Capacity Analysis

6: Paul VI Entrance & Fairfax Boulevard

Existing AM

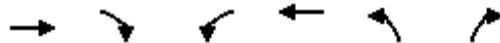


Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑		↵	↑↑	↵	
Traffic Volume (veh/h)	1697	22	27	748	5	14
Future Volume (Veh/h)	1697	22	27	748	5	14
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	1845	24	29	813	5	15
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	TWLTL		TWLTL			
Median storage (veh)	2		2			
Upstream signal (ft)	217		618			
pX, platoon unblocked			0.86		0.92	0.86
vC, conflicting volume			1869		2322	934
vC1, stage 1 conf vol					1857	
vC2, stage 2 conf vol					464	
vCu, unblocked vol			1683		1661	595
tC, single (s)			4.1		6.8	6.9
tC, 2 stage (s)					5.8	
tF (s)			2.2		3.5	3.3
p0 queue free %			91		96	96
cM capacity (veh/h)			323		116	384
Direction, Lane #	EB 1	EB 2	WB 1	WB 2	WB 3	NB 1
Volume Total	1230	639	29	406	406	20
Volume Left	0	0	29	0	0	5
Volume Right	0	24	0	0	0	15
cSH	1700	1700	323	1700	1700	243
Volume to Capacity	0.72	0.38	0.09	0.24	0.24	0.08
Queue Length 95th (ft)	0	0	7	0	0	7
Control Delay (s)	0.0	0.0	17.2	0.0	0.0	21.1
Lane LOS			C			C
Approach Delay (s)	0.0		0.6			21.1
Approach LOS						C
Intersection Summary						
Average Delay			0.3			
Intersection Capacity Utilization			57.6%	ICU Level of Service		B
Analysis Period (min)			15			

HCM Unsignalized Intersection Capacity Analysis

6: Paul VI Entrance & Fairfax Boulevard

Existing PM School



Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑		↵	↑↑	↵	
Traffic Volume (veh/h)	900	33	34	1216	16	75
Future Volume (Veh/h)	900	33	34	1216	16	75
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	978	36	37	1322	17	82
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	TWLTL		TWLTL			
Median storage (veh)	2		2			
Upstream signal (ft)	217		618			
pX, platoon unblocked			0.95		0.79	0.95
vC, conflicting volume			1014		1731	507
vC1, stage 1 conf vol					996	
vC2, stage 2 conf vol					735	
vCu, unblocked vol			907		1093	373
tC, single (s)			4.1		6.8	6.9
tC, 2 stage (s)					5.8	
tF (s)			2.2		3.5	3.3
p0 queue free %			95		95	86
cM capacity (veh/h)			708		321	593
Direction, Lane #	EB 1	EB 2	WB 1	WB 2	WB 3	NB 1
Volume Total	652	362	37	661	661	99
Volume Left	0	0	37	0	0	17
Volume Right	0	36	0	0	0	82
cSH	1700	1700	708	1700	1700	518
Volume to Capacity	0.38	0.21	0.05	0.39	0.39	0.19
Queue Length 95th (ft)	0	0	4	0	0	17
Control Delay (s)	0.0	0.0	10.4	0.0	0.0	13.6
Lane LOS			B			B
Approach Delay (s)	0.0		0.3			13.6
Approach LOS						B
Intersection Summary						
Average Delay			0.7			
Intersection Capacity Utilization			45.8%	ICU Level of Service	A	
Analysis Period (min)			15			

HCM Unsignalized Intersection Capacity Analysis

6: Paul VI Entrance & Fairfax Boulevard

Existing PM

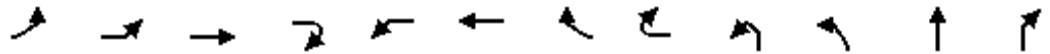


Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑		↵	↑↑	↵	
Traffic Volume (veh/h)	992	1	5	1509	1	4
Future Volume (Veh/h)	992	1	5	1509	1	4
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	1078	1	5	1640	1	4
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	TWLTL		TWLTL			
Median storage (veh)	2		2			
Upstream signal (ft)	217		618			
pX, platoon unblocked			0.95		0.63	0.95
vC, conflicting volume			1079		1908	540
vC1, stage 1 conf vol					1078	
vC2, stage 2 conf vol					830	
vCu, unblocked vol			972		875	402
tC, single (s)			4.1		6.8	6.9
tC, 2 stage (s)					5.8	
tF (s)			2.2		3.5	3.3
p0 queue free %			99		100	99
cM capacity (veh/h)			668		295	566
Direction, Lane #	EB 1	EB 2	WB 1	WB 2	WB 3	NB 1
Volume Total	719	360	5	820	820	5
Volume Left	0	0	5	0	0	1
Volume Right	0	1	0	0	0	4
cSH	1700	1700	668	1700	1700	478
Volume to Capacity	0.42	0.21	0.01	0.48	0.48	0.01
Queue Length 95th (ft)	0	0	1	0	0	1
Control Delay (s)	0.0	0.0	10.4	0.0	0.0	12.6
Lane LOS			B			B
Approach Delay (s)	0.0		0.0			12.6
Approach LOS						B
Intersection Summary						
Average Delay			0.0			
Intersection Capacity Utilization			51.7%	ICU Level of Service	A	
Analysis Period (min)			15			

HCM Signalized Intersection Capacity Analysis

8: Fairfax Boulevard & McLean Avenue & Warwick Road



















Existing AM



Movement	EBL2	EBL	EBT	EBR2	WBL	WBT	WBR	WBR2	NBL2	NBL	NBT	NBR
Lane Configurations			↕			↕					↕	
Traffic Volume (vph)	3	98	26	2	14	6	7	3	12	2	5	35
Future Volume (vph)	3	98	26	2	14	6	7	3	12	2	5	35
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)			5.0			5.0					5.0	
Lane Util. Factor			1.00			1.00					1.00	
Frt			1.00			0.95					0.91	
Flt Protected			0.96			0.98					0.99	
Satd. Flow (prot)			1477			1719					1710	
Flt Permitted			0.96			0.98					0.91	
Satd. Flow (perm)			1477			1719					1583	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	3	107	28	2	15	7	8	3	13	2	5	38
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	0	0	140	0	0	33	0	0	0	0	58	0
Heavy Vehicles (%)	100%	0%	100%	100%	7%	0%	0%	0%	0%	0%	0%	0%
Turn Type	Perm	Split	NA		Split	NA			Perm	Perm	NA	
Protected Phases		3	3		4	4					7	
Permitted Phases	3								7	7		
Actuated Green, G (s)			19.7			8.0					12.4	
Effective Green, g (s)			21.2			9.5					13.9	
Actuated g/C Ratio			0.11			0.05					0.07	
Clearance Time (s)			6.5			6.5					6.5	
Vehicle Extension (s)			3.0			3.0					3.0	
Lane Grp Cap (vph)			164			85					115	
v/s Ratio Prot			c0.09			c0.02						
v/s Ratio Perm											c0.04	
v/c Ratio			0.85			0.39					0.50	
Uniform Delay, d1			82.9			87.4					84.7	
Progression Factor			1.00			1.00					1.00	
Incremental Delay, d2			32.6			2.9					3.5	
Delay (s)			115.4			90.4					88.2	
Level of Service			F			F					F	
Approach Delay (s)			115.4			90.4					88.2	
Approach LOS			F			F					F	
Intersection Summary												
HCM 2000 Control Delay			28.5			HCM 2000 Level of Service			C			
HCM 2000 Volume to Capacity ratio			0.77									
Actuated Cycle Length (s)			190.0			Sum of lost time (s)			25.7			
Intersection Capacity Utilization			80.4%			ICU Level of Service			D			
Analysis Period (min)			15									
c Critical Lane Group												

HCM Signalized Intersection Capacity Analysis
 8: Fairfax Boulevard & McLean Avenue & Warwick Road

Existing AM

												
Movement	SBL2	SBL	SBT	SBR	SBR2	NEL	NET	NER	NER2	SWL2	SWL	SWT
Lane Configurations												
Traffic Volume (vph)	25	3	5	2	4	3	1645	26	7	1	16	745
Future Volume (vph)	25	3	5	2	4	3	1645	26	7	1	16	745
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		5.0	5.0			5.6	5.1				5.6	5.1
Lane Util. Factor		1.00	1.00			1.00	0.95				1.00	0.95
Frt		1.00	0.92			1.00	1.00				1.00	0.99
Flt Protected		0.95	1.00			0.95	1.00				0.95	1.00
Satd. Flow (prot)		1805	1745			1752	3599				1805	3482
Flt Permitted		0.61	1.00			0.27	1.00				0.03	1.00
Satd. Flow (perm)		1158	1745			500	3599				65	3482
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	27	3	5	2	4	3	1788	28	8	1	17	810
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	0	30	11	0	0	3	1824	0	0	0	18	886
Heavy Vehicles (%)	0%	0%	0%	0%	0%	3%	0%	0%	0%	0%	0%	2%
Turn Type	Perm	Perm	NA			pm+pt	NA			pm+pt	pm+pt	NA
Protected Phases			7			1	6			5	5	2
Permitted Phases	7	7				6				2	2	
Actuated Green, G (s)		12.4	12.4			113.4	112.2				119.0	115.0
Effective Green, g (s)		13.9	13.9			116.4	114.2				122.0	117.0
Actuated g/C Ratio		0.07	0.07			0.61	0.60				0.64	0.62
Clearance Time (s)		6.5	6.5			7.1	7.1				7.1	7.1
Vehicle Extension (s)		3.0	3.0			3.0	4.0				3.0	4.0
Lane Grp Cap (vph)		84	127			324	2163				92	2144
v/s Ratio Prot			0.01			0.00	c0.51				c0.01	c0.25
v/s Ratio Perm		0.03				0.01					0.12	
v/c Ratio		0.36	0.09			0.01	0.84				0.20	0.41
Uniform Delay, d1		83.8	82.1			15.1	30.7				32.5	18.8
Progression Factor		1.00	1.00			1.17	0.60				1.00	1.00
Incremental Delay, d2		2.6	0.3			0.0	3.5				1.0	0.6
Delay (s)		86.4	82.4			17.6	21.9				33.6	19.4
Level of Service		F	F			B	C				C	B
Approach Delay (s)			85.3				21.9					19.7
Approach LOS			F				C					B

Intersection Summary

HCM Signalized Intersection Capacity Analysis

8: Fairfax Boulevard & McLean Avenue & Warwick Road

Existing AM

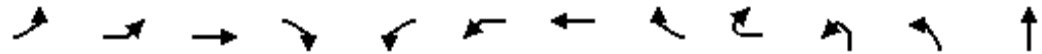


Movement	SWR	SWR2
Lane Configurations		
Traffic Volume (vph)	61	9
Future Volume (vph)	61	9
Ideal Flow (vphpl)	1900	1900
Total Lost time (s)		
Lane Util. Factor		
Fr		
Flt Protected		
Satd. Flow (prot)		
Flt Permitted		
Satd. Flow (perm)		
Peak-hour factor, PHF	0.92	0.92
Adj. Flow (vph)	66	10
RTOR Reduction (vph)	0	0
Lane Group Flow (vph)	0	0
Heavy Vehicles (%)	7%	0%
Turn Type		
Protected Phases		
Permitted Phases		
Actuated Green, G (s)		
Effective Green, g (s)		
Actuated g/C Ratio		
Clearance Time (s)		
Vehicle Extension (s)		
Lane Grp Cap (vph)		
v/s Ratio Prot		
v/s Ratio Perm		
v/c Ratio		
Uniform Delay, d1		
Progression Factor		
Incremental Delay, d2		
Delay (s)		
Level of Service		
Approach Delay (s)		
Approach LOS		
Intersection Summary		

HCM Signalized Intersection Capacity Analysis

8: Fairfax Boulevard & McLean Avenue & Warwick Road

Existing PM School



















Movement	EBL2	EBL	EBT	EBR	WBL2	WBL	WBT	WBR	WBR2	NBL2	NBL	NBT
Lane Configurations			↔				↔					↔
Traffic Volume (vph)	4	73	19	3	1	4	26	10	15	5	8	4
Future Volume (vph)	4	73	19	3	1	4	26	10	15	5	8	4
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)			5.0				5.0					5.0
Lane Util. Factor			1.00				1.00					1.00
Frt			1.00				0.94					0.95
Flt Protected			0.96				1.00					0.98
Satd. Flow (prot)			1444				1769					1756
Flt Permitted			0.96				1.00					0.84
Satd. Flow (perm)			1444				1769					1518
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	4	79	21	3	1	4	28	11	16	5	9	4
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	0	0	107	0	0	0	60	0	0	0	0	30
Heavy Vehicles (%)	100%	0%	100%	100%	0%	7%	0%	0%	0%	0%	0%	0%
Turn Type	Perm	Split	NA		Perm	Split	NA			Perm	Perm	NA
Protected Phases		3	3			4	4					7
Permitted Phases	3				4					7	7	
Actuated Green, G (s)			20.1				12.9					8.6
Effective Green, g (s)			21.6				14.4					10.1
Actuated g/C Ratio			0.10				0.07					0.05
Clearance Time (s)			6.5				6.5					6.5
Vehicle Extension (s)			3.0				3.0					3.0
Lane Grp Cap (vph)			141				115					69
v/s Ratio Prot			c0.07				c0.03					
v/s Ratio Perm												c0.02
v/c Ratio			0.76				0.52					0.43
Uniform Delay, d1			96.7				99.5					102.2
Progression Factor			1.00				1.00					1.00
Incremental Delay, d2			20.6				4.2					4.3
Delay (s)			117.3				103.7					106.5
Level of Service			F				F					F
Approach Delay (s)			117.3				103.7					106.5
Approach LOS			F				F					F
Intersection Summary												
HCM 2000 Control Delay			26.6				HCM 2000 Level of Service			C		
HCM 2000 Volume to Capacity ratio			0.61									
Actuated Cycle Length (s)			220.0				Sum of lost time (s)		25.7			
Intersection Capacity Utilization			69.7%				ICU Level of Service			C		
Analysis Period (min)			15									
c Critical Lane Group												

HCM Signalized Intersection Capacity Analysis

8: Fairfax Boulevard & McLean Avenue & Warwick Road

Existing PM School

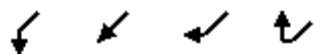
												
Movement	NBR	SBL2	SBL	SBT	SBR	SBR2	NEL2	NEL	NET	NER	NER2	SWL2
Lane Configurations												
Traffic Volume (vph)	11	17	7	6	5	1	5	5	888	51	11	3
Future Volume (vph)	11	17	7	6	5	1	5	5	888	51	11	3
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)			5.0	5.0				5.6	5.1			
Lane Util. Factor			1.00	1.00				1.00	0.95			
Frt			1.00	0.93				1.00	0.99			
Flt Protected			0.95	1.00				0.95	1.00			
Satd. Flow (prot)			1805	1768				1778	3575			
Flt Permitted			0.77	1.00				0.12	1.00			
Satd. Flow (perm)			1461	1768				232	3575			
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	12	18	8	7	5	1	5	5	965	55	12	3
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	0	0	26	13	0	0	0	10	1032	0	0	0
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	0%	3%	0%	0%	0%	0%
Turn Type		Perm	Perm	NA			custom	pm+pt	NA			pm+pt
Protected Phases				7				1	6			5
Permitted Phases		7	7				1	6				2
Actuated Green, G (s)			8.6	8.6				141.9	139.3			
Effective Green, g (s)			10.1	10.1				144.9	141.3			
Actuated g/C Ratio			0.05	0.05				0.66	0.64			
Clearance Time (s)			6.5	6.5				7.1	7.1			
Vehicle Extension (s)			3.0	3.0				3.0	4.0			
Lane Grp Cap (vph)			67	81				181	2296			
v/s Ratio Prot				0.01				0.00	0.29			
v/s Ratio Perm			0.02					0.04				
v/c Ratio			0.39	0.16				0.06	0.45			
Uniform Delay, d1			101.9	100.9				18.0	19.8			
Progression Factor			1.00	1.00				0.86	0.59			
Incremental Delay, d2			3.7	0.9				0.1	0.6			
Delay (s)			105.6	101.8				15.6	12.3			
Level of Service			F	F				B	B			
Approach Delay (s)				104.4					12.3			
Approach LOS				F					B			

Intersection Summary

HCM Signalized Intersection Capacity Analysis

8: Fairfax Boulevard & McLean Avenue & Warwick Road

Existing PM School



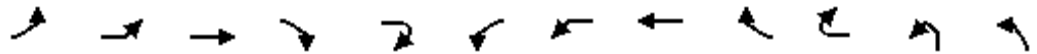
Movement	SWL	SWT	SWR	SWR2
Lane Configurations	↙	↑↑		
Traffic Volume (vph)	18	1236	68	14
Future Volume (vph)	18	1236	68	14
Ideal Flow (vphpl)	1900	1900	1900	1900
Total Lost time (s)	5.6	5.1		
Lane Util. Factor	1.00	0.95		
Frt	1.00	0.99		
Flt Protected	0.95	1.00		
Satd. Flow (prot)	1805	3498		
Flt Permitted	0.22	1.00		
Satd. Flow (perm)	411	3498		
Peak-hour factor, PHF	0.92	0.92	0.92	0.92
Adj. Flow (vph)	20	1343	74	15
RTOR Reduction (vph)	0	0	0	0
Lane Group Flow (vph)	23	1432	0	0
Heavy Vehicles (%)	0%	2%	7%	0%
Turn Type	pm+pt	NA		
Protected Phases	5	2		
Permitted Phases	2			
Actuated Green, G (s)	147.5	142.1		
Effective Green, g (s)	150.5	144.1		
Actuated g/C Ratio	0.68	0.65		
Clearance Time (s)	7.1	7.1		
Vehicle Extension (s)	3.0	4.0		
Lane Grp Cap (vph)	324	2291		
v/s Ratio Prot	c0.00	c0.41		
v/s Ratio Perm	0.05			
v/c Ratio	0.07	0.63		
Uniform Delay, d1	13.5	22.2		
Progression Factor	1.00	1.00		
Incremental Delay, d2	0.1	1.3		
Delay (s)	13.6	23.5		
Level of Service	B	C		
Approach Delay (s)		23.3		
Approach LOS		C		

Intersection Summary

HCM Signalized Intersection Capacity Analysis

8: Fairfax Boulevard & McLean Avenue & Warwick Road

Existing PM



Movement	EBL2	EBL	EBT	EBR	EBR2	WBL2	WBL	WBT	WBR	WBR2	NBL2	NBL
Lane Configurations			↔					↔				
Traffic Volume (vph)	5	61	31	4	2	1	4	30	12	5	18	16
Future Volume (vph)	5	61	31	4	2	1	4	30	12	5	18	16
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)			5.0					5.0				
Lane Util. Factor			1.00					1.00				
Frt			0.99					0.96				
Flt Protected			0.97					1.00				
Satd. Flow (prot)			1300					1800				
Flt Permitted			0.97					1.00				
Satd. Flow (perm)			1300					1800				
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	5	66	34	4	2	1	4	33	13	5	20	17
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	0	0	111	0	0	0	0	56	0	0	0	0
Heavy Vehicles (%)	100%	0%	100%	100%	100%	0%	7%	0%	0%	0%	0%	0%
Turn Type	Perm	Split	NA			Perm	Split	NA			Perm	Perm
Protected Phases		3	3				4	4				
Permitted Phases	3					4					7	7
Actuated Green, G (s)			21.5					12.3				
Effective Green, g (s)			23.0					13.8				
Actuated g/C Ratio			0.10					0.06				
Clearance Time (s)			6.5					6.5				
Vehicle Extension (s)			3.0					3.0				
Lane Grp Cap (vph)			135					112				
v/s Ratio Prot			c0.09					c0.03				
v/s Ratio Perm												
v/c Ratio			0.82					0.50				
Uniform Delay, d1			96.5					99.8				
Progression Factor			1.00					1.00				
Incremental Delay, d2			31.5					3.5				
Delay (s)			128.0					103.2				
Level of Service			F					F				
Approach Delay (s)			128.0					103.2				
Approach LOS			F					F				
Intersection Summary												
HCM 2000 Control Delay			37.9					HCM 2000 Level of Service			D	
HCM 2000 Volume to Capacity ratio			0.77									
Actuated Cycle Length (s)			220.0					Sum of lost time (s)		25.7		
Intersection Capacity Utilization			81.1%					ICU Level of Service		D		
Analysis Period (min)			15									
c Critical Lane Group												

HCM Signalized Intersection Capacity Analysis

8: Fairfax Boulevard & McLean Avenue & Warwick Road

Existing PM

	↑	↗	↘	↓	↙	↘	↗	↗	↘	↗	↘	↘
Movement	NBT	NBR	SBL2	SBL	SBT	SBR	SBR2	NEL2	NEL	NET	NER	NER2
Lane Configurations	↕			↖	↗				↖	↕		
Traffic Volume (vph)	20	38	11	1	6	2	2	12	12	797	33	9
Future Volume (vph)	20	38	11	1	6	2	2	12	12	797	33	9
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	5.0			5.0	5.0				5.6	5.1		
Lane Util. Factor	1.00			1.00	1.00				1.00	0.95		
Frt	0.94			1.00	0.95				1.00	0.99		
Flt Protected	0.98			0.95	1.00				0.95	1.00		
Satd. Flow (prot)	1762			1805	1796				1778	3583		
Flt Permitted	0.87			0.49	1.00				0.05	1.00		
Satd. Flow (perm)	1568			934	1796				90	3583		
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	22	41	12	1	7	2	2	13	13	866	36	10
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	100	0	0	13	11	0	0	0	26	912	0	0
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	0%	0%	3%	0%	0%	0%
Turn Type	NA		Perm	Perm	NA			custom	pm+pt	NA		
Protected Phases	7				7				1	6		
Permitted Phases			7	7				1	6			
Actuated Green, G (s)	17.8			17.8	17.8				134.8	129.2		
Effective Green, g (s)	19.3			19.3	19.3				137.8	131.2		
Actuated g/C Ratio	0.09			0.09	0.09				0.63	0.60		
Clearance Time (s)	6.5			6.5	6.5				7.1	7.1		
Vehicle Extension (s)	3.0			3.0	3.0				3.0	4.0		
Lane Grp Cap (vph)	137			81	157				110	2136		
v/s Ratio Prot					0.01				c0.01	0.25		
v/s Ratio Perm	c0.06			0.01					0.14			
v/c Ratio	0.73			0.16	0.07				0.24	0.43		
Uniform Delay, d1	97.8			92.9	92.1				34.0	24.0		
Progression Factor	1.00			1.00	1.00				0.70	0.44		
Incremental Delay, d2	17.6			0.9	0.2				1.1	0.6		
Delay (s)	115.4			93.8	92.3				24.9	11.1		
Level of Service	F			F	F				C	B		
Approach Delay (s)	115.4				93.1					11.5		
Approach LOS	F				F					B		
Intersection Summary												

HCM Signalized Intersection Capacity Analysis

8: Fairfax Boulevard & McLean Avenue & Warwick Road

Existing PM



Movement	SWL2	SWL	SWT	SWR	SWR2
Lane Configurations					
Traffic Volume (vph)	5	17	1488	93	5
Future Volume (vph)	5	17	1488	93	5
Ideal Flow (vphpl)	1900	1900	1900	1900	1900
Total Lost time (s)		5.6	5.1		
Lane Util. Factor		1.00	0.95		
Frt		1.00	0.99		
Flt Protected		0.95	1.00		
Satd. Flow (prot)		1805	3497		
Flt Permitted		0.25	1.00		
Satd. Flow (perm)		471	3497		
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	5	18	1617	101	5
RTOR Reduction (vph)	0	0	0	0	0
Lane Group Flow (vph)	0	23	1723	0	0
Heavy Vehicles (%)	0%	0%	2%	7%	0%
Turn Type	pm+pt	pm+pt	NA		
Protected Phases	5	5	2		
Permitted Phases	2	2			
Actuated Green, G (s)		134.6	129.1		
Effective Green, g (s)		137.6	131.1		
Actuated g/C Ratio		0.63	0.60		
Clearance Time (s)		7.1	7.1		
Vehicle Extension (s)		3.0	4.0		
Lane Grp Cap (vph)		337	2083		
v/s Ratio Prot		0.00	c0.49		
v/s Ratio Perm		0.04			
v/c Ratio		0.07	0.83		
Uniform Delay, d1		17.4	35.4		
Progression Factor		1.00	1.00		
Incremental Delay, d2		0.1	3.9		
Delay (s)		17.5	39.4		
Level of Service		B	D		
Approach Delay (s)			39.1		
Approach LOS			D		
Intersection Summary					

