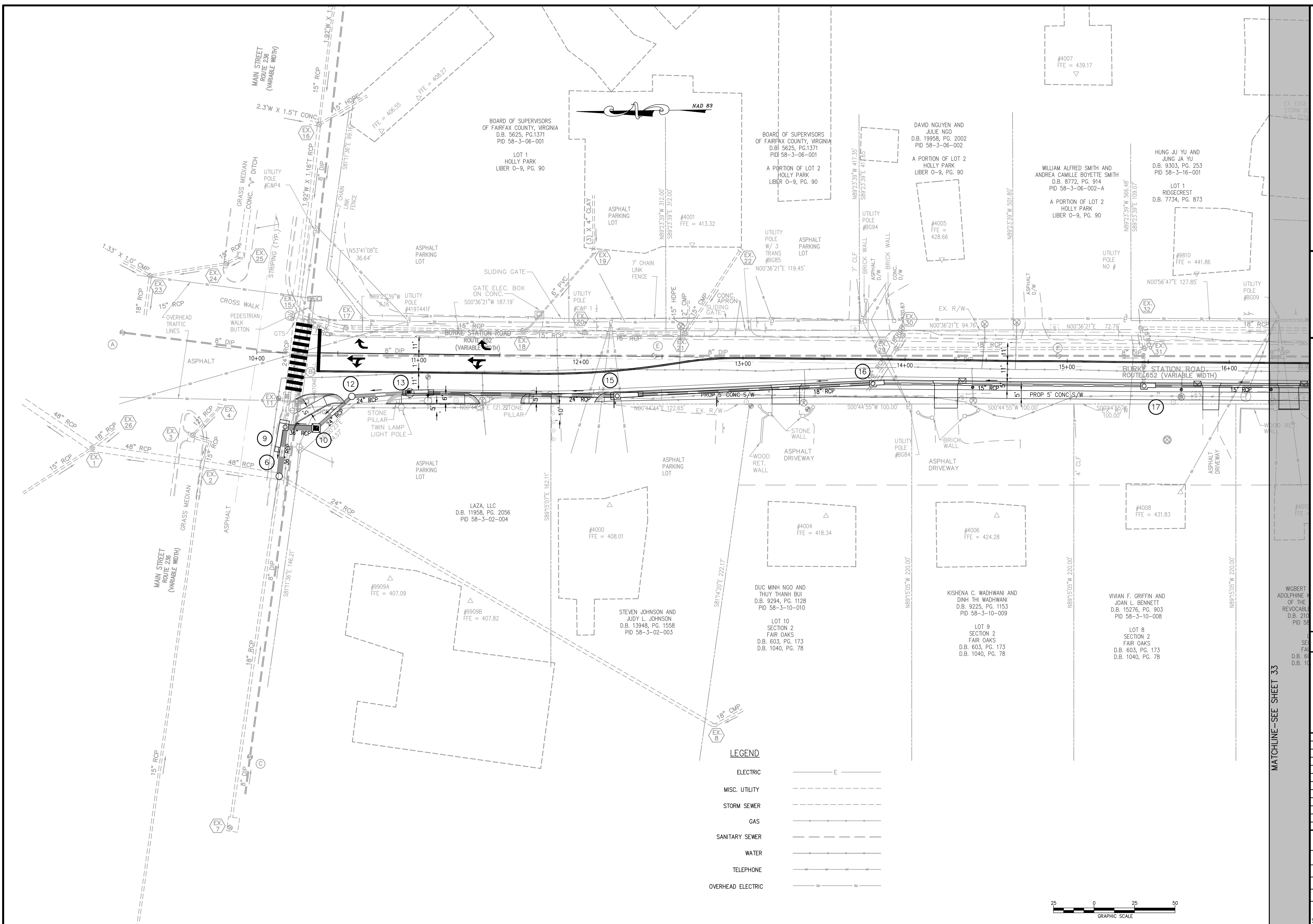


PROJECT NUMBER		
PLAN STATUS		
DATE	DESCRIPTION	
SB	SB	SD
DESIGN	DRAWN	CHKD
SCALE	H: 1"=25'	
	V: N/A	
JOB No. 6916-01-002		
DATE : DECEMBER, 2014		
FILE No. 6916-D-MP-002		
SHEET	32	OF 59