HCM Unsignalized Intersection Capacity Analysis

9: Walnut Street & Cedar Avenue

Existing AM



Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	22	53	87	146	85	53
Future Volume (Veh/h)	22	53	87	146	85	53
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	24	58	95	159	92	58
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)	3					
Median type			None			None
Median storage (veh)						
Upstream signal (ft)	366					
pX, platoon unblocked	0.99					
vC, conflicting volume	416	174			254	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	400	174			254	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)						
tF (s)	3.5	3.3			2.2	
p0 queue free %	96	93			93	
cM capacity (veh/h)	555	869			1311	
Direction, Lane #	WB 1	NB 1	SB 1			
Volume Total	82	254	150			
Volume Left	24	0	92			
Volume Right	58	159	0			
cSH	1228	1700	1311			
Volume to Capacity	0.07	0.15	0.07			
Queue Length 95th (ft)	5	0	6			
Control Delay (s)	10.1	0.0	5.1			
Lane LOS	B		A			
Approach Delay (s)	10.1	0.0	5.1			
Approach LOS	B					
Intersection Summary						
Average Delay			3.3			
Intersection Capacity Utilization			34.4%		ICU Level of Service	A
Analysis Period (min)			15			

HCM Unsignalized Intersection Capacity Analysis

9: Walnut Street & Cedar Avenue

Existing PM School



Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	48	117	60	31	21	54
Future Volume (Veh/h)	48	117	60	31	21	54
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	52	127	65	34	23	59
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)		3				
Median type			None			None
Median storage (veh)						
Upstream signal (ft)						366
pX, platoon unblocked						
vC, conflicting volume	187	82			99	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	187	82			99	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)						
tF (s)	3.5	3.3			2.2	
p0 queue free %	93	87			98	
cM capacity (veh/h)	790	978			1494	
Direction, Lane #	WB 1	NB 1	SB 1			
Volume Total	179	99	82			
Volume Left	52	0	23			
Volume Right	127	34	0			
cSH	1378	1700	1494			
Volume to Capacity	0.13	0.06	0.02			
Queue Length 95th (ft)	11	0	1			
Control Delay (s)	9.4	0.0	2.2			
Lane LOS	A		A			
Approach Delay (s)	9.4	0.0	2.2			
Approach LOS	A					
Intersection Summary						
Average Delay			5.2			
Intersection Capacity Utilization		20.7%		ICU Level of Service		A
Analysis Period (min)			15			

HCM Unsignalized Intersection Capacity Analysis

9: Walnut Street & Cedar Avenue

















Existing PM



Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	34	29	73	15	22	81
Future Volume (Veh/h)	34	29	73	15	22	81
Sign Control	Stop		Free		Free	
Grade	0%		0%		0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	37	32	79	16	24	88
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)	3					
Median type			None		None	
Median storage (veh)						
Upstream signal (ft)						366
pX, platoon unblocked						
vC, conflicting volume	223	87			95	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	223	87			95	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)						
tF (s)	3.5	3.3			2.2	
p0 queue free %	95	97			98	
cM capacity (veh/h)	753	971			1499	
Direction, Lane #	WB 1	NB 1	SB 1			
Volume Total	69	95	112			
Volume Left	37	0	24			
Volume Right	32	16	0			
cSH	1404	1700	1499			
Volume to Capacity	0.05	0.06	0.02			
Queue Length 95th (ft)	4	0	1			
Control Delay (s)	9.5	0.0	1.7			
Lane LOS	A		A			
Approach Delay (s)	9.5	0.0	1.7			
Approach LOS	A					
Intersection Summary						
Average Delay			3.1			
Intersection Capacity Utilization			22.1%	ICU Level of Service		A
Analysis Period (min)	15					

















HCM Unsignalized Intersection Capacity Analysis
 10: Oak Street & Cedar Avenue

Existing AM

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Sign Control		Stop			Stop			Stop			Stop	
Traffic Volume (vph)	11	214	4	58	72	189	2	22	205	213	22	6
Future Volume (vph)	11	214	4	58	72	189	2	22	205	213	22	6
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	12	233	4	63	78	205	2	24	223	232	24	7
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total (vph)	249	346	249	263								
Volume Left (vph)	12	63	2	232								
Volume Right (vph)	4	205	223	7								
Hadj (s)	0.03	-0.29	-0.50	0.19								
Departure Headway (s)	6.3	5.8	5.8	6.4								
Degree Utilization, x	0.43	0.55	0.40	0.47								
Capacity (veh/h)	509	570	544	498								
Control Delay (s)	14.0	15.7	12.7	15.0								
Approach Delay (s)	14.0	15.7	12.7	15.0								
Approach LOS	B	C	B	C								
Intersection Summary												
Delay			14.5									
Level of Service			B									
Intersection Capacity Utilization			71.3%	ICU Level of Service	C							
Analysis Period (min)			15									

HCM Unsignalized Intersection Capacity Analysis
 10: Oak Street & Cedar Avenue

















Existing PM School

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Sign Control		Stop			Stop			Stop			Stop	
Traffic Volume (vph)	11	39	3	72	125	111	5	83	32	45	79	13
Future Volume (vph)	11	39	3	72	125	111	5	83	32	45	79	13
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	12	42	3	78	136	121	5	90	35	49	86	14
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total (vph)	57	335	130	149								
Volume Left (vph)	12	78	5	49								
Volume Right (vph)	3	121	35	14								
Hadj (s)	0.04	-0.14	-0.12	0.04								
Departure Headway (s)	5.1	4.6	5.0	5.1								
Degree Utilization, x	0.08	0.42	0.18	0.21								
Capacity (veh/h)	639	749	666	650								
Control Delay (s)	8.5	10.9	9.0	9.4								
Approach Delay (s)	8.5	10.9	9.0	9.4								
Approach LOS	A	B	A	A								
Intersection Summary												
Delay			10.0									
Level of Service			B									
Intersection Capacity Utilization			44.8%	ICU Level of Service								A
Analysis Period (min)			15									

HCM Unsignalized Intersection Capacity Analysis

10: Oak Street & Cedar Avenue

Existing PM

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Sign Control		Stop			Stop			Stop			Stop	
Traffic Volume (vph)	9	16	4	24	35	36	8	83	13	22	78	23
Future Volume (vph)	9	16	4	24	35	36	8	83	13	22	78	23
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	10	17	4	26	38	39	9	90	14	24	85	25
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total (vph)	31	103	113	134								
Volume Left (vph)	10	26	9	24								
Volume Right (vph)	4	39	14	25								
Hadj (s)	0.02	-0.14	-0.02	-0.04								
Departure Headway (s)	4.6	4.4	4.3	4.3								
Degree Utilization, x	0.04	0.12	0.14	0.16								
Capacity (veh/h)	724	773	795	799								
Control Delay (s)	7.8	8.0	8.0	8.1								
Approach Delay (s)	7.8	8.0	8.0	8.1								
Approach LOS	A	A	A	A								
Intersection Summary												
Delay			8.0									
Level of Service			A									
Intersection Capacity Utilization			25.4%	ICU Level of Service	A							
Analysis Period (min)			15									

HCM Unsignalized Intersection Capacity Analysis

11: Cedar Avenue & McLean Avenue

Existing AM



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↔	↔		↔	
Sign Control		Stop	Stop		Stop	
Traffic Volume (vph)	61	27	28	22	9	20
Future Volume (vph)	61	27	28	22	9	20
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	66	29	30	24	10	22

Direction, Lane #	EB 1	WB 1	SB 1
Volume Total (vph)	95	54	32
Volume Left (vph)	66	0	10
Volume Right (vph)	0	24	22
Hadj (s)	0.17	-0.23	-0.32
Departure Headway (s)	4.2	3.8	3.9
Degree Utilization, x	0.11	0.06	0.03
Capacity (veh/h)	846	924	881
Control Delay (s)	7.7	7.1	7.0
Approach Delay (s)	7.7	7.1	7.0
Approach LOS	A	A	A

Intersection Summary			
Delay		7.4	
Level of Service		A	
Intersection Capacity Utilization	21.5%		ICU Level of Service A
Analysis Period (min)		15	

HCM Unsignalized Intersection Capacity Analysis

11: Cedar Avenue & McLean Avenue

Existing PM School



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↔	↔		↔	
Sign Control		Stop	Stop		Stop	
Traffic Volume (vph)	22	9	16	19	13	27
Future Volume (vph)	22	9	16	19	13	27
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	24	10	17	21	14	29

Direction, Lane #	EB 1	WB 1	SB 1
Volume Total (vph)	34	38	43
Volume Left (vph)	24	0	14
Volume Right (vph)	0	21	29
Hadj (s)	0.18	-0.30	-0.31
Departure Headway (s)	4.2	3.7	3.7
Degree Utilization, x	0.04	0.04	0.04
Capacity (veh/h)	842	949	932
Control Delay (s)	7.4	6.9	6.9
Approach Delay (s)	7.4	6.9	6.9
Approach LOS	A	A	A

Intersection Summary			
Delay		7.0	
Level of Service		A	
Intersection Capacity Utilization	18.4%		ICU Level of Service A
Analysis Period (min)		15	

HCM Unsignalized Intersection Capacity Analysis

11: Cedar Avenue & McLean Avenue

Existing PM



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↖	↗		↘	
Sign Control		Stop	Stop		Stop	
Traffic Volume (vph)	68	24	28	20	7	23
Future Volume (vph)	68	24	28	20	7	23
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	74	26	30	22	8	25

Direction, Lane #	EB 1	WB 1	SB 1
Volume Total (vph)	100	52	33
Volume Left (vph)	74	0	8
Volume Right (vph)	0	22	25
Hadj (s)	0.18	-0.22	-0.37
Departure Headway (s)	4.2	3.8	3.9
Degree Utilization, x	0.12	0.06	0.04
Capacity (veh/h)	844	919	891
Control Delay (s)	7.7	7.1	7.0
Approach Delay (s)	7.7	7.1	7.0
Approach LOS	A	A	A

Intersection Summary			
Delay		7.4	
Level of Service		A	
Intersection Capacity Utilization		21.7%	ICU Level of Service
Analysis Period (min)		15	A

APPENDIX D

2027 Background Future Capacity Analysis Worksheets

HCM Signalized Intersection Capacity Analysis

1: Lee Highway & Fairfax Boulevard & Main Street

Background AM



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖↗	↖↗		↖↗	↖↗	↖	↖	↖↗	↖	↖	↖↗	↖
Traffic Volume (vph)	506	890	17	346	724	36	9	986	733	43	368	246
Future Volume (vph)	506	890	17	346	724	36	9	986	733	43	368	246
Ideal Flow (vphp)	2500	2500	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	5.1	5.1		4.3	4.3	6.3	4.8	5.6	5.6	4.8	5.6	5.6
Lane Util. Factor	0.97	0.95		0.97	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00
Frt	1.00	1.00		1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85
Flt Protected	0.95	1.00		0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)	4430	4558		3335	3539	1509	1805	3539	1568	1703	3343	1524
Flt Permitted	0.95	1.00		0.95	1.00	1.00	0.41	1.00	1.00	0.07	1.00	1.00
Satd. Flow (perm)	4430	4558		3335	3539	1509	788	3539	1568	121	3343	1524
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	550	967	18	376	787	39	10	1072	797	47	400	267
RTOR Reduction (vph)	0	1	0	0	0	31	0	0	0	0	0	61
Lane Group Flow (vph)	550	984	0	376	787	8	10	1072	797	47	400	206
Heavy Vehicles (%)	4%	4%	0%	5%	2%	7%	0%	2%	3%	6%	8%	6%
Turn Type	Prot	NA		Prot	NA	Perm	pm+pt	NA	pt+ov	pm+pt	NA	pt+ov
Protected Phases	5	2		1	6		3	8	8	1	4	4
Permitted Phases						6	8			4		
Actuated Green, G (s)	59.9	59.9		37.8	37.8	37.8	64.5	57.4	95.2	64.5	57.4	124.4
Effective Green, g (s)	61.9	61.9		39.8	39.8	37.8	68.5	59.4	99.2	68.5	59.4	121.3
Actuated g/C Ratio	0.33	0.33		0.21	0.21	0.20	0.36	0.31	0.52	0.36	0.31	0.64
Clearance Time (s)	7.1	7.1		6.3	6.3	6.3	6.8	7.6		6.8	7.6	
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0		3.0	3.0	
Lane Grp Cap (vph)	1443	1484		698	741	300	332	1106	818	119	1045	972
v/s Ratio Prot	0.12	c0.22		0.11	c0.22		0.00	0.30	c0.51	c0.02	0.12	0.14
v/s Ratio Perm						0.01	0.01			0.12		
v/c Ratio	0.38	0.66		0.54	1.06	0.03	0.03	0.97	0.97	0.39	0.38	0.21
Uniform Delay, d1	49.3	55.1		66.9	75.1	61.3	47.2	64.4	44.2	83.6	51.0	14.4
Progression Factor	1.00	1.00		1.00	1.00	1.00	1.00	1.00	1.00	1.15	1.22	2.70
Incremental Delay, d2	0.8	2.4		3.0	50.8	0.2	0.0	20.6	25.8	2.1	1.0	0.5
Delay (s)	50.1	57.4		69.9	125.9	61.4	47.2	85.0	70.0	98.4	63.1	39.3
Level of Service	D	E		E	F	E	D	F	E	F	E	D
Approach Delay (s)		54.8			106.3			78.4			56.5	
Approach LOS		D			F			E			E	

Intersection Summary

HCM 2000 Control Delay	75.0	HCM 2000 Level of Service	E
HCM 2000 Volume to Capacity ratio	0.85		
Actuated Cycle Length (s)	190.0	Sum of lost time (s)	19.8
Intersection Capacity Utilization	83.2%	ICU Level of Service	E
Analysis Period (min)	15		
c Critical Lane Group			

HCM Signalized Intersection Capacity Analysis

1: Lee Highway & Fairfax Boulevard & Main Street

Background PM School



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	465	555	3	620	887	53	30	807	459	72	656	417
Future Volume (vph)	465	555	3	620	887	53	30	807	459	72	656	417
Ideal Flow (vphpl)	2500	2500	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	5.1	5.1		4.3	4.3	6.3	4.8	5.6	5.6	4.8	5.6	5.6
Lane Util. Factor	0.97	0.95		0.97	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00
Frt	1.00	1.00		1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85
Flt Protected	0.95	1.00		0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)	4430	4565		3335	3539	1509	1805	3539	1568	1703	3343	1524
Flt Permitted	0.95	1.00		0.95	1.00	1.00	0.23	1.00	1.00	0.15	1.00	1.00
Satd. Flow (perm)	4430	4565		3335	3539	1509	443	3539	1568	273	3343	1524
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	505	603	3	674	964	58	33	877	499	78	713	453
RTOR Reduction (vph)	0	0	0	0	0	40	0	0	0	0	0	42
Lane Group Flow (vph)	505	606	0	674	964	18	33	877	499	78	713	411
Heavy Vehicles (%)	4%	4%	0%	5%	2%	7%	0%	2%	3%	6%	8%	6%
Turn Type	Prot	NA		Prot	NA	Perm	pm+pt	NA	pt+ov	pm+pt	NA	pt+ov
Protected Phases	5	2		1	6		3	8	8	1	4	4
Permitted Phases						6	8			4		
Actuated Green, G (s)	37.9	37.9		68.9	68.9	68.9	85.4	78.4	147.3	85.4	78.4	123.4
Effective Green, g (s)	39.9	39.9		70.9	70.9	68.9	89.4	80.4	151.3	89.4	80.4	120.3
Actuated g/C Ratio	0.18	0.18		0.32	0.32	0.31	0.41	0.37	0.69	0.41	0.37	0.55
Clearance Time (s)	7.1	7.1		6.3	6.3	6.3	6.8	7.6		6.8	7.6	
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0		3.0	3.0	
Lane Grp Cap (vph)	803	827		1074	1140	472	235	1293	1078	169	1221	833
v/s Ratio Prot	0.11	c0.13		0.20	c0.27		0.01	c0.25	0.32	c0.02	0.21	0.27
v/s Ratio Perm						0.01	0.05			0.17		
v/c Ratio	0.63	0.73		0.63	0.85	0.04	0.14	0.68	0.46	0.46	0.58	0.49
Uniform Delay, d1	83.2	85.0		63.3	69.5	52.5	62.3	58.9	15.7	78.7	56.3	30.9
Progression Factor	1.00	1.00		1.00	1.00	1.00	1.00	1.00	1.00	0.92	0.98	1.15
Incremental Delay, d2	3.7	5.7		2.8	7.8	0.2	0.3	2.9	1.4	1.8	1.9	1.9
Delay (s)	86.9	90.7		66.1	77.2	52.7	62.6	61.8	17.2	74.6	57.1	37.5
Level of Service	F	F		E	E	D	E	E	B	E	E	D
Approach Delay (s)		89.0			72.0			46.0			51.1	
Approach LOS		F			E			D			D	






























Intersection Summary

HCM 2000 Control Delay	64.0	HCM 2000 Level of Service	E
HCM 2000 Volume to Capacity ratio	0.74		
Actuated Cycle Length (s)	220.0	Sum of lost time (s)	19.8
Intersection Capacity Utilization	79.2%	ICU Level of Service	D
Analysis Period (min)	15		
c Critical Lane Group			

HCM Signalized Intersection Capacity Analysis

1: Lee Highway & Fairfax Boulevard & Main Street

Background PM

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	 	 		 	 			 			 	
Traffic Volume (vph)	417	603	74	640	1018	36	34	606	528	49	960	482
Future Volume (vph)	417	603	74	640	1018	36	34	606	528	49	960	482
Ideal Flow (vphp)	2500	2500	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	5.1	5.1		4.3	4.3	6.3	4.8	5.6	5.6	4.8	5.6	5.6
Lane Util. Factor	0.97	0.95		0.97	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00
Frt	1.00	0.98		1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85
Flt Protected	0.95	1.00		0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)	4430	4512		3335	3539	1509	1805	3539	1568	1703	3343	1524
Flt Permitted	0.95	1.00		0.95	1.00	1.00	0.08	1.00	1.00	0.26	1.00	1.00
Satd. Flow (perm)	4430	4512		3335	3539	1509	161	3539	1568	471	3343	1524
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	453	655	80	696	1107	39	37	659	574	53	1043	524
RTOR Reduction (vph)	0	4	0	0	0	27	0	0	0	0	0	29
Lane Group Flow (vph)	453	731	0	696	1107	12	37	659	574	53	1043	495
Heavy Vehicles (%)	4%	4%	0%	5%	2%	7%	0%	2%	3%	6%	8%	6%
Turn Type	Prot	NA		Prot	NA	Perm	pm+pt	NA	pt+ov	pm+pt	NA	pt+ov
Protected Phases	5	2		1	6		3	8	8	1	4	4
Permitted Phases						6	8			4		
Actuated Green, G (s)	37.9	37.9		68.9	68.9	68.9	85.4	78.4	147.3	85.4	78.4	123.4
Effective Green, g (s)	39.9	39.9		70.9	70.9	68.9	89.4	80.4	151.3	89.4	80.4	120.3
Actuated g/C Ratio	0.18	0.18		0.32	0.32	0.31	0.41	0.37	0.69	0.41	0.37	0.55
Clearance Time (s)	7.1	7.1		6.3	6.3	6.3	6.8	7.6		6.8	7.6	
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0		3.0	3.0	
Lane Grp Cap (vph)	803	818		1074	1140	472	132	1293	1078	241	1221	833
v/s Ratio Prot	0.10	c0.16		0.21	c0.31		c0.01	0.19	0.37	0.01	c0.31	0.32
v/s Ratio Perm						0.01	0.10			0.08		
v/c Ratio	0.56	0.89		0.65	0.97	0.03	0.28	0.51	0.53	0.22	0.85	0.59
Uniform Delay, d1	82.1	88.0		63.9	73.5	52.3	84.9	54.4	16.9	61.5	64.4	33.5
Progression Factor	1.00	1.00		1.00	1.00	1.00	1.00	1.00	1.00	1.22	1.17	1.48
Incremental Delay, d2	2.9	14.2		3.0	20.5	0.1	1.2	1.4	1.9	0.4	6.8	2.7
Delay (s)	85.0	102.2		66.9	94.1	52.4	86.0	55.9	18.8	75.5	82.3	52.3
Level of Service	F	F		E	F	D	F	E	B	E	F	D
Approach Delay (s)		95.6			82.9			40.0			72.4	
Approach LOS		F			F			D			E	
Intersection Summary												
HCM 2000 Control Delay			73.4	HCM 2000 Level of Service				E				
HCM 2000 Volume to Capacity ratio			0.88									
Actuated Cycle Length (s)			220.0	Sum of lost time (s)				19.8				
Intersection Capacity Utilization			86.1%	ICU Level of Service				E				
Analysis Period (min)			15									
c Critical Lane Group												

HCM Signalized Intersection Capacity Analysis

2: Walnut Street/Fairchester Drive & Fairfax Boulevard

Background AM



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	15	1680	34	32	636	21	46	27	71	28	44	19
Future Volume (vph)	15	1680	34	32	636	21	46	27	71	28	44	19
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	5.6	5.6		5.6	5.6		4.5	4.5		4.5	4.5	
Lane Util. Factor	1.00	0.95		1.00	0.95		1.00	1.00		1.00	1.00	
Frt	1.00	1.00		1.00	1.00		1.00	0.89		1.00	0.95	
Flt Protected	0.95	1.00		0.95	1.00		0.95	1.00		0.95	1.00	
Satd. Flow (prot)	1671	3496		1805	3395		1805	1538		1752	1776	
Flt Permitted	0.37	1.00		0.08	1.00		0.64	1.00		0.41	1.00	
Satd. Flow (perm)	648	3496		146	3395		1211	1538		750	1776	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	16	1826	37	35	691	23	50	29	77	30	48	21
RTOR Reduction (vph)	0	0	0	0	1	0	0	61	0	0	9	0
Lane Group Flow (vph)	16	1863	0	35	713	0	50	45	0	30	60	0
Heavy Vehicles (%)	8%	3%	0%	0%	6%	0%	0%	5%	12%	3%	3%	0%
Turn Type	pm+pt	NA		pm+pt	NA		Perm	NA		Perm	NA	
Protected Phases	5	2		1	6			7				3
Permitted Phases	2			6			7			3		
Actuated Green, G (s)	146.8	142.6		150.2	144.3		21.8	21.8		12.4	12.4	
Effective Green, g (s)	148.8	143.6		152.2	145.3		23.8	23.8		14.4	14.4	
Actuated g/C Ratio	0.78	0.76		0.80	0.76		0.13	0.13		0.08	0.08	
Clearance Time (s)	6.6	6.6		6.6	6.6		6.5	6.5		6.5	6.5	
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Lane Grp Cap (vph)	535	2642		177	2596		151	192		56	134	
v/s Ratio Prot	0.00	c0.53		c0.01	0.21			0.03			0.03	
v/s Ratio Perm	0.02			0.15			c0.04			c0.04		
v/c Ratio	0.03	0.70		0.20	0.27		0.33	0.23		0.54	0.45	
Uniform Delay, d1	4.6	12.1		12.1	6.7		75.8	74.9		84.6	84.0	
Progression Factor	0.35	0.65		1.11	0.71		1.00	1.00		1.00	1.00	
Incremental Delay, d2	0.0	1.2		0.5	0.3		1.3	0.6		9.5	2.4	
Delay (s)	1.6	9.1		14.0	5.0		77.1	75.5		94.1	86.3	
Level of Service	A	A		B	A		E	E		F	F	
Approach Delay (s)		9.0			5.4			76.0			88.7	
Approach LOS		A			A			E			F	

Intersection Summary

HCM 2000 Control Delay	14.4	HCM 2000 Level of Service	B
HCM 2000 Volume to Capacity ratio	0.67		
Actuated Cycle Length (s)	190.0	Sum of lost time (s)	20.7
Intersection Capacity Utilization	65.2%	ICU Level of Service	C
Analysis Period (min)	15		
c Critical Lane Group			

HCM Signalized Intersection Capacity Analysis

2: Walnut Street/Fairchester Drive & Fairfax Boulevard

Background PM School



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	19	929	21	47	1071	19	107	54	45	44	28	12
Future Volume (vph)	19	929	21	47	1071	19	107	54	45	44	28	12
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	5.6	5.6		5.6	5.6		4.5	4.5		4.5	4.5	
Lane Util. Factor	1.00	0.95		1.00	0.95		1.00	1.00		1.00	1.00	
Frt	1.00	1.00		1.00	1.00		1.00	0.93		1.00	0.95	
Flt Protected	0.95	1.00		0.95	1.00		0.95	1.00		0.95	1.00	
Satd. Flow (prot)	1671	3495		1805	3400		1805	1637		1752	1777	
Flt Permitted	0.21	1.00		0.24	1.00		0.72	1.00		0.47	1.00	
Satd. Flow (perm)	367	3495		456	3400		1373	1637		874	1777	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	21	1010	23	51	1164	21	116	59	49	48	30	13
RTOR Reduction (vph)	0	1	0	0	0	0	0	15	0	0	7	0
Lane Group Flow (vph)	21	1032	0	51	1185	0	116	93	0	48	36	0
Heavy Vehicles (%)	8%	3%	0%	0%	6%	0%	0%	5%	12%	3%	3%	0%
Turn Type	pm+pt	NA		pm+pt	NA		Perm	NA		Perm	NA	
Protected Phases	5	2		1	6			7			3	
Permitted Phases	2			6			7			3		
Actuated Green, G (s)	165.9	161.6		172.1	164.7		31.3	31.3		21.9	21.9	
Effective Green, g (s)	167.9	162.6		174.1	165.7		33.3	33.3		23.9	23.9	
Actuated g/C Ratio	0.76	0.74		0.79	0.75		0.15	0.15		0.11	0.11	
Clearance Time (s)	6.6	6.6		6.6	6.6		6.5	6.5		6.5	6.5	
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Lane Grp Cap (vph)	311	2583		412	2560		207	247		94	193	
v/s Ratio Prot	0.00	0.30		c0.00	c0.35			0.06			0.02	
v/s Ratio Perm	0.05			0.09			c0.08			0.05		
v/c Ratio	0.07	0.40		0.12	0.46		0.56	0.38		0.51	0.19	
Uniform Delay, d1	7.2	10.6		6.2	10.3		86.6	84.0		92.5	89.2	
Progression Factor	0.07	0.10		0.29	0.20		1.00	1.00		1.00	1.00	
Incremental Delay, d2	0.1	0.4		0.1	0.5		3.4	1.0		4.6	0.5	
Delay (s)	0.6	1.5		1.9	2.6		90.0	85.0		97.2	89.7	
Level of Service	A	A		A	A		F	F		F	F	
Approach Delay (s)		1.4			2.6			87.6			93.6	
Approach LOS		A			A			F			F	

Intersection Summary			
HCM 2000 Control Delay	12.6	HCM 2000 Level of Service	B
HCM 2000 Volume to Capacity ratio	0.48		
Actuated Cycle Length (s)	220.0	Sum of lost time (s)	20.7
Intersection Capacity Utilization	60.1%	ICU Level of Service	B
Analysis Period (min)	15		
c Critical Lane Group			

HCM Signalized Intersection Capacity Analysis

2: Walnut Street/Fairchester Drive & Fairfax Boulevard

Background PM



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	28	874	13	48	1386	11	41	29	63	33	25	15
Future Volume (vph)	28	874	13	48	1386	11	41	29	63	33	25	15
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	5.6	5.6		5.6	5.6		4.5	4.5		4.5	4.5	
Lane Util. Factor	1.00	0.95		1.00	0.95		1.00	1.00		1.00	1.00	
Frt	1.00	1.00		1.00	1.00		1.00	0.90		1.00	0.94	
Flt Protected	0.95	1.00		0.95	1.00		0.95	1.00		0.95	1.00	
Satd. Flow (prot)	1671	3499		1805	3403		1805	1554		1752	1761	
Flt Permitted	0.14	1.00		0.27	1.00		0.71	1.00		0.38	1.00	
Satd. Flow (perm)	246	3499		515	3403		1357	1554		702	1761	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	30	950	14	52	1507	12	45	32	68	36	27	16
RTOR Reduction (vph)	0	0	0	0	0	0	0	41	0	0	10	0
Lane Group Flow (vph)	30	964	0	52	1519	0	45	59	0	36	33	0
Heavy Vehicles (%)	8%	3%	0%	0%	6%	0%	0%	5%	12%	3%	3%	0%
Turn Type	pm+pt	NA		pm+pt	NA		Perm	NA		Perm	NA	
Protected Phases	5	2		1	6			7			3	
Permitted Phases	2			6			7			3		
Actuated Green, G (s)	175.8	170.1		179.0	171.7		22.9	22.9		13.5	13.5	
Effective Green, g (s)	177.8	171.1		181.0	172.7		24.9	24.9		15.5	15.5	
Actuated g/C Ratio	0.81	0.78		0.82	0.78		0.11	0.11		0.07	0.07	
Clearance Time (s)	6.6	6.6		6.6	6.6		6.5	6.5		6.5	6.5	
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Lane Grp Cap (vph)	242	2721		472	2671		153	175		49	124	
v/s Ratio Prot	0.00	0.28		c0.00	c0.45			c0.04			0.02	
v/s Ratio Perm	0.10			0.09			0.03			c0.05		
v/c Ratio	0.12	0.35		0.11	0.57		0.29	0.34		0.73	0.26	
Uniform Delay, d1	6.5	7.5		4.1	9.2		89.5	90.0		100.2	96.8	
Progression Factor	0.14	0.16		0.11	0.12		1.00	1.00		1.00	1.00	
Incremental Delay, d2	0.2	0.3		0.1	0.7		1.1	1.2		43.4	1.1	
Delay (s)	1.1	1.5		0.5	1.8		90.6	91.1		143.7	98.0	
Level of Service	A	A		A	A		F	F		F	F	
Approach Delay (s)		1.5			1.8			90.9			118.8	
Approach LOS		A			A			F			F	

Intersection Summary

HCM 2000 Control Delay	9.6	HCM 2000 Level of Service	A
HCM 2000 Volume to Capacity ratio	0.57		
Actuated Cycle Length (s)	220.0	Sum of lost time (s)	20.7
Intersection Capacity Utilization	57.2%	ICU Level of Service	B
Analysis Period (min)	15		
c Critical Lane Group			

HCM Signalized Intersection Capacity Analysis

3: Oak Street/Meredith Drive & Fairfax Boulevard

Background AM



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	8	1671	87	132	682	2	98	1	167	32	2	13
Future Volume (vph)	8	1671	87	132	682	2	98	1	167	32	2	13
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	5.6	4.6		5.6	4.6			4.5	4.5		4.5	
Lane Util. Factor	1.00	0.95		1.00	0.95			1.00	1.00		1.00	
Frt	1.00	0.99		1.00	1.00			1.00	0.85		0.96	
Flt Protected	0.95	1.00		0.95	1.00			0.95	1.00		0.97	
Satd. Flow (prot)	1597	3484		1805	3405			1810	1615		1769	
Flt Permitted	0.37	1.00		0.03	1.00			0.95	1.00		0.97	
Satd. Flow (perm)	621	3484		63	3405			1810	1615		1769	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	9	1816	95	143	741	2	107	1	182	35	2	14
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	165	0	8	0
Lane Group Flow (vph)	9	1911	0	143	743	0	0	108	17	0	43	0
Heavy Vehicles (%)	13%	3%	0%	0%	6%	0%	0%	0%	0%	0%	0%	0%
Turn Type	pm+pt	NA		pm+pt	NA		Split	NA	Perm	Split	NA	
Protected Phases	5	2		1	6		4	4		7	7	
Permitted Phases	2			6					4			
Actuated Green, G (s)	116.3	113.5		136.8	127.4			15.7	15.7		8.9	
Effective Green, g (s)	118.3	115.5		137.8	129.4			17.7	17.7		10.9	
Actuated g/C Ratio	0.62	0.61		0.73	0.68			0.09	0.09		0.06	
Clearance Time (s)	6.6	6.6		6.6	6.6			6.5	6.5		6.5	
Vehicle Extension (s)	3.0	3.0		3.0	3.0			3.0	3.0		3.0	
Lane Grp Cap (vph)	406	2117		207	2318			168	150		101	
v/s Ratio Prot	0.00	c0.55		c0.06	0.22			c0.06			c0.02	
v/s Ratio Perm	0.01			0.43					0.01			
v/c Ratio	0.02	0.90		0.69	0.32			0.64	0.11		0.43	
Uniform Delay, d1	13.6	32.4		63.3	12.4			83.1	79.0		86.5	
Progression Factor	0.88	0.43		1.45	0.65			1.00	1.00		1.00	
Incremental Delay, d2	0.0	5.3		9.3	0.4			8.1	0.3		2.9	
Delay (s)	11.9	19.2		100.8	8.4			91.3	79.3		89.5	
Level of Service	B	B		F	A			F	E		F	
Approach Delay (s)		19.2			23.3			83.7			89.5	
Approach LOS		B			C			F			F	

Intersection Summary

HCM 2000 Control Delay	27.4	HCM 2000 Level of Service	C
HCM 2000 Volume to Capacity ratio	0.80		
Actuated Cycle Length (s)	190.0	Sum of lost time (s)	24.2
Intersection Capacity Utilization	77.9%	ICU Level of Service	D
Analysis Period (min)	15		
c Critical Lane Group			

HCM Signalized Intersection Capacity Analysis

3: Oak Street/Meredith Drive & Fairfax Boulevard

Background PM School



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	23	887	42	105	1231	5	79	19	173	19	5	38
Future Volume (vph)	23	887	42	105	1231	5	79	19	173	19	5	38
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	5.6	4.6		5.6	4.6			4.5	4.5		4.5	
Lane Util. Factor	1.00	0.95		1.00	0.95			1.00	1.00		1.00	
Frt	1.00	0.99		1.00	1.00			1.00	0.85		0.92	
Flt Protected	0.95	1.00		0.95	1.00			0.96	1.00		0.98	
Satd. Flow (prot)	1597	3486		1805	3404			1827	1615		1716	
Flt Permitted	0.16	1.00		0.23	1.00			0.96	1.00		0.98	
Satd. Flow (perm)	262	3486		439	3404			1827	1615		1716	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	25	964	46	114	1338	5	86	21	188	21	5	41
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	172	0	26	0
Lane Group Flow (vph)	25	1010	0	114	1343	0	0	107	16	0	41	0
Heavy Vehicles (%)	13%	3%	0%	0%	6%	0%	0%	0%	0%	0%	0%	0%
Turn Type	pm+pt	NA		pm+pt	NA		Split	NA	Perm	Split	NA	
Protected Phases	5	2		1	6		4	4		7	7	
Permitted Phases	2			6					4			
Actuated Green, G (s)	153.6	147.8		160.8	151.4			16.6	16.6		11.0	
Effective Green, g (s)	155.6	149.8		162.8	153.4			18.6	18.6		13.0	
Actuated g/C Ratio	0.71	0.68		0.74	0.70			0.08	0.08		0.06	
Clearance Time (s)	6.6	6.6		6.6	6.6			6.5	6.5		6.5	
Vehicle Extension (s)	3.0	3.0		3.0	3.0			3.0	3.0		3.0	
Lane Grp Cap (vph)	226	2373		389	2373			154	136		101	
v/s Ratio Prot	0.00	0.29		c0.01	c0.39			c0.06			c0.02	
v/s Ratio Perm	0.07			0.20					0.01			
v/c Ratio	0.11	0.43		0.29	0.57			0.69	0.12		0.40	
Uniform Delay, d1	12.6	15.8		10.3	16.7			97.9	93.1		99.8	
Progression Factor	0.30	0.24		0.64	0.40			1.00	1.00		1.00	
Incremental Delay, d2	0.2	0.5		0.4	0.9			12.8	0.4		2.6	
Delay (s)	4.0	4.3		6.9	7.6			110.7	93.5		102.4	
Level of Service	A	A		A	A			F	F		F	
Approach Delay (s)		4.3			7.5			99.7			102.4	
Approach LOS		A			A			F			F	

Intersection Summary

HCM 2000 Control Delay	18.1	HCM 2000 Level of Service	B
HCM 2000 Volume to Capacity ratio	0.55		
Actuated Cycle Length (s)	220.0	Sum of lost time (s)	24.2
Intersection Capacity Utilization	64.3%	ICU Level of Service	C
Analysis Period (min)	15		
c Critical Lane Group			

HCM Signalized Intersection Capacity Analysis

3: Oak Street/Meredith Drive & Fairfax Boulevard

Background PM



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	14	851	19	101	1513	7	18	10	130	23	4	32
Future Volume (vph)	14	851	19	101	1513	7	18	10	130	23	4	32
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	5.6	4.6		5.6	4.6			4.5	4.5		4.5	
Lane Util. Factor	1.00	0.95		1.00	0.95			1.00	1.00		1.00	
Frt	1.00	1.00		1.00	1.00			1.00	0.85		0.93	
Flt Protected	0.95	1.00		0.95	1.00			0.97	1.00		0.98	
Satd. Flow (prot)	1597	3495		1805	3404			1841	1615		1726	
Flt Permitted	0.11	1.00		0.26	1.00			0.97	1.00		0.98	
Satd. Flow (perm)	177	3495		488	3404			1841	1615		1726	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	15	925	21	110	1645	8	20	11	141	25	4	35
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	133	0	21	0
Lane Group Flow (vph)	15	946	0	110	1653	0	0	31	8	0	43	0
Heavy Vehicles (%)	13%	3%	0%	0%	6%	0%	0%	0%	0%	0%	0%	0%
Turn Type	pm+pt	NA		pm+pt	NA		Split	NA	Perm	Split	NA	
Protected Phases	5	2		1	6		4	4		7	7	
Permitted Phases	2			6					4			
Actuated Green, G (s)	159.0	154.8		168.4	159.5			9.9	9.9		11.2	
Effective Green, g (s)	161.0	156.8		170.4	161.5			11.9	11.9		13.2	
Actuated g/C Ratio	0.73	0.71		0.77	0.73			0.05	0.05		0.06	
Clearance Time (s)	6.6	6.6		6.6	6.6			6.5	6.5		6.5	
Vehicle Extension (s)	3.0	3.0		3.0	3.0			3.0	3.0		3.0	
Lane Grp Cap (vph)	163	2490		437	2498			99	87		103	
v/s Ratio Prot	0.00	0.27		c0.01	c0.49			c0.02			c0.03	
v/s Ratio Perm	0.07			0.18					0.00			
v/c Ratio	0.09	0.38		0.25	0.66			0.31	0.09		0.42	
Uniform Delay, d1	13.0	12.4		7.6	15.1			100.1	98.9		99.7	
Progression Factor	0.33	0.24		1.02	0.56			1.00	1.00		1.00	
Incremental Delay, d2	0.2	0.4		0.3	1.2			1.8	0.4		2.8	
Delay (s)	4.5	3.3		8.0	9.6			101.9	99.3		102.5	
Level of Service	A	A		A	A			F	F		F	
Approach Delay (s)		3.4			9.5			99.8			102.5	
Approach LOS		A			A			F			F	

Intersection Summary

HCM 2000 Control Delay	14.8	HCM 2000 Level of Service	B
HCM 2000 Volume to Capacity ratio	0.60		
Actuated Cycle Length (s)	220.0	Sum of lost time (s)	24.2
Intersection Capacity Utilization	70.2%	ICU Level of Service	C
Analysis Period (min)	15		
c Critical Lane Group			

HCM Signalized Intersection Capacity Analysis

5: Fairfax Boulevard & Fairfax Shoppes Entrance

Background AM



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (vph)	26	1791	770	9	11	18
Future Volume (vph)	26	1791	770	9	11	18
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	5.2	5.2	5.2		4.0	
Lane Util. Factor	1.00	0.95	0.95		1.00	
Frt	1.00	1.00	1.00		0.92	
Flt Protected	0.95	1.00	1.00		0.98	
Satd. Flow (prot)	1805	3505	3434		1708	
Flt Permitted	0.31	1.00	1.00		0.98	
Satd. Flow (perm)	591	3505	3434		1708	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	28	1947	837	10	12	20
RTOR Reduction (vph)	0	0	0	0	19	0
Lane Group Flow (vph)	28	1947	847	0	13	0
Heavy Vehicles (%)	0%	3%	5%	0%	0%	0%
Turn Type	pm+pt	NA	NA		Prot	
Protected Phases	1	6	2		4	
Permitted Phases	6					
Actuated Green, G (s)	167.4	167.4	155.6		10.4	
Effective Green, g (s)	168.4	168.4	156.6		12.4	
Actuated g/C Ratio	0.89	0.89	0.82		0.07	
Clearance Time (s)	6.2	6.2	6.2		6.0	
Vehicle Extension (s)	3.0	3.0	3.0		3.0	
Lane Grp Cap (vph)	565	3106	2830		111	
v/s Ratio Prot	0.00	c0.56	0.25		c0.01	
v/s Ratio Perm	0.04					
v/c Ratio	0.05	0.63	0.30		0.12	
Uniform Delay, d1	1.6	2.8	3.9		83.7	
Progression Factor	0.17	0.70	0.08		1.00	
Incremental Delay, d2	0.0	0.5	0.3		0.5	
Delay (s)	0.3	2.5	0.6		84.1	
Level of Service	A	A	A		F	
Approach Delay (s)		2.4	0.6		84.1	
Approach LOS		A	A		F	

Intersection Summary

HCM 2000 Control Delay	2.8	HCM 2000 Level of Service	A
HCM 2000 Volume to Capacity ratio	0.61		
Actuated Cycle Length (s)	190.0	Sum of lost time (s)	14.4
Intersection Capacity Utilization	65.5%	ICU Level of Service	C
Analysis Period (min)	15		
c Critical Lane Group			

HCM Signalized Intersection Capacity Analysis

5: Fairfax Boulevard & Fairfax Shoppes Entrance

Background PM School



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (vph)	44	943	1229	29	57	57
Future Volume (vph)	44	943	1229	29	57	57
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	5.2	5.2	5.2		4.0	
Lane Util. Factor	1.00	0.95	0.95		1.00	
Frt	1.00	1.00	1.00		0.93	
Flt Protected	0.95	1.00	1.00		0.98	
Satd. Flow (prot)	1805	3505	3430		1729	
Flt Permitted	0.17	1.00	1.00		0.98	
Satd. Flow (perm)	325	3505	3430		1729	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	48	1025	1336	32	62	62
RTOR Reduction (vph)	0	0	1	0	18	0
Lane Group Flow (vph)	48	1025	1367	0	106	0
Heavy Vehicles (%)	0%	3%	5%	0%	0%	0%
Turn Type	pm+pt	NA	NA		Prot	
Protected Phases	1	6	2		4	
Permitted Phases	6					
Actuated Green, G (s)	189.0	189.0	174.0		18.8	
Effective Green, g (s)	190.0	190.0	175.0		20.8	
Actuated g/C Ratio	0.86	0.86	0.80		0.09	
Clearance Time (s)	6.2	6.2	6.2		6.0	
Vehicle Extension (s)	3.0	3.0	3.0		3.0	
Lane Grp Cap (vph)	346	3027	2728		163	
v/s Ratio Prot	0.01	c0.29	c0.40		c0.06	
v/s Ratio Perm	0.11					
v/c Ratio	0.14	0.34	0.50		0.65	
Uniform Delay, d1	7.8	2.9	7.7		96.1	
Progression Factor	0.20	0.24	0.10		1.00	
Incremental Delay, d2	0.2	0.3	0.5		8.6	
Delay (s)	1.7	1.0	1.3		104.7	
Level of Service	A	A	A		F	
Approach Delay (s)		1.0	1.3		104.7	
Approach LOS		A	A		F	

Intersection Summary

HCM 2000 Control Delay	6.2	HCM 2000 Level of Service	A
HCM 2000 Volume to Capacity ratio	0.51		
Actuated Cycle Length (s)	220.0	Sum of lost time (s)	14.4
Intersection Capacity Utilization	52.6%	ICU Level of Service	A
Analysis Period (min)	15		
c Critical Lane Group			

HCM Signalized Intersection Capacity Analysis

5: Fairfax Boulevard & Fairfax Shoppes Entrance

Background PM



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (vph)	37	1003	1497	29	64	31
Future Volume (vph)	37	1003	1497	29	64	31
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	5.2	5.2	5.2		4.0	
Lane Util. Factor	1.00	0.95	0.95		1.00	
Frt	1.00	1.00	1.00		0.96	
Flt Protected	0.95	1.00	1.00		0.97	
Satd. Flow (prot)	1805	3505	3431		1757	
Flt Permitted	0.12	1.00	1.00		0.97	
Satd. Flow (perm)	228	3505	3431		1757	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	40	1090	1627	32	70	34
RTOR Reduction (vph)	0	0	0	0	9	0
Lane Group Flow (vph)	40	1090	1659	0	95	0
Heavy Vehicles (%)	0%	3%	5%	0%	0%	0%
Turn Type	pm+pt	NA	NA		Prot	
Protected Phases	1	6	2		4	
Permitted Phases	6					
Actuated Green, G (s)	190.5	190.5	177.3		17.3	
Effective Green, g (s)	191.5	191.5	178.3		19.3	
Actuated g/C Ratio	0.87	0.87	0.81		0.09	
Clearance Time (s)	6.2	6.2	6.2		6.0	
Vehicle Extension (s)	3.0	3.0	3.0		3.0	
Lane Grp Cap (vph)	255	3050	2780		154	
v/s Ratio Prot	0.01	c0.31	c0.48		c0.05	
v/s Ratio Perm	0.13					
v/c Ratio	0.16	0.36	0.60		0.62	
Uniform Delay, d1	10.4	2.7	7.7		96.8	
Progression Factor	0.29	0.37	0.01		1.00	
Incremental Delay, d2	0.3	0.3	0.4		7.1	
Delay (s)	3.3	1.3	0.5		103.9	
Level of Service	A	A	A		F	
Approach Delay (s)		1.4	0.5		103.9	
Approach LOS		A	A		F	

Intersection Summary

HCM 2000 Control Delay	4.5	HCM 2000 Level of Service	A
HCM 2000 Volume to Capacity ratio	0.59		
Actuated Cycle Length (s)	220.0	Sum of lost time (s)	14.4
Intersection Capacity Utilization	58.3%	ICU Level of Service	B
Analysis Period (min)	15		
c Critical Lane Group			

HCM Unsignalized Intersection Capacity Analysis

6: Paul VI Entrance & Fairfax Boulevard

Background AM

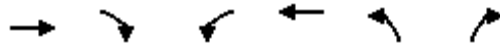


Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑		↵	↑↑	↵	
Traffic Volume (veh/h)	1780	22	27	787	5	14
Future Volume (Veh/h)	1780	22	27	787	5	14
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	1935	24	29	855	5	15
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	TWLTL		TWLTL			
Median storage (veh)	2		2			
Upstream signal (ft)	217		618			
pX, platoon unblocked			0.84		0.90	0.84
vC, conflicting volume			1959		2432	980
vC1, stage 1 conf vol					1947	
vC2, stage 2 conf vol					486	
vCu, unblocked vol			1757		1714	587
tC, single (s)			4.1		6.8	6.9
tC, 2 stage (s)					5.8	
tF (s)			2.2		3.5	3.3
p0 queue free %			90		95	96
cM capacity (veh/h)			295		103	379
Direction, Lane #	EB 1	EB 2	WB 1	WB 2	WB 3	NB 1
Volume Total	1290	669	29	428	428	20
Volume Left	0	0	29	0	0	5
Volume Right	0	24	0	0	0	15
cSH	1700	1700	295	1700	1700	227
Volume to Capacity	0.76	0.39	0.10	0.25	0.25	0.09
Queue Length 95th (ft)	0	0	8	0	0	7
Control Delay (s)	0.0	0.0	18.5	0.0	0.0	22.4
Lane LOS			C			C
Approach Delay (s)	0.0		0.6			22.4
Approach LOS						C
Intersection Summary						
Average Delay			0.3			
Intersection Capacity Utilization			59.9%	ICU Level of Service	B	
Analysis Period (min)			15			

HCM Unsignalized Intersection Capacity Analysis

6: Paul VI Entrance & Fairfax Boulevard

Background PM School

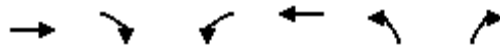


Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑		↵	↑↑	↵	
Traffic Volume (veh/h)	967	33	34	1285	16	75
Future Volume (Veh/h)	967	33	34	1285	16	75
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	1051	36	37	1397	17	82
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	TWLTL		TWLTL			
Median storage (veh)	2		2			
Upstream signal (ft)	217		618			
pX, platoon unblocked			0.94		0.77	0.94
vC, conflicting volume			1087		1842	544
vC1, stage 1 conf vol					1069	
vC2, stage 2 conf vol					772	
vCu, unblocked vol			971		1153	395
tC, single (s)			4.1		6.8	6.9
tC, 2 stage (s)					5.8	
tF (s)			2.2		3.5	3.3
p0 queue free %			94		94	86
cM capacity (veh/h)			665		297	570
Direction, Lane #	EB 1	EB 2	WB 1	WB 2	WB 3	NB 1
Volume Total	701	386	37	698	698	99
Volume Left	0	0	37	0	0	17
Volume Right	0	36	0	0	0	82
cSH	1700	1700	665	1700	1700	492
Volume to Capacity	0.41	0.23	0.06	0.41	0.41	0.20
Queue Length 95th (ft)	0	0	4	0	0	19
Control Delay (s)	0.0	0.0	10.7	0.0	0.0	14.2
Lane LOS			B			B
Approach Delay (s)	0.0		0.3			14.2
Approach LOS						B
Intersection Summary						
Average Delay			0.7			
Intersection Capacity Utilization			47.7%	ICU Level of Service		A
Analysis Period (min)			15			