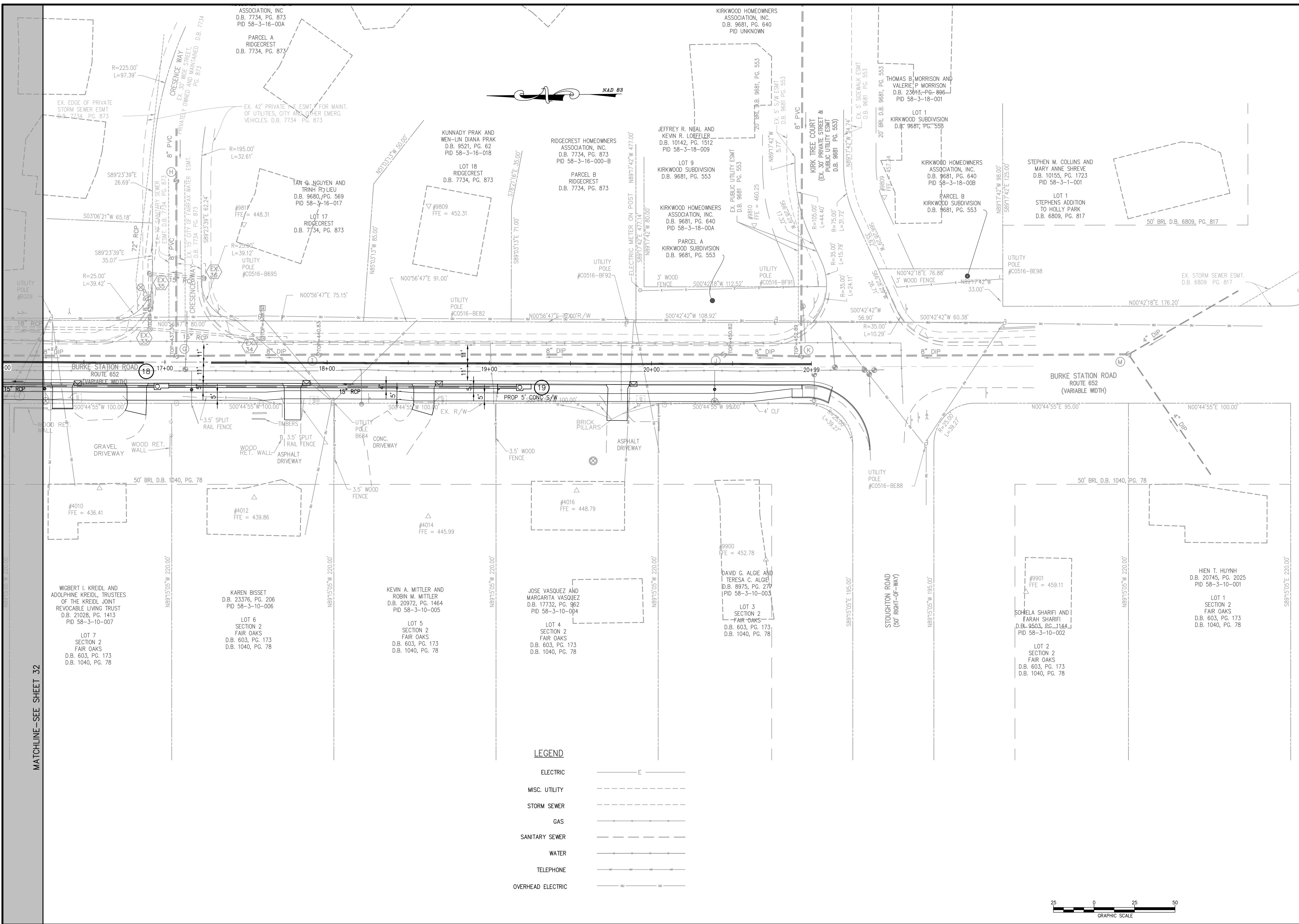


LEGEND

ELECTRIC	— E —
MISC. UTILITY	— — —
STORM SEWER	— — —
GAS	— — —
SANITARY SEWER	— — —
WATER	— — —
TELEPHONE	— — —
OVERHEAD ELECTRIC	— o — o —