HCM Unsignalized Intersection Capacity Analysis

6: Paul VI Entrance & Fairfax Boulevard

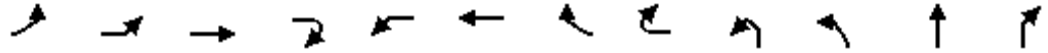
Background PM



Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑		↵	↑↑	↵	
Traffic Volume (veh/h)	1066	1	5	1623	1	4
Future Volume (Veh/h)	1066	1	5	1623	1	4
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	1159	1	5	1764	1	4
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	TWLTL		TWLTL			
Median storage veh)	2		2			
Upstream signal (ft)	217		618			
pX, platoon unblocked			0.94		0.56	0.94
vC, conflicting volume			1160		2052	580
vC1, stage 1 conf vol					1160	
vC2, stage 2 conf vol					892	
vCu, unblocked vol			1044		845	427
tC, single (s)			4.1		6.8	6.9
tC, 2 stage (s)					5.8	
tF (s)			2.2		3.5	3.3
p0 queue free %			99		100	99
cM capacity (veh/h)			623		268	542
Direction, Lane #	EB 1	EB 2	WB 1	WB 2	WB 3	NB 1
Volume Total	773	387	5	882	882	5
Volume Left	0	0	5	0	0	1
Volume Right	0	1	0	0	0	4
cSH	1700	1700	623	1700	1700	450
Volume to Capacity	0.45	0.23	0.01	0.52	0.52	0.01
Queue Length 95th (ft)	0	0	1	0	0	1
Control Delay (s)	0.0	0.0	10.8	0.0	0.0	13.1
Lane LOS			B			
Approach Delay (s)	0.0		0.0			
Approach LOS				B		
Intersection Summary						
Average Delay			0.0			
Intersection Capacity Utilization			54.9%	ICU Level of Service	A	
Analysis Period (min)			15			

HCM Signalized Intersection Capacity Analysis
 8: Fairfax Boulevard & McLean Avenue & Warwick Road

Background AM























Movement	EBL2	EBL	EBT	EBR2	WBL	WBT	WBR	WBR2	NBL2	NBL	NBT	NBR
Lane Configurations			↕			↕					↕	
Traffic Volume (vph)	3	98	26	2	14	6	7	3	12	2	5	35
Future Volume (vph)	3	98	26	2	14	6	7	3	12	2	5	35
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)			5.0			5.0					5.0	
Lane Util. Factor			1.00			1.00					1.00	
Frt			1.00			0.95					0.91	
Flt Protected			0.96			0.98					0.99	
Satd. Flow (prot)			1477			1719					1710	
Flt Permitted			0.96			0.98					0.91	
Satd. Flow (perm)			1477			1719					1583	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	3	107	28	2	15	7	8	3	13	2	5	38
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	0	0	140	0	0	33	0	0	0	0	58	0
Heavy Vehicles (%)	100%	0%	100%	100%	7%	0%	0%	0%	0%	0%	0%	0%
Turn Type	Perm	Split	NA		Split	NA			Perm	Perm	NA	
Protected Phases		3	3		4	4					7	
Permitted Phases	3								7	7		
Actuated Green, G (s)			19.7			8.0					12.4	
Effective Green, g (s)			21.2			9.5					13.9	
Actuated g/C Ratio			0.11			0.05					0.07	
Clearance Time (s)			6.5			6.5					6.5	
Vehicle Extension (s)			3.0			3.0					3.0	
Lane Grp Cap (vph)			164			85					115	
v/s Ratio Prot			c0.09			c0.02						
v/s Ratio Perm											c0.04	
v/c Ratio			0.85			0.39					0.50	
Uniform Delay, d1			82.9			87.4					84.7	
Progression Factor			1.00			1.00					1.00	
Incremental Delay, d2			32.6			2.9					3.5	
Delay (s)			115.4			90.4					88.2	
Level of Service			F			F					F	
Approach Delay (s)			115.4			90.4					88.2	
Approach LOS			F			F					F	
Intersection Summary												
HCM 2000 Control Delay			29.7			HCM 2000 Level of Service			C			
HCM 2000 Volume to Capacity ratio			0.80									
Actuated Cycle Length (s)			190.0			Sum of lost time (s)			25.7			
Intersection Capacity Utilization			82.7%			ICU Level of Service			E			
Analysis Period (min)			15									
c Critical Lane Group												

HCM Signalized Intersection Capacity Analysis

8: Fairfax Boulevard & McLean Avenue & Warwick Road

Background AM

												
Movement	SBL2	SBL	SBT	SBR	SBR2	NEL	NET	NER	NER2	SWL2	SWL	SWT
Lane Configurations							 					 
Traffic Volume (vph)	25	3	5	2	4	3	1728	26	7	1	16	784
Future Volume (vph)	25	3	5	2	4	3	1728	26	7	1	16	784
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		5.0	5.0			5.6	5.1				5.6	5.1
Lane Util. Factor		1.00	1.00			1.00	0.95				1.00	0.95
Frt		1.00	0.92			1.00	1.00				1.00	0.99
Flt Protected		0.95	1.00			0.95	1.00				0.95	1.00
Satd. Flow (prot)		1805	1745			1752	3600				1805	3484
Flt Permitted		0.61	1.00			0.26	1.00				0.03	1.00
Satd. Flow (perm)		1158	1745			472	3600				65	3484
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	27	3	5	2	4	3	1878	28	8	1	17	852
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	0	30	11	0	0	3	1914	0	0	0	18	928
Heavy Vehicles (%)	0%	0%	0%	0%	0%	3%	0%	0%	0%	0%	0%	2%
Turn Type	Perm	Perm	NA			pm+pt	NA			pm+pt	pm+pt	NA
Protected Phases			7			1	6			5	5	2
Permitted Phases	7	7				6				2	2	
Actuated Green, G (s)		12.4	12.4			113.4	112.2				119.0	115.0
Effective Green, g (s)		13.9	13.9			116.4	114.2				122.0	117.0
Actuated g/C Ratio		0.07	0.07			0.61	0.60				0.64	0.62
Clearance Time (s)		6.5	6.5			7.1	7.1				7.1	7.1
Vehicle Extension (s)		3.0	3.0			3.0	4.0				3.0	4.0
Lane Grp Cap (vph)		84	127			307	2163				92	2145
v/s Ratio Prot			0.01			0.00	c0.53				c0.01	c0.27
v/s Ratio Perm		0.03				0.01					0.12	
v/c Ratio		0.36	0.09			0.01	0.88				0.20	0.43
Uniform Delay, d1		83.8	82.1			15.3	32.3				36.9	19.1
Progression Factor		1.00	1.00			1.18	0.61				1.00	1.00
Incremental Delay, d2		2.6	0.3			0.0	4.7				1.0	0.6
Delay (s)		86.4	82.4			18.1	24.2				37.9	19.8
Level of Service		F	F			B	C				D	B
Approach Delay (s)			85.3				24.2					20.1
Approach LOS			F				C					C

Intersection Summary

HCM Signalized Intersection Capacity Analysis

8: Fairfax Boulevard & McLean Avenue & Warwick Road

Background AM

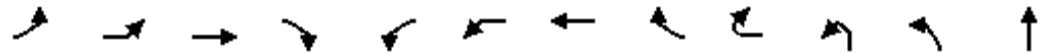


Movement	SWR	SWR2
Lane Configurations		
Traffic Volume (vph)	61	9
Future Volume (vph)	61	9
Ideal Flow (vphpl)	1900	1900
Total Lost time (s)		
Lane Util. Factor		
Fr		
Flt Protected		
Satd. Flow (prot)		
Flt Permitted		
Satd. Flow (perm)		
Peak-hour factor, PHF	0.92	0.92
Adj. Flow (vph)	66	10
RTOR Reduction (vph)	0	0
Lane Group Flow (vph)	0	0
Heavy Vehicles (%)	7%	0%
Turn Type		
Protected Phases		
Permitted Phases		
Actuated Green, G (s)		
Effective Green, g (s)		
Actuated g/C Ratio		
Clearance Time (s)		
Vehicle Extension (s)		
Lane Grp Cap (vph)		
v/s Ratio Prot		
v/s Ratio Perm		
v/c Ratio		
Uniform Delay, d1		
Progression Factor		
Incremental Delay, d2		
Delay (s)		
Level of Service		
Approach Delay (s)		
Approach LOS		
Intersection Summary		

HCM Signalized Intersection Capacity Analysis

8: Fairfax Boulevard & McLean Avenue & Warwick Road

















Background PM School



Movement	EBL2	EBL	EBT	EBR	WBL2	WBL	WBT	WBR	WBR2	NBL2	NBL	NBT
Lane Configurations			↔				↔					↔
Traffic Volume (vph)	4	73	19	3	1	4	26	10	15	5	8	4
Future Volume (vph)	4	73	19	3	1	4	26	10	15	5	8	4
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)			5.0				5.0					5.0
Lane Util. Factor			1.00				1.00					1.00
Frt			1.00				0.94					0.95
Flt Protected			0.96				1.00					0.98
Satd. Flow (prot)			1444				1769					1756
Flt Permitted			0.96				1.00					0.84
Satd. Flow (perm)			1444				1769					1518
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	4	79	21	3	1	4	28	11	16	5	9	4
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	0	0	107	0	0	0	60	0	0	0	0	30
Heavy Vehicles (%)	100%	0%	100%	100%	0%	7%	0%	0%	0%	0%	0%	0%
Turn Type	Perm	Split	NA		Perm	Split	NA			Perm	Perm	NA
Protected Phases		3	3			4	4					7
Permitted Phases	3				4					7	7	
Actuated Green, G (s)			20.1				12.9					8.6
Effective Green, g (s)			21.6				14.4					10.1
Actuated g/C Ratio			0.10				0.07					0.05
Clearance Time (s)			6.5				6.5					6.5
Vehicle Extension (s)			3.0				3.0					3.0
Lane Grp Cap (vph)			141				115					69
v/s Ratio Prot			c0.07				c0.03					
v/s Ratio Perm												c0.02
v/c Ratio			0.76				0.52					0.43
Uniform Delay, d1			96.7				99.5					102.2
Progression Factor			1.00				1.00					1.00
Incremental Delay, d2			20.6				4.2					4.3
Delay (s)			117.3				103.7					106.5
Level of Service			F				F					F
Approach Delay (s)			117.3				103.7					106.5
Approach LOS			F				F					F
Intersection Summary												
HCM 2000 Control Delay			27.1				HCM 2000 Level of Service			C		
HCM 2000 Volume to Capacity ratio			0.63									
Actuated Cycle Length (s)			220.0				Sum of lost time (s)		25.7			
Intersection Capacity Utilization			71.6%				ICU Level of Service		C			
Analysis Period (min)			15									
c Critical Lane Group												

HCM Signalized Intersection Capacity Analysis
 8: Fairfax Boulevard & McLean Avenue & Warwick Road

Background PM School

												
Movement	NBR	SBL2	SBL	SBT	SBR	SBR2	NEL2	NEL	NET	NER	NER2	SWL2
Lane Configurations												
Traffic Volume (vph)	11	17	7	6	5	1	5	5	955	51	11	3
Future Volume (vph)	11	17	7	6	5	1	5	5	955	51	11	3
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)			5.0	5.0				5.6	5.1			
Lane Util. Factor			1.00	1.00				1.00	0.95			
Frt			1.00	0.93				1.00	0.99			
Flt Protected			0.95	1.00				0.95	1.00			
Satd. Flow (prot)			1805	1768				1778	3577			
Flt Permitted			0.77	1.00				0.11	1.00			
Satd. Flow (perm)			1461	1768				203	3577			
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	12	18	8	7	5	1	5	5	1038	55	12	3
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	0	0	26	13	0	0	0	10	1105	0	0	0
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	0%	3%	0%	0%	0%	0%
Turn Type		Perm	Perm	NA			custom	pm+pt	NA			pm+pt
Protected Phases				7				1	6			5
Permitted Phases		7	7				1	6				2
Actuated Green, G (s)			8.6	8.6				141.9	139.3			
Effective Green, g (s)			10.1	10.1				144.9	141.3			
Actuated g/C Ratio			0.05	0.05				0.66	0.64			
Clearance Time (s)			6.5	6.5				7.1	7.1			
Vehicle Extension (s)			3.0	3.0				3.0	4.0			
Lane Grp Cap (vph)			67	81				163	2297			
v/s Ratio Prot				0.01				0.00	0.31			
v/s Ratio Perm			0.02					0.04				
v/c Ratio			0.39	0.16				0.06	0.48			
Uniform Delay, d1			101.9	100.9				19.2	20.4			
Progression Factor			1.00	1.00				0.90	0.61			
Incremental Delay, d2			3.7	0.9				0.2	0.7			
Delay (s)			105.6	101.8				17.4	13.1			
Level of Service			F	F				B	B			
Approach Delay (s)				104.4					13.2			
Approach LOS				F					B			

Intersection Summary

HCM Signalized Intersection Capacity Analysis

8: Fairfax Boulevard & McLean Avenue & Warwick Road

Background PM School



Movement	SWL	SWT	SWR	SWR2
Lane Configurations				
Traffic Volume (vph)	18	1305	68	14
Future Volume (vph)	18	1305	68	14
Ideal Flow (vphpl)	1900	1900	1900	1900
Total Lost time (s)	5.6	5.1		
Lane Util. Factor	1.00	0.95		
Frt	1.00	0.99		
Flt Protected	0.95	1.00		
Satd. Flow (prot)	1805	3500		
Flt Permitted	0.19	1.00		
Satd. Flow (perm)	370	3500		
Peak-hour factor, PHF	0.92	0.92	0.92	0.92
Adj. Flow (vph)	20	1418	74	15
RTOR Reduction (vph)	0	0	0	0
Lane Group Flow (vph)	23	1507	0	0
Heavy Vehicles (%)	0%	2%	7%	0%
Turn Type	pm+pt	NA		
Protected Phases	5	2		
Permitted Phases	2			
Actuated Green, G (s)	147.5	142.1		
Effective Green, g (s)	150.5	144.1		
Actuated g/C Ratio	0.68	0.65		
Clearance Time (s)	7.1	7.1		
Vehicle Extension (s)	3.0	4.0		
Lane Grp Cap (vph)	298	2292		
v/s Ratio Prot	c0.00	c0.43		
v/s Ratio Perm	0.05			
v/c Ratio	0.08	0.66		
Uniform Delay, d1	14.1	23.0		
Progression Factor	1.00	1.00		
Incremental Delay, d2	0.1	1.5		
Delay (s)	14.2	24.5		
Level of Service	B	C		
Approach Delay (s)		24.3		
Approach LOS		C		

Intersection Summary

HCM Signalized Intersection Capacity Analysis

8: Fairfax Boulevard & McLean Avenue & Warwick Road

Background PM



Movement	EBL2	EBL	EBT	EBR	EBR2	WBL2	WBL	WBT	WBR	WBR2	NBL2	NBL
Lane Configurations			↔					↔				
Traffic Volume (vph)	5	61	31	4	2	1	4	30	12	5	18	16
Future Volume (vph)	5	61	31	4	2	1	4	30	12	5	18	16
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)			5.0					5.0				
Lane Util. Factor			1.00					1.00				
Frt			0.99					0.96				
Flt Protected			0.97					1.00				
Satd. Flow (prot)			1300					1800				
Flt Permitted			0.97					1.00				
Satd. Flow (perm)			1300					1800				
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	5	66	34	4	2	1	4	33	13	5	20	17
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	0	0	111	0	0	0	0	56	0	0	0	0
Heavy Vehicles (%)	100%	0%	100%	100%	100%	0%	7%	0%	0%	0%	0%	0%
Turn Type	Perm	Split	NA			Perm	Split	NA			Perm	Perm
Protected Phases		3	3				4	4				
Permitted Phases	3										7	7
Actuated Green, G (s)			21.5					12.3				
Effective Green, g (s)			23.0					13.8				
Actuated g/C Ratio			0.10					0.06				
Clearance Time (s)			6.5					6.5				
Vehicle Extension (s)			3.0					3.0				
Lane Grp Cap (vph)			135					112				
v/s Ratio Prot			c0.09					c0.03				
v/s Ratio Perm												
v/c Ratio			0.82					0.50				
Uniform Delay, d1			96.5					99.8				
Progression Factor			1.00					1.00				
Incremental Delay, d2			31.5					3.5				
Delay (s)			128.0					103.2				
Level of Service			F					F				
Approach Delay (s)			128.0					103.2				
Approach LOS			F					F				
Intersection Summary												
HCM 2000 Control Delay			40.4					HCM 2000 Level of Service			D	
HCM 2000 Volume to Capacity ratio			0.81									
Actuated Cycle Length (s)			220.0					Sum of lost time (s)		25.7		
Intersection Capacity Utilization			84.2%					ICU Level of Service		E		
Analysis Period (min)			15									
c Critical Lane Group												

HCM Signalized Intersection Capacity Analysis

8: Fairfax Boulevard & McLean Avenue & Warwick Road

Background PM

	↑	↗	↘	↓	↙	↘	↗	↗	↘	↗	↘	↘
Movement	NBT	NBR	SBL2	SBL	SBT	SBR	SBR2	NEL2	NEL	NET	NER	NER2
Lane Configurations	↕			↗	↘				↗	↕		
Traffic Volume (vph)	20	38	11	1	6	2	2	12	12	871	33	9
Future Volume (vph)	20	38	11	1	6	2	2	12	12	871	33	9
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	5.0			5.0	5.0				5.6	5.1		
Lane Util. Factor	1.00			1.00	1.00				1.00	0.95		
Frt	0.94			1.00	0.95				1.00	0.99		
Flt Protected	0.98			0.95	1.00				0.95	1.00		
Satd. Flow (prot)	1762			1805	1796				1778	3585		
Flt Permitted	0.87			0.49	1.00				0.03	1.00		
Satd. Flow (perm)	1568			934	1796				57	3585		
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	22	41	12	1	7	2	2	13	13	947	36	10
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	100	0	0	13	11	0	0	0	26	993	0	0
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	0%	0%	3%	0%	0%	0%
Turn Type	NA		Perm	Perm	NA			custom	pm+pt	NA		
Protected Phases	7				7				1	6		
Permitted Phases			7	7				1	6			
Actuated Green, G (s)	17.8			17.8	17.8				134.8	129.2		
Effective Green, g (s)	19.3			19.3	19.3				137.8	131.2		
Actuated g/C Ratio	0.09			0.09	0.09				0.63	0.60		
Clearance Time (s)	6.5			6.5	6.5				7.1	7.1		
Vehicle Extension (s)	3.0			3.0	3.0				3.0	4.0		
Lane Grp Cap (vph)	137			81	157				91	2137		
v/s Ratio Prot					0.01				c0.01	0.28		
v/s Ratio Perm	c0.06			0.01					0.17			
v/c Ratio	0.73			0.16	0.07				0.29	0.46		
Uniform Delay, d1	97.8			92.9	92.1				40.8	24.8		
Progression Factor	1.00			1.00	1.00				1.61	0.42		
Incremental Delay, d2	17.6			0.9	0.2				1.6	0.7		
Delay (s)	115.4			93.8	92.3				67.2	11.2		
Level of Service	F			F	F				E	B		
Approach Delay (s)	115.4				93.1					12.6		
Approach LOS	F				F					B		

Intersection Summary

HCM Signalized Intersection Capacity Analysis

8: Fairfax Boulevard & McLean Avenue & Warwick Road

Background PM



Movement	SWL2	SWL	SWT	SWR	SWR2
Lane Configurations					
Traffic Volume (vph)	5	17	1602	93	5
Future Volume (vph)	5	17	1602	93	5
Ideal Flow (vphpl)	1900	1900	1900	1900	1900
Total Lost time (s)		5.6	5.1		
Lane Util. Factor		1.00	0.95		
Frt		1.00	0.99		
Flt Protected		0.95	1.00		
Satd. Flow (prot)		1805	3500		
Flt Permitted		0.22	1.00		
Satd. Flow (perm)		418	3500		
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	5	18	1741	101	5
RTOR Reduction (vph)	0	0	0	0	0
Lane Group Flow (vph)	0	23	1847	0	0
Heavy Vehicles (%)	0%	0%	2%	7%	0%
Turn Type	pm+pt	pm+pt	NA		
Protected Phases	5	5	2		
Permitted Phases	2	2			
Actuated Green, G (s)		134.6	129.1		
Effective Green, g (s)		137.6	131.1		
Actuated g/C Ratio		0.63	0.60		
Clearance Time (s)		7.1	7.1		
Vehicle Extension (s)		3.0	4.0		
Lane Grp Cap (vph)		305	2085		
v/s Ratio Prot		0.00	c0.53		
v/s Ratio Perm		0.04			
v/c Ratio		0.08	0.89		
Uniform Delay, d1		17.9	38.0		
Progression Factor		1.00	1.00		
Incremental Delay, d2		0.1	6.0		
Delay (s)		18.0	44.0		
Level of Service		B	D		
Approach Delay (s)			43.7		
Approach LOS			D		
Intersection Summary					

HCM Unsignalized Intersection Capacity Analysis

9: Walnut Street & Cedar Avenue

Background AM



Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	22	54	87	146	85	53
Future Volume (Veh/h)	22	54	87	146	85	53
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	24	59	95	159	92	58
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)		3				
Median type			None			None
Median storage (veh)						
Upstream signal (ft)						366
pX, platoon unblocked	0.99					
vC, conflicting volume	416	174			254	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	401	174			254	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)						
tF (s)	3.5	3.3			2.2	
p0 queue free %	96	93			93	
cM capacity (veh/h)	555	869			1311	
Direction, Lane #	WB 1	NB 1	SB 1			
Volume Total	83	254	150			
Volume Left	24	0	92			
Volume Right	59	159	0			
cSH	1222	1700	1311			
Volume to Capacity	0.07	0.15	0.07			
Queue Length 95th (ft)	5	0	6			
Control Delay (s)	10.1	0.0	5.1			
Lane LOS	B		A			
Approach Delay (s)	10.1	0.0	5.1			
Approach LOS	B					
Intersection Summary						
Average Delay			3.3			
Intersection Capacity Utilization			34.4%	ICU Level of Service		A
Analysis Period (min)			15			

HCM Unsignalized Intersection Capacity Analysis

9: Walnut Street & Cedar Avenue

Background PM School



Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	48	118	60	31	21	54
Future Volume (Veh/h)	48	118	60	31	21	54
Sign Control	Stop		Free		Free	
Grade	0%		0%		0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	52	128	65	34	23	59
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)	3					
Median type			None		None	
Median storage (veh)						
Upstream signal (ft)						366
pX, platoon unblocked						
vC, conflicting volume	187	82			99	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	187	82			99	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)						
tF (s)	3.5	3.3			2.2	
p0 queue free %	93	87			98	
cM capacity (veh/h)	790	978			1494	
Direction, Lane #	WB 1	NB 1	SB 1			
Volume Total	180	99	82			
Volume Left	52	0	23			
Volume Right	128	34	0			
cSH	1375	1700	1494			
Volume to Capacity	0.13	0.06	0.02			
Queue Length 95th (ft)	11	0	1			
Control Delay (s)	9.4	0.0	2.2			
Lane LOS	A		A			
Approach Delay (s)	9.4	0.0	2.2			
Approach LOS	A					
Intersection Summary						
Average Delay			5.2			
Intersection Capacity Utilization			20.7%	ICU Level of Service		A
Analysis Period (min)			15			

HCM Unsignalized Intersection Capacity Analysis

9: Walnut Street & Cedar Avenue

















Background PM



Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	34	29	73	15	23	81
Future Volume (Veh/h)	34	29	73	15	23	81
Sign Control	Stop		Free		Free	
Grade	0%		0%		0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	37	32	79	16	25	88
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)	3					
Median type			None		None	
Median storage (veh)						
Upstream signal (ft)						366
pX, platoon unblocked						
vC, conflicting volume	225	87			95	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	225	87			95	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)						
tF (s)	3.5	3.3			2.2	
p0 queue free %	95	97			98	
cM capacity (veh/h)	750	971			1499	
Direction, Lane #	WB 1	NB 1	SB 1			
Volume Total	69	95	113			
Volume Left	37	0	25			
Volume Right	32	16	0			
cSH	1399	1700	1499			
Volume to Capacity	0.05	0.06	0.02			
Queue Length 95th (ft)	4	0	1			
Control Delay (s)	9.5	0.0	1.7			
Lane LOS	A		A			
Approach Delay (s)	9.5	0.0	1.7			
Approach LOS	A					
Intersection Summary						
Average Delay			3.1			
Intersection Capacity Utilization			22.2%	ICU Level of Service		A
Analysis Period (min)	15					

















HCM Unsignalized Intersection Capacity Analysis
 10: Oak Street & Cedar Avenue

Background AM

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Sign Control		Stop			Stop			Stop			Stop	
Traffic Volume (vph)	11	214	4	58	72	189	3	41	205	209	26	6
Future Volume (vph)	11	214	4	58	72	189	3	41	205	209	26	6
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	12	233	4	63	78	205	3	45	223	227	28	7
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total (vph)	249	346	271	262								
Volume Left (vph)	12	63	3	227								
Volume Right (vph)	4	205	223	7								
Hadj (s)	0.03	-0.29	-0.46	0.19								
Departure Headway (s)	6.4	5.9	5.9	6.5								
Degree Utilization, x	0.44	0.56	0.45	0.48								
Capacity (veh/h)	500	560	540	491								
Control Delay (s)	14.3	16.2	13.6	15.3								
Approach Delay (s)	14.3	16.2	13.6	15.3								
Approach LOS	B	C	B	C								
Intersection Summary												
Delay			14.9									
Level of Service			B									
Intersection Capacity Utilization			72.3%	ICU Level of Service	C							
Analysis Period (min)			15									

HCM Unsignalized Intersection Capacity Analysis
 10: Oak Street & Cedar Avenue

















Background PM School

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Sign Control		Stop			Stop			Stop			Stop	
Traffic Volume (vph)	11	39	3	72	125	111	6	99	32	45	93	13
Future Volume (vph)	11	39	3	72	125	111	6	99	32	45	93	13
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	12	42	3	78	136	121	7	108	35	49	101	14
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total (vph)	57	335	150	164								
Volume Left (vph)	12	78	7	49								
Volume Right (vph)	3	121	35	14								
Hadj (s)	0.04	-0.14	-0.10	0.04								
Departure Headway (s)	5.2	4.7	5.0	5.1								
Degree Utilization, x	0.08	0.43	0.21	0.23								
Capacity (veh/h)	620	731	658	644								
Control Delay (s)	8.7	11.2	9.3	9.7								
Approach Delay (s)	8.7	11.2	9.3	9.7								
Approach LOS	A	B	A	A								
Intersection Summary												
Delay			10.2									
Level of Service			B									
Intersection Capacity Utilization			49.7%	ICU Level of Service	A							
Analysis Period (min)			15									

HCM Unsignalized Intersection Capacity Analysis

10: Oak Street & Cedar Avenue

Background PM

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Sign Control		Stop			Stop			Stop			Stop	
Traffic Volume (vph)	9	16	5	24	35	36	8	96	13	22	104	23
Future Volume (vph)	9	16	5	24	35	36	8	96	13	22	104	23
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	10	17	5	26	38	39	9	104	14	24	113	25
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total (vph)	32	103	127	162								
Volume Left (vph)	10	26	9	24								
Volume Right (vph)	5	39	14	25								
Hadj (s)	0.00	-0.14	-0.02	-0.03								
Departure Headway (s)	4.7	4.5	4.4	4.3								
Degree Utilization, x	0.04	0.13	0.15	0.19								
Capacity (veh/h)	706	751	786	792								
Control Delay (s)	7.9	8.1	8.2	8.4								
Approach Delay (s)	7.9	8.1	8.2	8.4								
Approach LOS	A	A	A	A								
Intersection Summary												
Delay			8.2									
Level of Service			A									
Intersection Capacity Utilization			27.1%	ICU Level of Service	A							
Analysis Period (min)			15									

HCM Unsignalized Intersection Capacity Analysis

11: Cedar Avenue & McLean Avenue

Background AM



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↔	↔		↔	
Sign Control		Stop	Stop		Stop	
Traffic Volume (vph)	61	27	28	22	9	20
Future Volume (vph)	61	27	28	22	9	20
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	66	29	30	24	10	22

Direction, Lane #	EB 1	WB 1	SB 1
Volume Total (vph)	95	54	32
Volume Left (vph)	66	0	10
Volume Right (vph)	0	24	22
Hadj (s)	0.17	-0.23	-0.32
Departure Headway (s)	4.2	3.8	3.9
Degree Utilization, x	0.11	0.06	0.03
Capacity (veh/h)	846	924	881
Control Delay (s)	7.7	7.1	7.0
Approach Delay (s)	7.7	7.1	7.0
Approach LOS	A	A	A

Intersection Summary			
Delay		7.4	
Level of Service		A	
Intersection Capacity Utilization		21.5%	ICU Level of Service
Analysis Period (min)		15	A

HCM Unsignalized Intersection Capacity Analysis
 11: Cedar Avenue & McLean Avenue

Background PM School



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↔	↔		↔	
Sign Control		Stop	Stop		Stop	
Traffic Volume (vph)	22	9	16	19	13	27
Future Volume (vph)	22	9	16	19	13	27
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	24	10	17	21	14	29

Direction, Lane #	EB 1	WB 1	SB 1
Volume Total (vph)	34	38	43
Volume Left (vph)	24	0	14
Volume Right (vph)	0	21	29
Hadj (s)	0.18	-0.30	-0.31
Departure Headway (s)	4.2	3.7	3.7
Degree Utilization, x	0.04	0.04	0.04
Capacity (veh/h)	842	949	932
Control Delay (s)	7.4	6.9	6.9
Approach Delay (s)	7.4	6.9	6.9
Approach LOS	A	A	A

Intersection Summary			
Delay		7.0	
Level of Service		A	
Intersection Capacity Utilization	18.4%		ICU Level of Service A
Analysis Period (min)		15	

HCM Unsignalized Intersection Capacity Analysis

11: Cedar Avenue & McLean Avenue

Background PM



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↖	↗		↖	↗
Sign Control		Stop	Stop		Stop	
Traffic Volume (vph)	68	24	28	20	7	23
Future Volume (vph)	68	24	28	20	7	23
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	74	26	30	22	8	25

Direction, Lane #	EB 1	WB 1	SB 1
Volume Total (vph)	100	52	33
Volume Left (vph)	74	0	8
Volume Right (vph)	0	22	25
Hadj (s)	0.18	-0.22	-0.37
Departure Headway (s)	4.2	3.8	3.9
Degree Utilization, x	0.12	0.06	0.04
Capacity (veh/h)	844	919	891
Control Delay (s)	7.7	7.1	7.0
Approach Delay (s)	7.7	7.1	7.0
Approach LOS	A	A	A

Intersection Summary			
Delay		7.4	
Level of Service		A	
Intersection Capacity Utilization		21.7%	ICU Level of Service
Analysis Period (min)		15	A






























APPENDIX E

2027 Total Future Capacity Analysis Worksheets

HCM Signalized Intersection Capacity Analysis

1: Lee Highway & Fairfax Boulevard & Main Street
























Total Future AM

													
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations	 	 		 	 			 			 		
Traffic Volume (vph)	405	775	17	346	692	36	9	899	733	43	342	223	
Future Volume (vph)	405	775	17	346	692	36	9	899	733	43	342	223	
Ideal Flow (vphp)	2500	2500	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Total Lost time (s)	5.1	5.1		4.3	4.3	6.3	4.8	5.6	5.6	4.8	5.6	5.6	
Lane Util. Factor	0.97	0.95		0.97	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	
Frt	1.00	1.00		1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	
Flt Protected	0.95	1.00		0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	
Satd. Flow (prot)	4430	4557		3335	3539	1509	1805	3539	1568	1703	3343	1524	
Flt Permitted	0.95	1.00		0.95	1.00	1.00	0.44	1.00	1.00	0.07	1.00	1.00	
Satd. Flow (perm)	4430	4557		3335	3539	1509	833	3539	1568	121	3343	1524	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	440	842	18	376	752	39	10	977	797	47	372	242	
RTOR Reduction (vph)	0	1	0	0	0	31	0	0	0	0	0	61	
Lane Group Flow (vph)	440	859	0	376	752	8	10	977	797	47	372	181	
Heavy Vehicles (%)	4%	4%	0%	5%	2%	7%	0%	2%	3%	6%	8%	6%	
Turn Type	Prot	NA		Prot	NA	Perm	pm+pt	NA	pt+ov	pm+pt	NA	pt+ov	
Protected Phases	5	2		1	6		3	8	8	1	4	4	
Permitted Phases						6	8			4			
Actuated Green, G (s)	59.9	59.9		37.8	37.8	37.8	64.5	57.4	95.2	64.5	57.4	124.4	
Effective Green, g (s)	61.9	61.9		39.8	39.8	37.8	68.5	59.4	99.2	68.5	59.4	121.3	
Actuated g/C Ratio	0.33	0.33		0.21	0.21	0.20	0.36	0.31	0.52	0.36	0.31	0.64	
Clearance Time (s)	7.1	7.1		6.3	6.3	6.3	6.8	7.6		6.8	7.6		
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0		3.0	3.0		
Lane Grp Cap (vph)	1443	1484		698	741	300	346	1106	818	119	1045	972	
v/s Ratio Prot	0.10	c0.19		0.11	c0.21		0.00	0.28	c0.51	c0.02	0.11	0.12	
v/s Ratio Perm						0.01	0.01			0.12			
v/c Ratio	0.30	0.58		0.54	1.01	0.03	0.03	0.88	0.97	0.39	0.36	0.19	
Uniform Delay, d1	47.9	53.2		66.9	75.1	61.3	46.2	62.0	44.2	80.1	50.5	14.1	
Progression Factor	1.00	1.00		1.00	1.00	1.00	1.00	1.00	1.00	1.16	1.29	3.68	
Incremental Delay, d2	0.5	1.7		3.0	36.8	0.2	0.0	10.3	25.8	2.1	0.9	0.4	
Delay (s)	48.5	54.9		69.9	111.9	61.4	46.2	72.3	70.0	95.1	66.0	52.2	
Level of Service	D	D		E	F	E	D	E	E	F	E	D	
Approach Delay (s)		52.7			96.7			71.1			63.0		
Approach LOS		D			F			E			E		
Intersection Summary													
HCM 2000 Control Delay			71.2									HCM 2000 Level of Service	E
HCM 2000 Volume to Capacity ratio			0.81										
Actuated Cycle Length (s)			190.0									Sum of lost time (s)	19.8
Intersection Capacity Utilization			80.8%									ICU Level of Service	D
Analysis Period (min)			15										
c Critical Lane Group													

HCM Signalized Intersection Capacity Analysis

1: Lee Highway & Fairfax Boulevard & Main Street
























Total Future PM School

													
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations													
Traffic Volume (vph)	462	548	3	620	880	53	30	828	460	72	656	377	
Future Volume (vph)	462	548	3	620	880	53	30	828	460	72	656	377	
Ideal Flow (vphpl)	2500	2500	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Total Lost time (s)	5.1	5.1		4.3	4.3	6.3	4.8	5.6	5.6	4.8	5.6	5.6	
Lane Util. Factor	0.97	0.95		0.97	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	
Frt	1.00	1.00		1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	
Flt Protected	0.95	1.00		0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	
Satd. Flow (prot)	4430	4565		3335	3539	1509	1805	3539	1568	1703	3343	1524	
Flt Permitted	0.95	1.00		0.95	1.00	1.00	0.23	1.00	1.00	0.14	1.00	1.00	
Satd. Flow (perm)	4430	4565		3335	3539	1509	443	3539	1568	255	3343	1524	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	502	596	3	674	957	58	33	900	500	78	713	410	
RTOR Reduction (vph)	0	0	0	0	0	40	0	0	0	0	0	42	
Lane Group Flow (vph)	502	599	0	674	957	18	33	900	500	78	713	368	
Heavy Vehicles (%)	4%	4%	0%	5%	2%	7%	0%	2%	3%	6%	8%	6%	
Turn Type	Prot	NA		Prot	NA	Perm	pm+pt	NA	pt+ov	pm+pt	NA	pt+ov	
Protected Phases	5	2		1	6		3	8	8	1	4	4	
Permitted Phases						6	8			4			
Actuated Green, G (s)	37.9	37.9		68.9	68.9	68.9	85.4	78.4	147.3	85.4	78.4	123.4	
Effective Green, g (s)	39.9	39.9		70.9	70.9	68.9	89.4	80.4	151.3	89.4	80.4	120.3	
Actuated g/C Ratio	0.18	0.18		0.32	0.32	0.31	0.41	0.37	0.69	0.41	0.37	0.55	
Clearance Time (s)	7.1	7.1		6.3	6.3	6.3	6.8	7.6		6.8	7.6		
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0		3.0	3.0		
Lane Grp Cap (vph)	803	827		1074	1140	472	235	1293	1078	162	1221	833	
v/s Ratio Prot	0.11	c0.13		0.20	c0.27		0.01	c0.25	0.32	c0.02	0.21	0.24	
v/s Ratio Perm						0.01	0.05			0.18			
v/c Ratio	0.63	0.72		0.63	0.84	0.04	0.14	0.70	0.46	0.48	0.58	0.44	
Uniform Delay, d1	83.1	84.9		63.3	69.3	52.5	62.3	59.4	15.8	80.6	56.3	29.8	
Progression Factor	1.00	1.00		1.00	1.00	1.00	1.00	1.00	1.00	1.17	1.06	1.20	
Incremental Delay, d2	3.7	5.5		2.8	7.5	0.2	0.3	3.1	1.4	2.1	1.9	1.6	
Delay (s)	86.8	90.3		66.1	76.7	52.7	62.6	62.5	17.2	96.0	61.6	37.3	
Level of Service	F	F		E	E	D	E	E	B	F	E	D	
Approach Delay (s)		88.7			71.7			46.7			55.5		
Approach LOS		F			E			D			E		
Intersection Summary													
HCM 2000 Control Delay			65.0									HCM 2000 Level of Service	E
HCM 2000 Volume to Capacity ratio			0.74										
Actuated Cycle Length (s)			220.0									Sum of lost time (s)	19.8
Intersection Capacity Utilization			79.6%									ICU Level of Service	D
Analysis Period (min)			15										
c Critical Lane Group													

HCM Signalized Intersection Capacity Analysis

1: Lee Highway & Fairfax Boulevard & Main Street

Total Future PM

													
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations													
Traffic Volume (vph)	474	604	74	640	1017	36	34	645	528	49	985	513	
Future Volume (vph)	474	604	74	640	1017	36	34	645	528	49	985	513	
Ideal Flow (vphp)	2500	2500	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Total Lost time (s)	5.1	5.1		4.3	4.3	6.3	4.8	5.6	5.6	4.8	5.6	5.6	
Lane Util. Factor	0.97	0.95		0.97	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	
Frt	1.00	0.98		1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	
Flt Protected	0.95	1.00		0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	
Satd. Flow (prot)	4430	4512		3335	3539	1509	1805	3539	1568	1703	3343	1524	
Flt Permitted	0.95	1.00		0.95	1.00	1.00	0.07	1.00	1.00	0.24	1.00	1.00	
Satd. Flow (perm)	4430	4512		3335	3539	1509	141	3539	1568	429	3343	1524	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	515	657	80	696	1105	39	37	701	574	53	1071	558	
RTOR Reduction (vph)	0	4	0	0	0	27	0	0	0	0	0	28	
Lane Group Flow (vph)	515	733	0	696	1105	12	37	701	574	53	1071	530	
Heavy Vehicles (%)	4%	4%	0%	5%	2%	7%	0%	2%	3%	6%	8%	6%	
Turn Type	Prot	NA		Prot	NA	Perm	pm+pt	NA	pt+ov	pm+pt	NA	pt+ov	
Protected Phases	5	2		1	6		3	8	8	1	4	4	
Permitted Phases						6	8			4			
Actuated Green, G (s)	37.9	37.9		68.9	68.9	68.9	85.4	78.4	147.3	85.4	78.4	123.4	
Effective Green, g (s)	39.9	39.9		70.9	70.9	68.9	89.4	80.4	151.3	89.4	80.4	120.3	
Actuated g/C Ratio	0.18	0.18		0.32	0.32	0.31	0.41	0.37	0.69	0.41	0.37	0.55	
Clearance Time (s)	7.1	7.1		6.3	6.3	6.3	6.8	7.6		6.8	7.6		
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0		3.0	3.0		
Lane Grp Cap (vph)	803	818		1074	1140	472	125	1293	1078	226	1221	833	
v/s Ratio Prot	0.12	c0.16		0.21	c0.31		c0.01	0.20	0.37	0.01	c0.32	0.35	
v/s Ratio Perm						0.01	0.11			0.09			
v/c Ratio	0.64	0.90		0.65	0.97	0.03	0.30	0.54	0.53	0.23	0.88	0.64	
Uniform Delay, d1	83.4	88.0		63.9	73.5	52.3	87.2	55.2	16.9	63.8	65.2	34.7	
Progression Factor	1.00	1.00		1.00	1.00	1.00	1.00	1.00	1.00	1.06	1.03	1.06	
Incremental Delay, d2	3.9	14.5		3.0	20.2	0.1	1.3	1.6	1.9	0.5	7.8	3.1	
Delay (s)	87.3	102.5		66.9	93.7	52.4	88.6	56.9	18.8	68.3	74.8	39.9	
Level of Service	F	F		E	F	D	F	E	B	E	E	D	
Approach Delay (s)		96.3			82.7			41.1			63.0		
Approach LOS		F			F			D			E		
Intersection Summary													
HCM 2000 Control Delay			71.1									HCM 2000 Level of Service	E
HCM 2000 Volume to Capacity ratio			0.89										
Actuated Cycle Length (s)			220.0									Sum of lost time (s)	19.8
Intersection Capacity Utilization			88.0%									ICU Level of Service	E
Analysis Period (min)			15										
c Critical Lane Group													

HCM Signalized Intersection Capacity Analysis

2: Walnut Street/Fairchester Drive & Fairfax Boulevard

Total Future AM



























Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	15	1603	13	32	610	21	24	5	71	28	0	19
Future Volume (vph)	15	1603	13	32	610	21	24	5	71	28	0	19
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	5.6	5.6		5.6	5.6		4.5	4.5		4.5	4.5	
Lane Util. Factor	1.00	0.95		1.00	0.95		1.00	1.00		1.00	1.00	
Frt	1.00	1.00		1.00	0.99		1.00	0.86		1.00	0.85	
Flt Protected	0.95	1.00		0.95	1.00		0.95	1.00		0.95	1.00	
Satd. Flow (prot)	1671	3501		1805	3395		1805	1463		1752	1615	
Flt Permitted	0.38	1.00		0.09	1.00		0.74	1.00		0.48	1.00	
Satd. Flow (perm)	671	3501		179	3395		1413	1463		892	1615	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	16	1742	14	35	663	23	26	5	77	30	0	21
RTOR Reduction (vph)	0	0	0	0	1	0	0	68	0	0	20	0
Lane Group Flow (vph)	16	1756	0	35	685	0	26	14	0	30	1	0
Heavy Vehicles (%)	8%	3%	0%	0%	6%	0%	0%	5%	12%	3%	3%	0%
Turn Type	pm+pt	NA		pm+pt	NA		Perm	NA		Perm	NA	
Protected Phases	5	2		1	6			7				3
Permitted Phases	2			6			7			3		
Actuated Green, G (s)	148.3	144.1		151.7	145.8		20.3	20.3		9.2	9.2	
Effective Green, g (s)	150.3	145.1		153.7	146.8		22.3	22.3		11.2	11.2	
Actuated g/C Ratio	0.79	0.76		0.81	0.77		0.12	0.12		0.06	0.06	
Clearance Time (s)	6.6	6.6		6.6	6.6		6.5	6.5		6.5	6.5	
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Lane Grp Cap (vph)	558	2673		203	2623		165	171		52	95	
v/s Ratio Prot	0.00	c0.50		c0.01	0.20			0.01			0.00	
v/s Ratio Perm	0.02			0.13			c0.02			c0.03		
v/c Ratio	0.03	0.66		0.17	0.26		0.16	0.08		0.58	0.01	
Uniform Delay, d1	4.2	10.6		9.3	6.2		75.4	74.7		87.1	84.2	
Progression Factor	0.40	0.61		0.33	0.12		1.00	1.00		1.00	1.00	
Incremental Delay, d2	0.0	1.1		0.4	0.2		0.4	0.2		14.6	0.1	
Delay (s)	1.7	7.6		3.5	1.0		75.9	74.9		101.7	84.2	
Level of Service	A	A		A	A		E	E		F	F	
Approach Delay (s)		7.5			1.1			75.2			94.5	
Approach LOS		A			A			E			F	

Intersection Summary		
HCM 2000 Control Delay	10.2	HCM 2000 Level of Service
HCM 2000 Volume to Capacity ratio	0.62	B
Actuated Cycle Length (s)	190.0	Sum of lost time (s)
Intersection Capacity Utilization	61.4%	20.7
Analysis Period (min)	15	ICU Level of Service
c Critical Lane Group		B

HCM Signalized Intersection Capacity Analysis

2: Walnut Street/Fairchester Drive & Fairfax Boulevard























Total Future PM School

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		 			 			 			 	
Traffic Volume (vph)	19	967	18	47	1113	19	25	33	45	44	21	12
Future Volume (vph)	19	967	18	47	1113	19	25	33	45	44	21	12
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	5.6	5.6		5.6	5.6		4.5	4.5		4.5	4.5	
Lane Util. Factor	1.00	0.95		1.00	0.95		1.00	1.00		1.00	1.00	
Frt	1.00	1.00		1.00	1.00		1.00	0.91		1.00	0.95	
Flt Protected	0.95	1.00		0.95	1.00		0.95	1.00		0.95	1.00	
Satd. Flow (prot)	1671	3497		1805	3400		1805	1592		1752	1763	
Flt Permitted	0.20	1.00		0.24	1.00		0.73	1.00		0.49	1.00	
Satd. Flow (perm)	358	3497		447	3400		1394	1592		909	1763	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	21	1051	20	51	1210	21	27	36	49	48	23	13
RTOR Reduction (vph)	0	0	0	0	0	0	0	26	0	0	9	0
Lane Group Flow (vph)	21	1071	0	51	1231	0	27	59	0	48	27	0
Heavy Vehicles (%)	8%	3%	0%	0%	6%	0%	0%	5%	12%	3%	3%	0%
Turn Type	pm+pt	NA		pm+pt	NA		Perm	NA		Perm	NA	
Protected Phases	5	2		1	6			7				3
Permitted Phases	2			6			7			3		
Actuated Green, G (s)	172.7	168.5		178.9	171.6		24.5	24.5		15.1	15.1	
Effective Green, g (s)	174.7	169.5		180.9	172.6		26.5	26.5		17.1	17.1	
Actuated g/C Ratio	0.79	0.77		0.82	0.78		0.12	0.12		0.08	0.08	
Clearance Time (s)	6.6	6.6		6.6	6.6		6.5	6.5		6.5	6.5	
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Lane Grp Cap (vph)	315	2694		418	2667		167	191		70	137	
v/s Ratio Prot	0.00	0.31		c0.00	c0.36			c0.04			0.02	
v/s Ratio Perm	0.05			0.10			0.02			c0.05		
v/c Ratio	0.07	0.40		0.12	0.46		0.16	0.31		0.69	0.20	
Uniform Delay, d1	5.5	8.4		4.6	8.0		86.8	88.4		98.8	95.0	
Progression Factor	0.08	0.12		0.16	0.09		1.00	1.00		1.00	1.00	
Incremental Delay, d2	0.1	0.3		0.1	0.5		0.5	0.9		24.3	0.7	
Delay (s)	0.5	1.3		0.9	1.2		87.2	89.3		123.2	95.7	
Level of Service	A	A		A	A		F	F		F	F	
Approach Delay (s)		1.3			1.2			88.8			111.4	
Approach LOS		A			A			F			F	
Intersection Summary												
HCM 2000 Control Delay			8.7				HCM 2000 Level of Service			A		
HCM 2000 Volume to Capacity ratio			0.47									
Actuated Cycle Length (s)			220.0				Sum of lost time (s)			20.7		
Intersection Capacity Utilization			56.6%				ICU Level of Service			B		
Analysis Period (min)			15									
c Critical Lane Group												

HCM Signalized Intersection Capacity Analysis

2: Walnut Street/Fairchester Drive & Fairfax Boulevard





















Total Future PM

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		 			 							
Traffic Volume (vph)	28	972	15	48	1463	11	21	23	63	33	22	15
Future Volume (vph)	28	972	15	48	1463	11	21	23	63	33	22	15
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	5.6	5.6		5.6	5.6		4.5	4.5		4.5	4.5	
Lane Util. Factor	1.00	0.95		1.00	0.95		1.00	1.00		1.00	1.00	
Frt	1.00	1.00		1.00	1.00		1.00	0.89		1.00	0.94	
Flt Protected	0.95	1.00		0.95	1.00		0.95	1.00		0.95	1.00	
Satd. Flow (prot)	1671	3499		1805	3403		1805	1536		1752	1754	
Flt Permitted	0.13	1.00		0.24	1.00		0.73	1.00		0.41	1.00	
Satd. Flow (perm)	220	3499		453	3403		1380	1536		762	1754	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	30	1057	16	52	1590	12	23	25	68	36	24	16
RTOR Reduction (vph)	0	0	0	0	0	0	0	52	0	0	11	0
Lane Group Flow (vph)	30	1073	0	52	1602	0	23	41	0	36	29	0
Heavy Vehicles (%)	8%	3%	0%	0%	6%	0%	0%	5%	12%	3%	3%	0%
Turn Type	pm+pt	NA		pm+pt	NA		Perm	NA		Perm	NA	
Protected Phases	5	2		1	6			7			3	
Permitted Phases	2			6			7			3		
Actuated Green, G (s)	176.2	170.5		179.4	172.1		22.5	22.5		13.1	13.1	
Effective Green, g (s)	178.2	171.5		181.4	173.1		24.5	24.5		15.1	15.1	
Actuated g/C Ratio	0.81	0.78		0.82	0.79		0.11	0.11		0.07	0.07	
Clearance Time (s)	6.6	6.6		6.6	6.6		6.5	6.5		6.5	6.5	
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Lane Grp Cap (vph)	222	2727		424	2677		153	171		52	120	
v/s Ratio Prot	0.00	0.31		c0.00	c0.47			c0.03			0.02	
v/s Ratio Perm	0.11			0.10			0.02			c0.05		
v/c Ratio	0.14	0.39		0.12	0.60		0.15	0.24		0.69	0.24	
Uniform Delay, d1	7.0	7.7		4.3	9.4		88.3	89.3		100.2	97.0	
Progression Factor	0.15	0.17		0.12	0.24		1.00	1.00		1.00	1.00	
Incremental Delay, d2	0.2	0.4		0.1	0.7		0.5	0.7		33.0	1.0	
Delay (s)	1.3	1.7		0.6	3.0		88.8	90.0		133.1	98.1	
Level of Service	A	A		A	A		F	F		F	F	
Approach Delay (s)		1.7			2.9			89.8			114.7	
Approach LOS		A			A			F			F	
Intersection Summary												
HCM 2000 Control Delay			8.7	HCM 2000 Level of Service				A				
HCM 2000 Volume to Capacity ratio			0.59									
Actuated Cycle Length (s)			220.0	Sum of lost time (s)				20.7				
Intersection Capacity Utilization			57.7%	ICU Level of Service				B				
Analysis Period (min)			15									
c Critical Lane Group												

HCM Signalized Intersection Capacity Analysis

3: Oak Street/Meredith Drive & Fairfax Boulevard
























Total Future AM

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	8	1675	6	7	743	2	11	1	72	32	2	13
Future Volume (vph)	8	1675	6	7	743	2	11	1	72	32	2	13
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	5.6	4.6		5.6	4.6			4.5	4.5		4.5	
Lane Util. Factor	1.00	0.95		1.00	0.95			1.00	1.00		1.00	
Frt	1.00	1.00		1.00	1.00			1.00	0.85		0.96	
Flt Protected	0.95	1.00		0.95	1.00			0.96	1.00		0.97	
Satd. Flow (prot)	1597	3503		1805	3405			1816	1615		1769	
Flt Permitted	0.32	1.00		0.07	1.00			0.96	1.00		0.97	
Satd. Flow (perm)	532	3503		140	3405			1816	1615		1769	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	9	1821	7	8	808	2	12	1	78	35	2	14
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	74	0	8	0
Lane Group Flow (vph)	9	1828	0	8	810	0	0	13	4	0	43	0
Heavy Vehicles (%)	13%	3%	0%	0%	6%	0%	0%	0%	0%	0%	0%	0%
Turn Type	pm+pt	NA		pm+pt	NA		Split	NA	Perm	Split	NA	
Protected Phases	5	2		1	6		4	4		7	7	
Permitted Phases	2			6					4			
Actuated Green, G (s)	138.4	135.6		138.4	135.6			7.5	7.5		8.9	
Effective Green, g (s)	140.4	137.6		140.4	137.6			9.5	9.5		10.9	
Actuated g/C Ratio	0.74	0.72		0.74	0.72			0.05	0.05		0.06	
Clearance Time (s)	6.6	6.6		6.6	6.6			6.5	6.5		6.5	
Vehicle Extension (s)	3.0	3.0		3.0	3.0			3.0	3.0		3.0	
Lane Grp Cap (vph)	414	2536		136	2465			90	80		101	
v/s Ratio Prot	0.00	c0.52		c0.00	0.24			c0.01			c0.02	
v/s Ratio Perm	0.02			0.04					0.00			
v/c Ratio	0.02	0.72		0.06	0.33			0.14	0.05		0.43	
Uniform Delay, d1	6.9	15.1		14.9	9.5			86.4	85.9		86.5	
Progression Factor	0.84	0.33		0.52	0.45			1.00	1.00		1.00	
Incremental Delay, d2	0.0	1.4		0.2	0.3			0.7	0.3		2.9	
Delay (s)	5.8	6.5		8.0	4.6			87.1	86.2		89.5	
Level of Service	A	A		A	A			F	F		F	
Approach Delay (s)		6.5			4.7			86.3			89.5	
Approach LOS		A			A			F			F	
Intersection Summary												
HCM 2000 Control Delay			10.1									B
HCM 2000 Volume to Capacity ratio			0.64									
Actuated Cycle Length (s)			190.0							24.2		
Intersection Capacity Utilization			69.5%									C
Analysis Period (min)			15									
c Critical Lane Group												

HCM Signalized Intersection Capacity Analysis

3: Oak Street/Meredith Drive & Fairfax Boulevard

Total Future PM School

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		 			 				 		 	
Traffic Volume (vph)	23	941	26	83	1304	5	48	19	101	19	5	38
Future Volume (vph)	23	941	26	83	1304	5	48	19	101	19	5	38
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	5.6	4.6		5.6	4.6			4.5	4.5		4.5	
Lane Util. Factor	1.00	0.95		1.00	0.95			1.00	1.00		1.00	
Frt	1.00	1.00		1.00	1.00			1.00	0.85		0.92	
Flt Protected	0.95	1.00		0.95	1.00			0.97	1.00		0.98	
Satd. Flow (prot)	1597	3494		1805	3405			1835	1615		1716	
Flt Permitted	0.14	1.00		0.22	1.00			0.97	1.00		0.98	
Satd. Flow (perm)	236	3494		425	3405			1835	1615		1716	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	25	1023	28	90	1417	5	52	21	110	21	5	41
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	102	0	26	0
Lane Group Flow (vph)	25	1051	0	90	1422	0	0	73	8	0	41	0
Heavy Vehicles (%)	13%	3%	0%	0%	6%	0%	0%	0%	0%	0%	0%	0%
Turn Type	pm+pt	NA		pm+pt	NA		Split	NA	Perm	Split	NA	
Protected Phases	5	2		1	6		4	4		7	7	
Permitted Phases	2			6					4			
Actuated Green, G (s)	157.1	151.3		162.5	154.0			14.0	14.0		11.0	
Effective Green, g (s)	159.1	153.3		164.5	156.0			16.0	16.0		13.0	
Actuated g/C Ratio	0.72	0.70		0.75	0.71			0.07	0.07		0.06	
Clearance Time (s)	6.6	6.6		6.6	6.6			6.5	6.5		6.5	
Vehicle Extension (s)	3.0	3.0		3.0	3.0			3.0	3.0		3.0	
Lane Grp Cap (vph)	212	2434		377	2414			133	117		101	
v/s Ratio Prot	0.00	0.30		c0.01	c0.42			c0.04			c0.02	
v/s Ratio Perm	0.08			0.17					0.00			
v/c Ratio	0.12	0.43		0.24	0.59			0.55	0.07		0.40	
Uniform Delay, d1	12.2	14.5		9.5	16.0			98.5	95.1		99.8	
Progression Factor	0.31	0.23		1.90	1.36			1.00	1.00		1.00	
Incremental Delay, d2	0.2	0.5		0.3	0.9			4.6	0.2		2.6	
Delay (s)	4.0	3.9		18.2	22.7			103.1	95.3		102.4	
Level of Service	A	A		B	C			F	F		F	
Approach Delay (s)		3.9			22.4			98.4			102.4	
Approach LOS		A			C			F			F	
Intersection Summary												
HCM 2000 Control Delay			22.2									C
HCM 2000 Volume to Capacity ratio			0.55									
Actuated Cycle Length (s)			220.0							24.2		
Intersection Capacity Utilization			64.6%									C
Analysis Period (min)			15									
c Critical Lane Group												

HCM Signalized Intersection Capacity Analysis

3: Oak Street/Meredith Drive & Fairfax Boulevard

Total Future PM



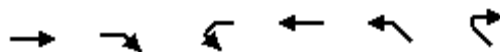
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	14	951	17	93	1596	7	12	10	110	23	4	32
Future Volume (vph)	14	951	17	93	1596	7	12	10	110	23	4	32
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	5.6	4.6		5.6	4.6			4.5	4.5		4.5	
Lane Util. Factor	1.00	0.95		1.00	0.95			1.00	1.00		1.00	
Frt	1.00	1.00		1.00	1.00			1.00	0.85		0.93	
Flt Protected	0.95	1.00		0.95	1.00			0.97	1.00		0.98	
Satd. Flow (prot)	1597	3498		1805	3404			1850	1615		1726	
Flt Permitted	0.09	1.00		0.23	1.00			0.97	1.00		0.98	
Satd. Flow (perm)	153	3498		429	3404			1850	1615		1726	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	15	1034	18	101	1735	8	13	11	120	25	4	35
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	114	0	21	0
Lane Group Flow (vph)	15	1052	0	101	1743	0	0	24	6	0	43	0
Heavy Vehicles (%)	13%	3%	0%	0%	6%	0%	0%	0%	0%	0%	0%	0%
Turn Type	pm+pt	NA		pm+pt	NA		Split	NA	Perm	Split	NA	
Protected Phases	5	2		1	6		4	4		7	7	
Permitted Phases	2			6					4			
Actuated Green, G (s)	160.0	155.8		168.8	160.2			9.2	9.2		11.2	
Effective Green, g (s)	162.0	157.8		170.8	162.2			11.2	11.2		13.2	
Actuated g/C Ratio	0.74	0.72		0.78	0.74			0.05	0.05		0.06	
Clearance Time (s)	6.6	6.6		6.6	6.6			6.5	6.5		6.5	
Vehicle Extension (s)	3.0	3.0		3.0	3.0			3.0	3.0		3.0	
Lane Grp Cap (vph)	146	2509		393	2509			94	82		103	
v/s Ratio Prot	0.00	0.30		c0.01	c0.51			c0.01			c0.03	
v/s Ratio Perm	0.07			0.19					0.00			
v/c Ratio	0.10	0.42		0.26	0.69			0.26	0.07		0.42	
Uniform Delay, d1	14.1	12.6		8.0	15.6			100.4	99.5		99.7	
Progression Factor	0.31	0.21		1.85	1.60			1.00	1.00		1.00	
Incremental Delay, d2	0.3	0.5		0.3	1.2			1.4	0.4		2.8	
Delay (s)	4.6	3.2		15.0	26.1			101.8	99.9		102.5	
Level of Service	A	A		B	C			F	F		F	
Approach Delay (s)		3.2			25.5			100.2			102.5	
Approach LOS		A			C			F			F	

Intersection Summary			
HCM 2000 Control Delay	22.9	HCM 2000 Level of Service	C
HCM 2000 Volume to Capacity ratio	0.63		
Actuated Cycle Length (s)	220.0	Sum of lost time (s)	24.2
Intersection Capacity Utilization	72.5%	ICU Level of Service	C
Analysis Period (min)	15		
c Critical Lane Group			

HCM Unsignalized Intersection Capacity Analysis

4: Site Entrance & Fairfax Boulevard

Total Future AM

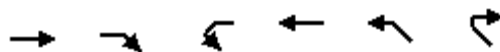


Movement	EBT	EBR	WBL	WBT	NWL	NWR
Lane Configurations	↑↑			↑↑		
Traffic Volume (veh/h)	1758	21	0	752	0	0
Future Volume (Veh/h)	1758	21	0	752	0	0
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	1911	23	0	817	0	0
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	TWLTL		TWLTL			
Median storage (veh)	2		2			
Upstream signal (ft)	173		384			
pX, platoon unblocked			0.69	0.72	0.69	
vC, conflicting volume			1934	2331	967	
vC1, stage 1 conf vol				1922		
vC2, stage 2 conf vol				408		
vCu, unblocked vol			1453	1690	50	
tC, single (s)			4.1	6.8	6.9	
tC, 2 stage (s)				5.8		
tF (s)			2.2	3.5	3.3	
p0 queue free %			100	100	100	
cM capacity (veh/h)			318	124	694	
Direction, Lane #	EB 1	EB 2	WB 1	WB 2		
Volume Total	1274	660	408	408		
Volume Left	0	0	0	0		
Volume Right	0	23	0	0		
cSH	1700	1700	1700	1700		
Volume to Capacity	0.75	0.39	0.24	0.24		
Queue Length 95th (ft)	0	0	0	0		
Control Delay (s)	0.0	0.0	0.0	0.0		
Lane LOS						
Approach Delay (s)	0.0		0.0			
Approach LOS						
Intersection Summary						
Average Delay			0.0			
Intersection Capacity Utilization			52.6%	ICU Level of Service		A
Analysis Period (min)			15			

HCM Unsignalized Intersection Capacity Analysis

4: Site Entrance & Fairfax Boulevard

Total Future PM School

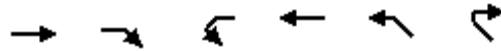


Movement	EBT	EBR	WBL	WBT	NWL	NWR
Lane Configurations	↑↑			↑↑		
Traffic Volume (veh/h)	996	65	0	1392	0	0
Future Volume (Veh/h)	996	65	0	1392	0	0
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	1083	71	0	1513	0	0
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	TWLTL		TWLTL			
Median storage (veh)	2		2			
Upstream signal (ft)	173		384			
pX, platoon unblocked			0.87	0.84	0.87	
vC, conflicting volume			1154	1875	577	
vC1, stage 1 conf vol				1118		
vC2, stage 2 conf vol				756		
vCu, unblocked vol			877	950	214	
tC, single (s)			4.1	6.8	6.9	
tC, 2 stage (s)				5.8		
tF (s)			2.2	3.5	3.3	
p0 queue free %			100	100	100	
cM capacity (veh/h)			666	322	688	
Direction, Lane #	EB 1	EB 2	WB 1	WB 2		
Volume Total	722	432	756	756		
Volume Left	0	0	0	0		
Volume Right	0	71	0	0		
cSH	1700	1700	1700	1700		
Volume to Capacity	0.42	0.25	0.45	0.45		
Queue Length 95th (ft)	0	0	0	0		
Control Delay (s)	0.0	0.0	0.0	0.0		
Lane LOS						
Approach Delay (s)	0.0		0.0			
Approach LOS						
Intersection Summary						
Average Delay			0.0			
Intersection Capacity Utilization			41.8%	ICU Level of Service		A
Analysis Period (min)			15			

HCM Unsignalized Intersection Capacity Analysis

4: Site Entrance & Fairfax Boulevard

Total Future PM



Movement	EBT	EBR	WBL	WBT	NWL	NWR
Lane Configurations	↑↑			↑↑		
Traffic Volume (veh/h)	1010	74	0	1696	0	0
Future Volume (Veh/h)	1010	74	0	1696	0	0
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	1098	80	0	1843	0	0
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	TWLTL		TWLTL			
Median storage (veh)	2		2			
Upstream signal (ft)	173		384			
pX, platoon unblocked			0.88	0.73	0.88	
vC, conflicting volume			1178	2060	589	
vC1, stage 1 conf vol				1138		
vC2, stage 2 conf vol				922		
vCu, unblocked vol			925	957	254	
tC, single (s)			4.1	6.8	6.9	
tC, 2 stage (s)				5.8		
tF (s)			2.2	3.5	3.3	
p0 queue free %			100	100	100	
cM capacity (veh/h)			645	307	654	
Direction, Lane #	EB 1	EB 2	WB 1	WB 2		
Volume Total	732	446	922	922		
Volume Left	0	0	0	0		
Volume Right	0	80	0	0		
cSH	1700	1700	1700	1700		
Volume to Capacity	0.43	0.26	0.54	0.54		
Queue Length 95th (ft)	0	0	0	0		
Control Delay (s)	0.0	0.0	0.0	0.0		
Lane LOS						
Approach Delay (s)	0.0		0.0			
Approach LOS						
Intersection Summary						
Average Delay			0.0			
Intersection Capacity Utilization			50.2%	ICU Level of Service	A	
Analysis Period (min)			15			

HCM Signalized Intersection Capacity Analysis

5: Site Entrance/Fairfax Shoppes Entrance & Fairfax Boulevard

Total Future AM



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↘	↑↑		↘	↑↑			↑	↘		↑	↘
Traffic Volume (vph)	26	1677	2	13	665	9	42	0	47	11	0	18
Future Volume (vph)	26	1677	2	13	665	9	42	0	47	11	0	18
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	5.2		5.0	5.2			6.2	6.2		6.2	6.2
Lane Util. Factor	1.00	0.95		1.00	0.95			1.00	1.00		1.00	1.00
Frt	1.00	1.00		1.00	1.00			1.00	0.85		1.00	0.85
Flt Protected	0.95	1.00		0.95	1.00			0.95	1.00		0.95	1.00
Satd. Flow (prot)	1805	3504		1770	3433			1770	1583		1805	1615
Flt Permitted	0.35	1.00		0.09	1.00			0.95	1.00		0.95	1.00
Satd. Flow (perm)	670	3504		164	3433			1770	1583		1805	1615
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	28	1823	2	14	723	10	46	0	51	12	0	20
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	49	0	0	19
Lane Group Flow (vph)	28	1825	0	14	733	0	0	46	2	0	12	1
Heavy Vehicles (%)	0%	3%	2%	2%	5%	0%	2%	2%	2%	0%	2%	0%
Turn Type	pm+pt	NA		pm+pt	NA		Split	NA	Perm	Split	NA	Perm
Protected Phases	5	2		1	6		4	4		3	3	
Permitted Phases	2			6					4			3
Actuated Green, G (s)	152.0	146.4		149.2	145.0			7.0	7.0		8.8	8.8
Effective Green, g (s)	154.0	147.4		149.2	146.0			7.0	7.0		8.8	8.8
Actuated g/C Ratio	0.81	0.78		0.79	0.77			0.04	0.04		0.05	0.05
Clearance Time (s)	5.0	6.2		5.0	6.2			6.2	6.2		6.2	6.2
Vehicle Extension (s)	3.0	3.0		3.0	3.0			3.0	3.0		3.0	3.0
Lane Grp Cap (vph)	582	2718		164	2637			65	58		83	74
v/s Ratio Prot	0.00	c0.52		c0.00	0.21			c0.03			c0.01	
v/s Ratio Perm	0.04			0.06					0.00			0.00
v/c Ratio	0.05	0.67		0.09	0.28			0.71	0.03		0.14	0.01
Uniform Delay, d1	3.6	10.0		9.4	6.5			90.5	88.2		87.0	86.5
Progression Factor	1.32	0.73		0.97	0.81			1.00	1.00		1.00	1.00
Incremental Delay, d2	0.0	1.0		0.2	0.3			48.9	1.0		3.6	0.3
Delay (s)	4.8	8.3		9.3	5.5			139.4	89.3		90.6	86.8
Level of Service	A	A		A	A			F	F		F	F
Approach Delay (s)		8.2			5.5			113.0			88.2	
Approach LOS		A			A			F			F	

Intersection Summary			
HCM 2000 Control Delay	12.1	HCM 2000 Level of Service	B
HCM 2000 Volume to Capacity ratio	0.63		
Actuated Cycle Length (s)	190.0	Sum of lost time (s)	22.6
Intersection Capacity Utilization	72.8%	ICU Level of Service	C
Analysis Period (min)	15		
c Critical Lane Group			

HCM Signalized Intersection Capacity Analysis
 5: Site Entrance/Fairfax Shoppes Entrance & Fairfax Boulevard

Total Future PM School



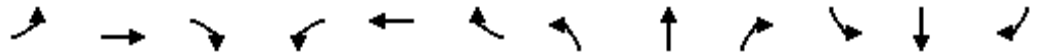
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	44	853	7	36	1217	29	64	0	26	57	0	57
Future Volume (vph)	44	853	7	36	1217	29	64	0	26	57	0	57
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	5.2		5.0	5.2			6.2	6.2		6.2	6.2
Lane Util. Factor	1.00	0.95		1.00	0.95			1.00	1.00		1.00	1.00
Frt	1.00	1.00		1.00	1.00			1.00	0.85		1.00	0.85
Flt Protected	0.95	1.00		0.95	1.00			0.95	1.00		0.95	1.00
Satd. Flow (prot)	1805	3501		1770	3430			1770	1583		1805	1615
Flt Permitted	0.14	1.00		0.26	1.00			0.95	1.00		0.95	1.00
Satd. Flow (perm)	258	3501		484	3430			1770	1583		1805	1615
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	48	927	8	39	1323	32	70	0	28	62	0	62
RTOR Reduction (vph)	0	0	0	0	1	0	0	0	25	0	0	56
Lane Group Flow (vph)	48	935	0	39	1354	0	0	70	3	0	62	6
Heavy Vehicles (%)	0%	3%	2%	2%	5%	0%	2%	2%	2%	0%	2%	0%
Turn Type	pm+pt	NA		pm+pt	NA		Split	NA	Perm	Split	NA	Perm
Protected Phases	5	2		1	6		4	4		3	3	
Permitted Phases	2			6					4			3
Actuated Green, G (s)	151.4	143.9		148.2	142.3			24.8	24.8		21.8	21.8
Effective Green, g (s)	153.4	144.9		148.2	143.3			24.8	24.8		21.8	21.8
Actuated g/C Ratio	0.70	0.66		0.67	0.65			0.11	0.11		0.10	0.10
Clearance Time (s)	5.0	6.2		5.0	6.2			6.2	6.2		6.2	6.2
Vehicle Extension (s)	3.0	3.0		3.0	3.0			3.0	3.0		3.0	3.0
Lane Grp Cap (vph)	239	2305		360	2234			199	178		178	160
v/s Ratio Prot	c0.01	0.27		0.00	c0.39			c0.04			c0.03	
v/s Ratio Perm	0.13			0.07					0.00			0.00
v/c Ratio	0.20	0.41		0.11	0.61			0.35	0.02		0.35	0.04
Uniform Delay, d1	16.0	17.5		13.1	22.1			90.2	86.8		92.5	89.6
Progression Factor	0.70	0.97		1.32	1.97			1.00	1.00		1.00	1.00
Incremental Delay, d2	0.4	0.5		0.1	0.9			4.8	0.2		5.3	0.4
Delay (s)	11.6	17.5		17.4	44.4			95.0	87.0		97.8	90.1
Level of Service	B	B		B	D			F	F		F	F
Approach Delay (s)		17.2			43.6			92.7			93.9	
Approach LOS		B			D			F			F	

Intersection Summary			
HCM 2000 Control Delay	37.9	HCM 2000 Level of Service	D
HCM 2000 Volume to Capacity ratio	0.53		
Actuated Cycle Length (s)	220.0	Sum of lost time (s)	22.6
Intersection Capacity Utilization	60.9%	ICU Level of Service	B
Analysis Period (min)	15		
c Critical Lane Group			

HCM Signalized Intersection Capacity Analysis

5: Site Entrance/Fairfax Shoppes Entrance & Fairfax Boulevard

Total Future PM



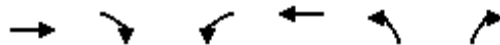
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↘	↑↑		↘	↑↑			↑	↗		↘	↗
Traffic Volume (vph)	37	1002	9	45	1510	29	61	0	25	64	0	31
Future Volume (vph)	37	1002	9	45	1510	29	61	0	25	64	0	31
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	5.2		5.0	5.2			6.2	6.2		6.2	6.2
Lane Util. Factor	1.00	0.95		1.00	0.95			1.00	1.00		1.00	1.00
Frt	1.00	1.00		1.00	1.00			1.00	0.85		1.00	0.85
Flt Protected	0.95	1.00		0.95	1.00			0.95	1.00		0.95	1.00
Satd. Flow (prot)	1805	3500		1770	3431			1770	1583		1805	1615
Flt Permitted	0.08	1.00		0.20	1.00			0.95	1.00		0.95	1.00
Satd. Flow (perm)	151	3500		373	3431			1770	1583		1805	1615
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	40	1089	10	49	1641	32	66	0	27	70	0	34
RTOR Reduction (vph)	0	0	0	0	1	0	0	0	24	0	0	31
Lane Group Flow (vph)	40	1099	0	49	1672	0	0	66	3	0	70	3
Heavy Vehicles (%)	0%	3%	2%	2%	5%	0%	2%	2%	2%	0%	2%	0%
Turn Type	pm+pt	NA		pm+pt	NA		Split	NA	Perm	Split	NA	Perm
Protected Phases	5	2		1	6		4	4		3	3	
Permitted Phases	2			6					4			3
Actuated Green, G (s)	148.1	142.2		151.5	143.9			24.8	24.8		21.8	21.8
Effective Green, g (s)	150.1	143.2		151.5	144.9			24.8	24.8		21.8	21.8
Actuated g/C Ratio	0.68	0.65		0.69	0.66			0.11	0.11		0.10	0.10
Clearance Time (s)	5.0	6.2		5.0	6.2			6.2	6.2		6.2	6.2
Vehicle Extension (s)	3.0	3.0		3.0	3.0			3.0	3.0		3.0	3.0
Lane Grp Cap (vph)	154	2278		305	2259			199	178		178	160
v/s Ratio Prot	c0.01	0.31		0.01	c0.49			c0.04			c0.04	
v/s Ratio Perm	0.17			0.10					0.00			0.00
v/c Ratio	0.26	0.48		0.16	0.74			0.33	0.02		0.39	0.02
Uniform Delay, d1	22.6	19.5		13.7	25.0			90.0	86.8		92.9	89.5
Progression Factor	0.79	1.32		0.71	1.70			1.00	1.00		1.00	1.00
Incremental Delay, d2	0.8	0.7		0.1	0.8			4.4	0.2		6.4	0.2
Delay (s)	18.6	26.5		9.8	43.3			94.4	86.9		99.3	89.7
Level of Service	B	C		A	D			F	F		F	F
Approach Delay (s)		26.2			42.4			92.2			96.2	
Approach LOS		C			D			F			F	

Intersection Summary			
HCM 2000 Control Delay	39.7	HCM 2000 Level of Service	D
HCM 2000 Volume to Capacity ratio	0.64		
Actuated Cycle Length (s)	220.0	Sum of lost time (s)	22.6
Intersection Capacity Utilization	69.0%	ICU Level of Service	C
Analysis Period (min)	15		
c Critical Lane Group			

HCM Unsignalized Intersection Capacity Analysis

6: Site Entrance & Fairfax Boulevard

Total Future AM



Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑		↵	↑↑	↵	
Traffic Volume (veh/h)	1732	5	27	675	24	73
Future Volume (Veh/h)	1732	5	27	675	24	73
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	1883	5	29	734	26	79
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	TWLTL		TWLTL			
Median storage veh)	2		2			
Upstream signal (ft)	378		458			
pX, platoon unblocked			0.74		0.79	0.74
vC, conflicting volume			1888		2310	944
vC1, stage 1 conf vol					1886	
vC2, stage 2 conf vol					425	
vCu, unblocked vol			1495		1513	217
tC, single (s)			4.1		6.8	6.9
tC, 2 stage (s)					5.8	
tF (s)			2.2		3.5	3.3
p0 queue free %			91		79	86
cM capacity (veh/h)			329		124	582
Direction, Lane #	EB 1	EB 2	WB 1	WB 2	WB 3	NB 1
Volume Total	1255	633	29	367	367	105
Volume Left	0	0	29	0	0	26
Volume Right	0	5	0	0	0	79
cSH	1700	1700	329	1700	1700	305
Volume to Capacity	0.74	0.37	0.09	0.22	0.22	0.34
Queue Length 95th (ft)	0	0	7	0	0	37
Control Delay (s)	0.0	0.0	17.0	0.0	0.0	22.9
Lane LOS			C			C
Approach Delay (s)	0.0		0.6			22.9
Approach LOS						C
Intersection Summary						
Average Delay			1.1			
Intersection Capacity Utilization			60.5%	ICU Level of Service	B	
Analysis Period (min)			15			

HCM Unsignalized Intersection Capacity Analysis

6: Site Entrance & Fairfax Boulevard

Total Future PM School



Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑		↵	↑↑	↵	
Traffic Volume (veh/h)	922	15	75	1300	25	46
Future Volume (Veh/h)	922	15	75	1300	25	46
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	1002	16	82	1413	27	50
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	TWLTL		TWLTL			
Median storage veh)	2		2			
Upstream signal (ft)	378		458			
pX, platoon unblocked			0.88		0.78	0.88
vC, conflicting volume			1018		1880	509
vC1, stage 1 conf vol					1010	
vC2, stage 2 conf vol					870	
vCu, unblocked vol			736		851	155
tC, single (s)			4.1		6.8	6.9
tC, 2 stage (s)					5.8	
tF (s)			2.2		3.5	3.3
p0 queue free %			89		92	93
cM capacity (veh/h)			757		356	756
Direction, Lane #	EB 1	EB 2	WB 1	WB 2	WB 3	NB 1
Volume Total	668	350	82	706	706	77
Volume Left	0	0	82	0	0	27
Volume Right	0	16	0	0	0	50
cSH	1700	1700	757	1700	1700	543
Volume to Capacity	0.39	0.21	0.11	0.42	0.42	0.14
Queue Length 95th (ft)	0	0	9	0	0	12
Control Delay (s)	0.0	0.0	10.3	0.0	0.0	12.7
Lane LOS			B			B
Approach Delay (s)	0.0		0.6			12.7
Approach LOS						B
Intersection Summary						
Average Delay			0.7			
Intersection Capacity Utilization			46.8%	ICU Level of Service	A	
Analysis Period (min)			15			

HCM Unsignalized Intersection Capacity Analysis

6: Site Entrance & Fairfax Boulevard

Total Future PM



Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑		↵	↑↑	↵	
Traffic Volume (veh/h)	1072	21	99	1659	23	44
Future Volume (Veh/h)	1072	21	99	1659	23	44
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	1165	23	108	1803	25	48
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	TWLTL		TWLTL			
Median storage veh)	2		2			
Upstream signal (ft)	378		458			
pX, platoon unblocked			0.84		0.54	0.84
vC, conflicting volume			1188		2294	594
vC1, stage 1 conf vol					1176	
vC2, stage 2 conf vol					1118	
vCu, unblocked vol			841		350	133
tC, single (s)			4.1		6.8	6.9
tC, 2 stage (s)					5.8	
tF (s)			2.2		3.5	3.3
p0 queue free %			84		92	94
cM capacity (veh/h)			663		314	748
Direction, Lane #	EB 1	EB 2	WB 1	WB 2	WB 3	NB 1
Volume Total	777	411	108	902	902	73
Volume Left	0	0	108	0	0	25
Volume Right	0	23	0	0	0	48
cSH	1700	1700	663	1700	1700	508
Volume to Capacity	0.46	0.24	0.16	0.53	0.53	0.14
Queue Length 95th (ft)	0	0	14	0	0	12
Control Delay (s)	0.0	0.0	11.5	0.0	0.0	13.3
Lane LOS			B			B
Approach Delay (s)	0.0		0.6			13.3
Approach LOS						B
Intersection Summary						
Average Delay			0.7			
Intersection Capacity Utilization			56.5%		ICU Level of Service	B
Analysis Period (min)			15			

HCM Unsignalized Intersection Capacity Analysis

7: Frontage Road & Fairfax Boulevard

Total Future AM

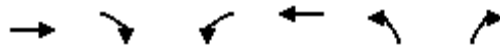


Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑			↑↑		↗
Traffic Volume (veh/h)	1805	0	0	702	0	4
Future Volume (Veh/h)	1805	0	0	702	0	4
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	1962	0	0	763	0	4
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	TWLTL		TWLTL			
Median storage (veh)	2		2			
Upstream signal (ft)	536		300			
pX, platoon unblocked			0.73		0.79	0.73
vC, conflicting volume			1962		2344	981
vC1, stage 1 conf vol					1962	
vC2, stage 2 conf vol					382	
vCu, unblocked vol			1585		1528	247
tC, single (s)			4.1		6.8	6.9
tC, 2 stage (s)					5.8	
tF (s)			2.2		3.5	3.3
p0 queue free %			100		100	99
cM capacity (veh/h)			301		110	552
Direction, Lane #	EB 1	EB 2	WB 1	WB 2	NB 1	
Volume Total	981	981	382	382	4	
Volume Left	0	0	0	0	0	
Volume Right	0	0	0	0	4	
cSH	1700	1700	1700	1700	552	
Volume to Capacity	0.58	0.58	0.22	0.22	0.01	
Queue Length 95th (ft)	0	0	0	0	1	
Control Delay (s)	0.0	0.0	0.0	0.0	11.6	
Lane LOS						B
Approach Delay (s)	0.0		0.0		11.6	
Approach LOS						B
Intersection Summary						
Average Delay			0.0			
Intersection Capacity Utilization			59.9%	ICU Level of Service	B	
Analysis Period (min)			15			

HCM Unsignalized Intersection Capacity Analysis

7: Frontage Road & Fairfax Boulevard

Total Future PM School

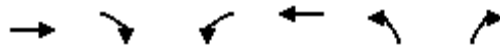


Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑			↑↑		↗
Traffic Volume (veh/h)	968	0	0	1375	0	28
Future Volume (Veh/h)	968	0	0	1375	0	28
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	1052	0	0	1495	0	30
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	TWLTL		TWLTL			
Median storage (veh)	2		2			
Upstream signal (ft)	536		300			
pX, platoon unblocked			0.88		0.78	0.88
vC, conflicting volume			1052		1800	526
vC1, stage 1 conf vol					1052	
vC2, stage 2 conf vol					748	
vCu, unblocked vol			777		743	177
tC, single (s)			4.1		6.8	6.9
tC, 2 stage (s)					5.8	
tF (s)			2.2		3.5	3.3
p0 queue free %			100		100	96
cM capacity (veh/h)			732		351	732
Direction, Lane #	EB 1	EB 2	WB 1	WB 2	NB 1	
Volume Total	526	526	748	748	30	
Volume Left	0	0	0	0	0	
Volume Right	0	0	0	0	30	
cSH	1700	1700	1700	1700	732	
Volume to Capacity	0.31	0.31	0.44	0.44	0.04	
Queue Length 95th (ft)	0	0	0	0	3	
Control Delay (s)	0.0	0.0	0.0	0.0	10.1	
Lane LOS						B
Approach Delay (s)	0.0		0.0		10.1	
Approach LOS						B
Intersection Summary						
Average Delay			0.1			
Intersection Capacity Utilization			41.3%	ICU Level of Service	A	
Analysis Period (min)			15			

HCM Unsignalized Intersection Capacity Analysis

7: Frontage Road & Fairfax Boulevard

Total Future PM


















Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑			↑↑		↗
Traffic Volume (veh/h)	1116	0	0	1758	0	27
Future Volume (Veh/h)	1116	0	0	1758	0	27
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	1213	0	0	1911	0	29
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	TWLTL		TWLTL			
Median storage (veh)	2		2			
Upstream signal (ft)	536		300			
pX, platoon unblocked			0.84	0.53	0.84	
vC, conflicting volume			1213	2168	606	
vC1, stage 1 conf vol				1213		
vC2, stage 2 conf vol				956		
vCu, unblocked vol			872	98	150	
tC, single (s)			4.1	6.8	6.9	
tC, 2 stage (s)				5.8		
tF (s)			2.2	3.5	3.3	
p0 queue free %			100	100	96	
cM capacity (veh/h)			646	476	730	
Direction, Lane #	EB 1	EB 2	WB 1	WB 2	NB 1	
Volume Total	606	606	956	956	29	
Volume Left	0	0	0	0	0	
Volume Right	0	0	0	0	29	
cSH	1700	1700	1700	1700	730	
Volume to Capacity	0.36	0.36	0.56	0.56	0.04	
Queue Length 95th (ft)	0	0	0	0	3	
Control Delay (s)	0.0	0.0	0.0	0.0	10.1	
Lane LOS						B
Approach Delay (s)	0.0		0.0		10.1	
Approach LOS						B
Intersection Summary						
Average Delay			0.1			
Intersection Capacity Utilization			51.9%	ICU Level of Service	A	
Analysis Period (min)			15			

HCM Signalized Intersection Capacity Analysis

8: Fairfax Boulevard & McLean Avenue & Warwick Road



















Total Future AM

												
Movement	EBL2	EBL	EBT	EBR2	WBL	WBT	WBR	WBR2	NBL2	NBL	NBT	NBR
Lane Configurations												
Traffic Volume (vph)	3	98	26	2	14	6	7	3	12	2	4	35
Future Volume (vph)	3	98	26	2	14	6	7	3	12	2	4	35
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)			5.0			5.0					5.0	
Lane Util. Factor			1.00			1.00					1.00	
Frt			1.00			0.95					0.91	
Flt Protected			0.96			0.98					0.99	
Satd. Flow (prot)			1477			1719					1707	
Flt Permitted			0.96			0.98					0.91	
Satd. Flow (perm)			1477			1719					1578	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	3	107	28	2	15	7	8	3	13	2	4	38
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	0	0	140	0	0	33	0	0	0	0	57	0
Heavy Vehicles (%)	100%	0%	100%	100%	7%	0%	0%	0%	0%	0%	0%	0%
Turn Type	Perm	Split	NA		Split	NA			Perm	Perm	NA	
Protected Phases		3	3		4	4					7	
Permitted Phases	3								7	7		
Actuated Green, G (s)			19.7			8.0					12.3	
Effective Green, g (s)			21.2			9.5					13.8	
Actuated g/C Ratio			0.11			0.05					0.07	
Clearance Time (s)			6.5			6.5					6.5	
Vehicle Extension (s)			3.0			3.0					3.0	
Lane Grp Cap (vph)			164			85					114	
v/s Ratio Prot			c0.09			c0.02						
v/s Ratio Perm											c0.04	
v/c Ratio			0.85			0.39					0.50	
Uniform Delay, d1			82.9			87.4					84.8	
Progression Factor			1.00			1.00					1.00	
Incremental Delay, d2			32.6			2.9					3.4	
Delay (s)			115.4			90.4					88.2	
Level of Service			F			F					F	
Approach Delay (s)			115.4			90.4					88.2	
Approach LOS			F			F					F	
Intersection Summary												
HCM 2000 Control Delay			27.0			HCM 2000 Level of Service					C	
HCM 2000 Volume to Capacity ratio			0.80									
Actuated Cycle Length (s)			190.0			Sum of lost time (s)			25.7			
Intersection Capacity Utilization			83.1%			ICU Level of Service			E			
Analysis Period (min)			15									
c Critical Lane Group												

HCM Signalized Intersection Capacity Analysis

8: Fairfax Boulevard & McLean Avenue & Warwick Road

Total Future AM

												
Movement	SBL2	SBL	SBT	SBR	SBR2	NEL	NET	NER	NER2	SWL2	SWL	SWT
Lane Configurations												
Traffic Volume (vph)	25	3	5	2	4	3	1744	26	7	1	16	673
Future Volume (vph)	25	3	5	2	4	3	1744	26	7	1	16	673
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		5.0	5.0			5.6	5.1				5.6	5.1
Lane Util. Factor		1.00	1.00			1.00	0.95				1.00	0.95
Frt		1.00	0.92			1.00	1.00				1.00	0.99
Flt Protected		0.95	1.00			0.95	1.00				0.95	1.00
Satd. Flow (prot)		1805	1745			1752	3600				1805	3476
Flt Permitted		0.62	1.00			0.30	1.00				0.03	1.00
Satd. Flow (perm)		1169	1745			557	3600				65	3476
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	27	3	5	2	4	3	1896	28	8	1	17	732
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	0	30	11	0	0	3	1932	0	0	0	18	808
Heavy Vehicles (%)	0%	0%	0%	0%	0%	3%	0%	0%	0%	0%	0%	2%
Turn Type	Perm	Perm	NA			pm+pt	NA			pm+pt	pm+pt	NA
Protected Phases			7			1	6			5	5	2
Permitted Phases	7	7				6				2	2	
Actuated Green, G (s)		12.3	12.3			113.5	112.3				119.1	115.1
Effective Green, g (s)		13.8	13.8			116.5	114.3				122.1	117.1
Actuated g/C Ratio		0.07	0.07			0.61	0.60				0.64	0.62
Clearance Time (s)		6.5	6.5			7.1	7.1				7.1	7.1
Vehicle Extension (s)		3.0	3.0			3.0	4.0				3.0	4.0
Lane Grp Cap (vph)		84	126			358	2165				92	2142
v/s Ratio Prot			0.01			0.00	c0.54				c0.01	c0.23
v/s Ratio Perm		0.03				0.01					0.12	
v/c Ratio		0.36	0.09			0.01	0.89				0.20	0.38
Uniform Delay, d1		83.9	82.2			14.8	32.6				37.7	18.2
Progression Factor		1.00	1.00			0.30	0.46				1.00	1.00
Incremental Delay, d2		2.6	0.3			0.0	4.9				1.0	0.5
Delay (s)		86.5	82.5			4.5	19.8				38.8	18.7
Level of Service		F	F			A	B				D	B
Approach Delay (s)			85.4				19.8					19.2
Approach LOS			F				B					B

Intersection Summary

HCM Signalized Intersection Capacity Analysis
 8: Fairfax Boulevard & McLean Avenue & Warwick Road

















Total Future AM



Movement	SWR	SWR2
Lane Configurations		
Traffic Volume (vph)	61	9
Future Volume (vph)	61	9
Ideal Flow (vphpl)	1900	1900
Total Lost time (s)		
Lane Util. Factor		
Fr		
Flt Protected		
Satd. Flow (prot)		
Flt Permitted		
Satd. Flow (perm)		
Peak-hour factor, PHF	0.92	0.92
Adj. Flow (vph)	66	10
RTOR Reduction (vph)	0	0
Lane Group Flow (vph)	0	0
Heavy Vehicles (%)	7%	0%
Turn Type		
Protected Phases		
Permitted Phases		
Actuated Green, G (s)		
Effective Green, g (s)		
Actuated g/C Ratio		
Clearance Time (s)		
Vehicle Extension (s)		
Lane Grp Cap (vph)		
v/s Ratio Prot		
v/s Ratio Perm		
v/c Ratio		
Uniform Delay, d1		
Progression Factor		
Incremental Delay, d2		
Delay (s)		
Level of Service		
Approach Delay (s)		
Approach LOS		
Intersection Summary		


















HCM Signalized Intersection Capacity Analysis
 8: Fairfax Boulevard & McLean Avenue & Warwick Road

Total Future PM School

													
Movement	EBL2	EBL	EBT	EBR	WBL2	WBL	WBT	WBR	WBR2	NBL2	NBL	NBT	
Lane Configurations													
Traffic Volume (vph)	4	73	19	3	1	4	26	10	15	5	8	4	
Future Volume (vph)	4	73	19	3	1	4	26	10	15	5	8	4	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Total Lost time (s)			5.0				5.0					5.0	
Lane Util. Factor			1.00				1.00					1.00	
Frt			1.00				0.94					0.95	
Flt Protected			0.96				1.00					0.98	
Satd. Flow (prot)			1444				1769					1756	
Flt Permitted			0.96				1.00					0.84	
Satd. Flow (perm)			1444				1769					1518	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	4	79	21	3	1	4	28	11	16	5	9	4	
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	0	0	0	0	
Lane Group Flow (vph)	0	0	107	0	0	0	60	0	0	0	0	30	
Heavy Vehicles (%)	100%	0%	100%	100%	0%	7%	0%	0%	0%	0%	0%	0%	
Turn Type	Perm	Split	NA		Perm	Split	NA			Perm	Perm	NA	
Protected Phases		3	3			4	4					7	
Permitted Phases	3				4					7	7		
Actuated Green, G (s)			20.1				12.9					8.6	
Effective Green, g (s)			21.6				14.4					10.1	
Actuated g/C Ratio			0.10				0.07					0.05	
Clearance Time (s)			6.5				6.5					6.5	
Vehicle Extension (s)			3.0				3.0					3.0	
Lane Grp Cap (vph)			141				115					69	
v/s Ratio Prot			c0.07				c0.03						
v/s Ratio Perm												c0.02	
v/c Ratio			0.76				0.52					0.43	
Uniform Delay, d1			96.7				99.5					102.2	
Progression Factor			1.00				1.00					1.00	
Incremental Delay, d2			20.6				4.2					4.3	
Delay (s)			117.3				103.7					106.5	
Level of Service			F				F					F	
Approach Delay (s)			117.3				103.7					106.5	
Approach LOS			F				F					F	
Intersection Summary													
HCM 2000 Control Delay			28.6				HCM 2000 Level of Service			C			
HCM 2000 Volume to Capacity ratio			0.65										
Actuated Cycle Length (s)			220.0				Sum of lost time (s)		25.7				
Intersection Capacity Utilization			73.2%				ICU Level of Service		D				
Analysis Period (min)			15										
c Critical Lane Group													

HCM Signalized Intersection Capacity Analysis
 8: Fairfax Boulevard & McLean Avenue & Warwick Road

Total Future PM School

												
Movement	NBR	SBL2	SBL	SBT	SBR	SBR2	NEL2	NEL	NET	NER	NER2	SWL2
Lane Configurations												
Traffic Volume (vph)	11	17	7	6	5	1	5	5	910	51	11	3
Future Volume (vph)	11	17	7	6	5	1	5	5	910	51	11	3
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)			5.0	5.0				5.6	5.1			
Lane Util. Factor			1.00	1.00				1.00	0.95			
Frt			1.00	0.93				1.00	0.99			
Flt Protected			0.95	1.00				0.95	1.00			
Satd. Flow (prot)			1805	1768				1778	3576			
Flt Permitted			0.77	1.00				0.10	1.00			
Satd. Flow (perm)			1461	1768				181	3576			
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	12	18	8	7	5	1	5	5	989	55	12	3
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	0	0	26	13	0	0	0	10	1056	0	0	0
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	0%	3%	0%	0%	0%	0%
Turn Type		Perm	Perm	NA			custom	pm+pt	NA			pm+pt
Protected Phases				7				1	6			5
Permitted Phases		7	7				1	6				2
Actuated Green, G (s)			8.6	8.6				141.9	139.3			
Effective Green, g (s)			10.1	10.1				144.9	141.3			
Actuated g/C Ratio			0.05	0.05				0.66	0.64			
Clearance Time (s)			6.5	6.5				7.1	7.1			
Vehicle Extension (s)			3.0	3.0				3.0	4.0			
Lane Grp Cap (vph)			67	81				148	2296			
v/s Ratio Prot				0.01				0.00	0.30			
v/s Ratio Perm			0.02					0.04				
v/c Ratio			0.39	0.16				0.07	0.46			
Uniform Delay, d1			101.9	100.9				20.4	20.0			
Progression Factor			1.00	1.00				1.19	0.74			
Incremental Delay, d2			3.7	0.9				0.2	0.6			
Delay (s)			105.6	101.8				24.5	15.4			
Level of Service			F	F				C	B			
Approach Delay (s)				104.4					15.5			
Approach LOS				F					B			
Intersection Summary												

HCM Signalized Intersection Capacity Analysis

8: Fairfax Boulevard & McLean Avenue & Warwick Road

Total Future PM School



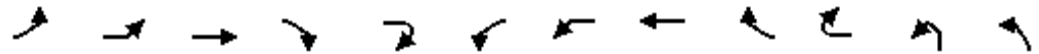
Movement	SWL	SWT	SWR	SWR2
Lane Configurations	↙	↑↘		
Traffic Volume (vph)	18	1362	68	14
Future Volume (vph)	18	1362	68	14
Ideal Flow (vphpl)	1900	1900	1900	1900
Total Lost time (s)	5.6	5.1		
Lane Util. Factor	1.00	0.95		
Frt	1.00	0.99		
Flt Protected	0.95	1.00		
Satd. Flow (prot)	1805	3502		
Flt Permitted	0.21	1.00		
Satd. Flow (perm)	397	3502		
Peak-hour factor, PHF	0.92	0.92	0.92	0.92
Adj. Flow (vph)	20	1480	74	15
RTOR Reduction (vph)	0	0	0	0
Lane Group Flow (vph)	23	1569	0	0
Heavy Vehicles (%)	0%	2%	7%	0%
Turn Type	pm+pt	NA		
Protected Phases	5	2		
Permitted Phases	2			
Actuated Green, G (s)	147.5	142.1		
Effective Green, g (s)	150.5	144.1		
Actuated g/C Ratio	0.68	0.65		
Clearance Time (s)	7.1	7.1		
Vehicle Extension (s)	3.0	4.0		
Lane Grp Cap (vph)	315	2293		
v/s Ratio Prot	c0.00	c0.45		
v/s Ratio Perm	0.05			
v/c Ratio	0.07	0.68		
Uniform Delay, d1	13.7	23.7		
Progression Factor	1.00	1.00		
Incremental Delay, d2	0.1	1.7		
Delay (s)	13.8	25.4		
Level of Service	B	C		
Approach Delay (s)		25.2		
Approach LOS		C		

Intersection Summary

HCM Signalized Intersection Capacity Analysis

8: Fairfax Boulevard & McLean Avenue & Warwick Road

Total Future PM



Movement	EBL2	EBL	EBT	EBR	EBR2	WBL2	WBL	WBT	WBR	WBR2	NBL2	NBL
Lane Configurations			↔					↔				
Traffic Volume (vph)	5	61	31	4	2	1	4	30	12	5	18	16
Future Volume (vph)	5	61	31	4	2	1	4	30	12	5	18	16
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)			5.0					5.0				
Lane Util. Factor			1.00					1.00				
Frt			0.99					0.96				
Flt Protected			0.97					1.00				
Satd. Flow (prot)			1300					1800				
Flt Permitted			0.97					1.00				
Satd. Flow (perm)			1300					1800				
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	5	66	34	4	2	1	4	33	13	5	20	17
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	0	0	111	0	0	0	0	56	0	0	0	0
Heavy Vehicles (%)	100%	0%	100%	100%	100%	0%	7%	0%	0%	0%	0%	0%
Turn Type	Perm	Split	NA			Perm	Split	NA			Perm	Perm
Protected Phases		3	3				4	4				
Permitted Phases	3					4					7	7
Actuated Green, G (s)			21.5					12.3				
Effective Green, g (s)			23.0					13.8				
Actuated g/C Ratio			0.10					0.06				
Clearance Time (s)			6.5					6.5				
Vehicle Extension (s)			3.0					3.0				
Lane Grp Cap (vph)			135					112				
v/s Ratio Prot			c0.09					c0.03				
v/s Ratio Perm												
v/c Ratio			0.82					0.50				
Uniform Delay, d1			96.5					99.8				
Progression Factor			1.00					1.00				
Incremental Delay, d2			31.5					3.5				
Delay (s)			128.0					103.2				
Level of Service			F					F				
Approach Delay (s)			128.0					103.2				
Approach LOS			F					F				
Intersection Summary												
HCM 2000 Control Delay			50.3					HCM 2000 Level of Service			D	
HCM 2000 Volume to Capacity ratio			0.86									
Actuated Cycle Length (s)			220.0					Sum of lost time (s)		25.7		
Intersection Capacity Utilization			87.8%					ICU Level of Service		E		
Analysis Period (min)			15									
c Critical Lane Group												

HCM Signalized Intersection Capacity Analysis
 8: Fairfax Boulevard & McLean Avenue & Warwick Road

Total Future PM

	↑	↗	↘	↓	↙	↘	↗	↗	↘	↗	↘	↘
Movement	NBT	NBR	SBL2	SBL	SBT	SBR	SBR2	NEL2	NEL	NET	NER	NER2
Lane Configurations	↕			↗	↘				↗	↕		
Traffic Volume (vph)	20	38	11	1	6	2	2	12	12	944	33	9
Future Volume (vph)	20	38	11	1	6	2	2	12	12	944	33	9
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	5.0			5.0	5.0				5.6	5.1		
Lane Util. Factor	1.00			1.00	1.00				1.00	0.95		
Frt	0.94			1.00	0.95				1.00	0.99		
Flt Protected	0.98			0.95	1.00				0.95	1.00		
Satd. Flow (prot)	1762			1805	1796				1778	3587		
Flt Permitted	0.87			0.49	1.00				0.03	1.00		
Satd. Flow (perm)	1568			934	1796				57	3587		
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	22	41	12	1	7	2	2	13	13	1026	36	10
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	100	0	0	13	11	0	0	0	26	1072	0	0
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	0%	0%	3%	0%	0%	0%
Turn Type	NA		Perm	Perm	NA			custom	pm+pt	NA		
Protected Phases	7				7				1	6		
Permitted Phases			7	7				1	6			
Actuated Green, G (s)	17.8			17.8	17.8				134.8	129.2		
Effective Green, g (s)	19.3			19.3	19.3				137.8	131.2		
Actuated g/C Ratio	0.09			0.09	0.09				0.63	0.60		
Clearance Time (s)	6.5			6.5	6.5				7.1	7.1		
Vehicle Extension (s)	3.0			3.0	3.0				3.0	4.0		
Lane Grp Cap (vph)	137			81	157				91	2139		
v/s Ratio Prot					0.01				c0.01	0.30		
v/s Ratio Perm	c0.06			0.01					0.17			
v/c Ratio	0.73			0.16	0.07				0.29	0.50		
Uniform Delay, d1	97.8			92.9	92.1				48.9	25.6		
Progression Factor	1.00			1.00	1.00				2.65	0.99		
Incremental Delay, d2	17.6			0.9	0.2				1.6	0.8		
Delay (s)	115.4			93.8	92.3				131.2	26.2		
Level of Service	F			F	F				F	C		
Approach Delay (s)	115.4				93.1					28.7		
Approach LOS	F				F					C		

Intersection Summary

HCM Signalized Intersection Capacity Analysis

8: Fairfax Boulevard & McLean Avenue & Warwick Road

Total Future PM



Movement	SWL2	SWL	SWT	SWR	SWR2
Lane Configurations		↶	↷		
Traffic Volume (vph)	5	17	1732	93	5
Future Volume (vph)	5	17	1732	93	5
Ideal Flow (vphpl)	1900	1900	1900	1900	1900
Total Lost time (s)		5.6	5.1		
Lane Util. Factor		1.00	0.95		
Frt		1.00	0.99		
Flt Protected		0.95	1.00		
Satd. Flow (prot)		1805	3502		
Flt Permitted		0.19	1.00		
Satd. Flow (perm)		370	3502		
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	5	18	1883	101	5
RTOR Reduction (vph)	0	0	0	0	0
Lane Group Flow (vph)	0	23	1989	0	0
Heavy Vehicles (%)	0%	0%	2%	7%	0%
Turn Type	pm+pt	pm+pt	NA		
Protected Phases	5	5	2		
Permitted Phases	2	2			
Actuated Green, G (s)		134.6	129.1		
Effective Green, g (s)		137.6	131.1		
Actuated g/C Ratio		0.63	0.60		
Clearance Time (s)		7.1	7.1		
Vehicle Extension (s)		3.0	4.0		
Lane Grp Cap (vph)		277	2086		
v/s Ratio Prot		0.00	c0.57		
v/s Ratio Perm		0.05			
v/c Ratio		0.08	0.95		
Uniform Delay, d1		18.6	41.6		
Progression Factor		1.00	1.00		
Incremental Delay, d2		0.1	11.4		
Delay (s)		18.7	53.0		
Level of Service		B	D		
Approach Delay (s)			52.6		
Approach LOS			D		
Intersection Summary					

HCM Unsignalized Intersection Capacity Analysis

9: Walnut Street & Cedar Avenue

Total Future AM













Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	2	10	87	1	20	53
Future Volume (Veh/h)	2	10	87	1	20	53
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	2	11	95	1	22	58
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)		3				
Median type			None			None
Median storage (veh)						
Upstream signal (ft)						366
pX, platoon unblocked						
vC, conflicting volume	198	96			96	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	198	96			96	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)						
tF (s)	3.5	3.3			2.2	
p0 queue free %	100	99			99	
cM capacity (veh/h)	780	961			1498	
Direction, Lane #						
	WB 1	NB 1	SB 1			
Volume Total	13	96	80			
Volume Left	2	0	22			
Volume Right	11	1	0			
cSH	1136	1700	1498			
Volume to Capacity	0.01	0.06	0.01			
Queue Length 95th (ft)	1	0	1			
Control Delay (s)	8.9	0.0	2.1			
Lane LOS	A		A			
Approach Delay (s)	8.9	0.0	2.1			
Approach LOS	A					
Intersection Summary						
Average Delay			1.5			
Intersection Capacity Utilization		20.6%		ICU Level of Service		A
Analysis Period (min)			15			

HCM Unsignalized Intersection Capacity Analysis

9: Walnut Street & Cedar Avenue

Total Future PM School

						
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	48	15	60	9	11	54
Future Volume (Veh/h)	48	15	60	9	11	54
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	52	16	65	10	12	59
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)	3					
Median type			None		None	
Median storage (veh)						
Upstream signal (ft)						366
pX, platoon unblocked						
vC, conflicting volume	153	70			75	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	153	70			75	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)						
tF (s)	3.5	3.3			2.2	
p0 queue free %	94	98			99	
cM capacity (veh/h)	832	993			1524	
Direction, Lane #	WB 1	NB 1	SB 1			
Volume Total	68	75	71			
Volume Left	52	0	12			
Volume Right	16	10	0			
cSH	1088	1700	1524			
Volume to Capacity	0.06	0.04	0.01			
Queue Length 95th (ft)	5	0	1			
Control Delay (s)	9.4	0.0	1.3			
Lane LOS	A		A			
Approach Delay (s)	9.4	0.0	1.3			
Approach LOS	A					
Intersection Summary						
Average Delay			3.4			
Intersection Capacity Utilization			20.1%	ICU Level of Service		A
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis

9: Walnut Street & Cedar Avenue

Total Future PM



















Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	34	4	73	7	23	81
Future Volume (Veh/h)	34	4	73	7	23	81
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	37	4	79	8	25	88
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)		3				
Median type			None			None
Median storage (veh)						
Upstream signal (ft)						366
pX, platoon unblocked						
vC, conflicting volume	221	83			87	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	221	83			87	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)						
tF (s)	3.5	3.3			2.2	
p0 queue free %	95	100			98	
cM capacity (veh/h)	754	976			1509	
Direction, Lane #	WB 1	NB 1	SB 1			
Volume Total	41	87	113			
Volume Left	37	0	25			
Volume Right	4	8	0			
cSH	836	1700	1509			
Volume to Capacity	0.05	0.05	0.02			
Queue Length 95th (ft)	4	0	1			
Control Delay (s)	9.9	0.0	1.7			
Lane LOS	A		A			
Approach Delay (s)	9.9	0.0	1.7			
Approach LOS	A					
Intersection Summary						
Average Delay			2.5			
Intersection Capacity Utilization			22.2%	ICU Level of Service		A
Analysis Period (min)			15			

HCM Unsignalized Intersection Capacity Analysis

















10: Oak Street & Cedar Avenue

Total Future AM

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Sign Control		Stop			Stop			Stop			Stop	
Traffic Volume (vph)	11	4	4	25	8	7	3	41	6	3	26	6
Future Volume (vph)	11	4	4	25	8	7	3	41	6	3	26	6
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	12	4	4	27	9	8	3	45	7	3	28	7
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total (vph)	20	44	55	38								
Volume Left (vph)	12	27	3	3								
Volume Right (vph)	4	8	7	7								
Hadj (s)	0.03	0.05	-0.03	-0.06								
Departure Headway (s)	4.2	4.2	4.0	4.0								
Degree Utilization, x	0.02	0.05	0.06	0.04								
Capacity (veh/h)	836	841	864	872								
Control Delay (s)	7.3	7.4	7.3	7.2								
Approach Delay (s)	7.3	7.4	7.3	7.2								
Approach LOS	A	A	A	A								
Intersection Summary												
Delay			7.3									
Level of Service			A									
Intersection Capacity Utilization			13.3%	ICU Level of Service	A							
Analysis Period (min)			15									

HCM Unsignalized Intersection Capacity Analysis
 10: Oak Street & Cedar Avenue

















Total Future PM School

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Sign Control		Stop			Stop			Stop			Stop	
Traffic Volume (vph)	11	7	3	20	23	9	6	99	19	8	93	13
Future Volume (vph)	11	7	3	20	23	9	6	99	19	8	93	13
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	12	8	3	22	25	10	7	108	21	9	101	14
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total (vph)	23	57	136	124								
Volume Left (vph)	12	22	7	9								
Volume Right (vph)	3	10	21	14								
Hadj (s)	0.06	0.01	-0.05	-0.02								
Departure Headway (s)	4.6	4.5	4.2	4.2								
Degree Utilization, x	0.03	0.07	0.16	0.15								
Capacity (veh/h)	724	744	836	833								
Control Delay (s)	7.7	7.9	7.9	7.9								
Approach Delay (s)	7.7	7.9	7.9	7.9								
Approach LOS	A	A	A	A								
Intersection Summary												
Delay			7.9									
Level of Service			A									
Intersection Capacity Utilization			18.5%	ICU Level of Service	A							
Analysis Period (min)			15									

HCM Unsignalized Intersection Capacity Analysis

10: Oak Street & Cedar Avenue

Total Future PM

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Sign Control		Stop			Stop			Stop			Stop	
Traffic Volume (vph)	9	8	5	19	10	10	8	96	22	12	104	23
Future Volume (vph)	9	8	5	19	10	10	8	96	22	12	104	23
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	10	9	5	21	11	11	9	104	24	13	113	25
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total (vph)	24	43	137	151								
Volume Left (vph)	10	21	9	13								
Volume Right (vph)	5	11	24	25								
Hadj (s)	-0.01	-0.02	-0.06	-0.05								
Departure Headway (s)	4.6	4.5	4.2	4.2								
Degree Utilization, x	0.03	0.05	0.16	0.17								
Capacity (veh/h)	726	735	840	848								
Control Delay (s)	7.7	7.8	7.9	8.0								
Approach Delay (s)	7.7	7.8	7.9	8.0								
Approach LOS	A	A	A	A								
Intersection Summary												
Delay			7.9									
Level of Service			A									
Intersection Capacity Utilization			20.6%	ICU Level of Service	A							
Analysis Period (min)			15									

HCM Unsignalized Intersection Capacity Analysis

11: Cedar Avenue & McLean Avenue

Total Future AM



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↖	↗		↘	
Sign Control		Stop	Stop		Stop	
Traffic Volume (vph)	61	27	28	22	9	20
Future Volume (vph)	61	27	28	22	9	20
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	66	29	30	24	10	22

Direction, Lane #	EB 1	WB 1	SB 1
Volume Total (vph)	95	54	32
Volume Left (vph)	66	0	10
Volume Right (vph)	0	24	22
Hadj (s)	0.17	-0.23	-0.32
Departure Headway (s)	4.2	3.8	3.9
Degree Utilization, x	0.11	0.06	0.03
Capacity (veh/h)	846	924	881
Control Delay (s)	7.7	7.1	7.0
Approach Delay (s)	7.7	7.1	7.0
Approach LOS	A	A	A

Intersection Summary			
Delay		7.4	
Level of Service		A	
Intersection Capacity Utilization		21.5%	ICU Level of Service
Analysis Period (min)		15	A

HCM Unsignalized Intersection Capacity Analysis
 11: Cedar Avenue & McLean Avenue

Total Future PM School



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↔	↔		↔	
Sign Control		Stop	Stop		Stop	
Traffic Volume (vph)	22	9	16	19	13	27
Future Volume (vph)	22	9	16	19	13	27
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	24	10	17	21	14	29

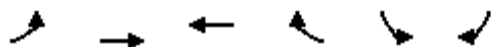
Direction, Lane #	EB 1	WB 1	SB 1
Volume Total (vph)	34	38	43
Volume Left (vph)	24	0	14
Volume Right (vph)	0	21	29
Hadj (s)	0.18	-0.30	-0.31
Departure Headway (s)	4.2	3.7	3.7
Degree Utilization, x	0.04	0.04	0.04
Capacity (veh/h)	842	949	932
Control Delay (s)	7.4	6.9	6.9
Approach Delay (s)	7.4	6.9	6.9
Approach LOS	A	A	A

Intersection Summary			
Delay		7.0	
Level of Service		A	
Intersection Capacity Utilization	18.4%		ICU Level of Service A
Analysis Period (min)		15	

HCM Unsignalized Intersection Capacity Analysis

11: Cedar Avenue & McLean Avenue

Total Future PM



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↖	↗		↘	
Sign Control		Stop	Stop		Stop	
Traffic Volume (vph)	68	24	28	20	7	23
Future Volume (vph)	68	24	28	20	7	23
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	74	26	30	22	8	25

Direction, Lane #	EB 1	WB 1	SB 1
Volume Total (vph)	100	52	33
Volume Left (vph)	74	0	8
Volume Right (vph)	0	22	25
Hadj (s)	0.18	-0.22	-0.37
Departure Headway (s)	4.2	3.8	3.9
Degree Utilization, x	0.12	0.06	0.04
Capacity (veh/h)	844	919	891
Control Delay (s)	7.7	7.1	7.0
Approach Delay (s)	7.7	7.1	7.0
Approach LOS	A	A	A
















Intersection Summary

Delay		7.4
Level of Service		A
Intersection Capacity Utilization	21.7%	ICU Level of Service
Analysis Period (min)	15	A

HCM Unsignalized Intersection Capacity Analysis

12: Internal Road/Site Entrance & Frontage Road
















Total Future AM

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	2	8	7	0	0	0	0	87	3	5	10	0
Future Volume (Veh/h)	2	8	7	0	0	0	0	87	3	5	10	0
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	2	9	8	0	0	0	0	95	3	5	11	0
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								None			None	
Median storage (veh)												
Upstream signal (ft)											90	
pX, platoon unblocked												
vC, conflicting volume	118	119	11	130	118	96	11			98		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	118	119	11	130	118	96	11			98		
tC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	100	99	99	100	100	100	100			100		
cM capacity (veh/h)	856	769	1070	827	770	960	1608			1495		
Direction, Lane #	EB 1	NB 1	SB 1									
Volume Total	19	98	16									
Volume Left	2	0	5									
Volume Right	8	3	0									
cSH	883	1700	1495									
Volume to Capacity	0.02	0.06	0.00									
Queue Length 95th (ft)	2	0	0									
Control Delay (s)	9.2	0.0	2.3									
Lane LOS	A		A									
Approach Delay (s)	9.2	0.0	2.3									
Approach LOS	A											
Intersection Summary												
Average Delay			1.6									
Intersection Capacity Utilization			15.0%		ICU Level of Service				A			
Analysis Period (min)			15									

HCM Unsignalized Intersection Capacity Analysis

12: Internal Road/Site Entrance & Frontage Road
















Total Future PM School

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	2	31	22	0	0	0	0	88	12	13	30	0
Future Volume (Veh/h)	2	31	22	0	0	0	0	88	12	13	30	0
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	2	34	24	0	0	0	0	96	13	14	33	0
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								None			None	
Median storage (veh)												
Upstream signal (ft)											90	
pX, platoon unblocked	0.99	0.99	0.99	0.99	0.99		0.99					
vC, conflicting volume	164	170	33	204	164	102	33			109		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	155	162	24	197	155	102	24			109		
tC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	100	95	98	100	100	100	100			99		
cM capacity (veh/h)	800	719	1046	709	725	953	1581			1481		
Direction, Lane #	EB 1	NB 1	SB 1									
Volume Total	60	109	47									
Volume Left	2	0	14									
Volume Right	24	13	0									
cSH	825	1700	1481									
Volume to Capacity	0.07	0.06	0.01									
Queue Length 95th (ft)	6	0	1									
Control Delay (s)	9.7	0.0	2.3									
Lane LOS	A		A									
Approach Delay (s)	9.7	0.0	2.3									
Approach LOS	A											
Intersection Summary												
Average Delay			3.2									
Intersection Capacity Utilization			19.0%			ICU Level of Service				A		
Analysis Period (min)			15									

HCM Unsignalized Intersection Capacity Analysis

12: Internal Road/Site Entrance & Frontage Road
















Total Future PM

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	2	34	25	0	0	0	0	84	10	14	40	0
Future Volume (Veh/h)	2	34	25	0	0	0	0	84	10	14	40	0
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	2	37	27	0	0	0	0	91	11	15	43	0
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								None			None	
Median storage (veh)												
Upstream signal (ft)											90	
pX, platoon unblocked	0.99	0.99	0.99	0.99	0.99		0.99					
vC, conflicting volume	170	175	43	215	170	96	43			102		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	159	164	31	205	159	96	31			102		
tC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	100	95	97	100	100	100	100			99		
cM capacity (veh/h)	794	715	1035	694	720	960	1569			1490		
Direction, Lane #	EB 1	NB 1	SB 1									
Volume Total	66	102	58									
Volume Left	2	0	15									
Volume Right	27	11	0									
cSH	821	1700	1490									
Volume to Capacity	0.08	0.06	0.01									
Queue Length 95th (ft)	7	0	1									
Control Delay (s)	9.8	0.0	2.0									
Lane LOS	A		A									
Approach Delay (s)	9.8	0.0	2.0									
Approach LOS	A											
Intersection Summary												
Average Delay			3.4									
Intersection Capacity Utilization			19.6%			ICU Level of Service				A		
Analysis Period (min)			15									

HCM Unsignalized Intersection Capacity Analysis

13: Internal Road/Site Entrance & Frontage Road
















Total Future AM

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	1	3	12	0	0	0	0	96	1	0	32	0
Future Volume (Veh/h)	1	3	12	0	0	0	0	96	1	0	32	0
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	1	3	13	0	0	0	0	104	1	0	35	0
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								None			None	
Median storage (veh)												
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	140	140	35	154	140	104	35			105		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	140	140	35	154	140	104	35			105		
tC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	100	100	99	100	100	100	100			100		
cM capacity (veh/h)	831	751	1038	800	752	950	1576			1486		
Direction, Lane #	EB 1	NB 1	SB 1									
Volume Total	17	105	35									
Volume Left	1	0	0									
Volume Right	13	1	0									
cSH	959	1700	1486									
Volume to Capacity	0.02	0.06	0.00									
Queue Length 95th (ft)	1	0	0									
Control Delay (s)	8.8	0.0	0.0									
Lane LOS	A											
Approach Delay (s)	8.8	0.0	0.0									
Approach LOS	A											
Intersection Summary												
Average Delay			1.0									
Intersection Capacity Utilization			15.1%		ICU Level of Service				A			
Analysis Period (min)			15									

HCM Unsignalized Intersection Capacity Analysis

13: Internal Road/Site Entrance & Frontage Road
















Total Future PM School

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	2	19	35	0	0	0	0	69	9	0	90	0
Future Volume (Veh/h)	2	19	35	0	0	0	0	69	9	0	90	0
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	2	21	38	0	0	0	0	75	10	0	98	0
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								None			None	
Median storage (veh)												
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	178	183	98	226	178	80	98			85		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	178	183	98	226	178	80	98			85		
tC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	100	97	96	100	100	100	100			100		
cM capacity (veh/h)	784	711	958	684	716	980	1495			1512		
Direction, Lane #	EB 1	NB 1	SB 1									
Volume Total	61	85	98									
Volume Left	2	0	0									
Volume Right	38	10	0									
cSH	850	1700	1512									
Volume to Capacity	0.07	0.05	0.00									
Queue Length 95th (ft)	6	0	0									
Control Delay (s)	9.6	0.0	0.0									
Lane LOS	A											
Approach Delay (s)	9.6	0.0	0.0									
Approach LOS	A											
Intersection Summary												
Average Delay			2.4									
Intersection Capacity Utilization			14.7%		ICU Level of Service					A		
Analysis Period (min)			15									

HCM Unsignalized Intersection Capacity Analysis

13: Internal Road/Site Entrance & Frontage Road

Total Future PM

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	1	18	39	0	0	0	0	66	9	0	120	0
Future Volume (Veh/h)	1	18	39	0	0	0	0	66	9	0	120	0
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	1	20	42	0	0	0	0	72	10	0	130	0
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								None			None	
Median storage (veh)												
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	207	212	130	259	207	77	130			82		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	207	212	130	259	207	77	130			82		
tC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	100	97	95	100	100	100	100			100		
cM capacity (veh/h)	751	685	920	648	690	984	1455			1515		
Direction, Lane #	EB 1	NB 1	SB 1									
Volume Total	63	82	130									
Volume Left	1	0	0									
Volume Right	42	10	0									
cSH	827	1700	1515									
Volume to Capacity	0.08	0.05	0.00									
Queue Length 95th (ft)	6	0	0									
Control Delay (s)	9.7	0.0	0.0									
Lane LOS	A											
Approach Delay (s)	9.7	0.0	0.0									
Approach LOS	A											
Intersection Summary												
Average Delay			2.2									
Intersection Capacity Utilization			16.4%			ICU Level of Service				A		
Analysis Period (min)			15									