MATCHLINE—SEE SHEET 33



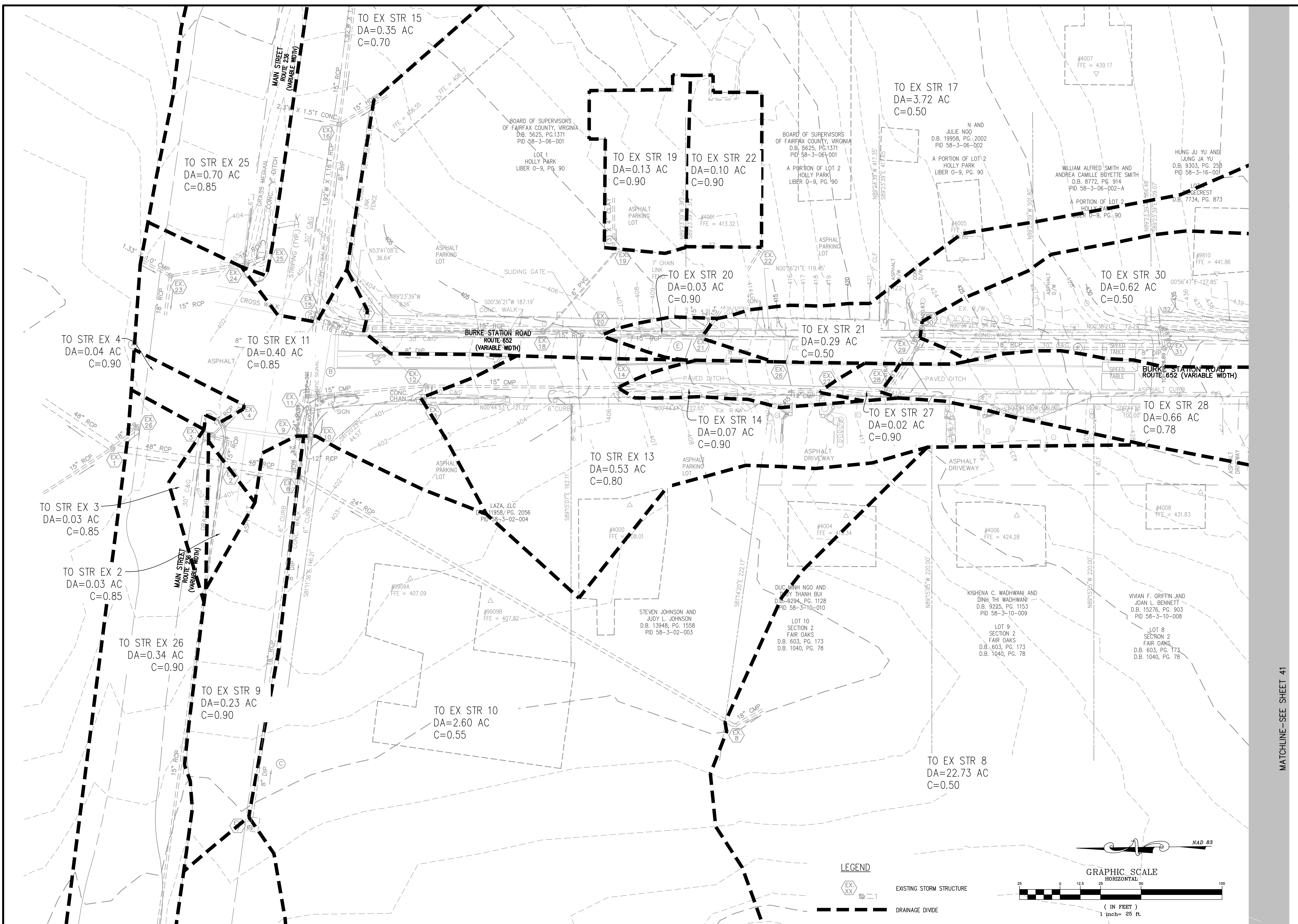
LEGEND

- ELECTRIC ———— E ————
- MISC. UTILITY - - - - -
- STORM SEWER ————
- GAS ————
- SANITARY SEWER ————
- WATER ————
- TELEPHONE ————
- OVERHEAD ELECTRIC ————



MATCHLINE—SEE SHEET 32

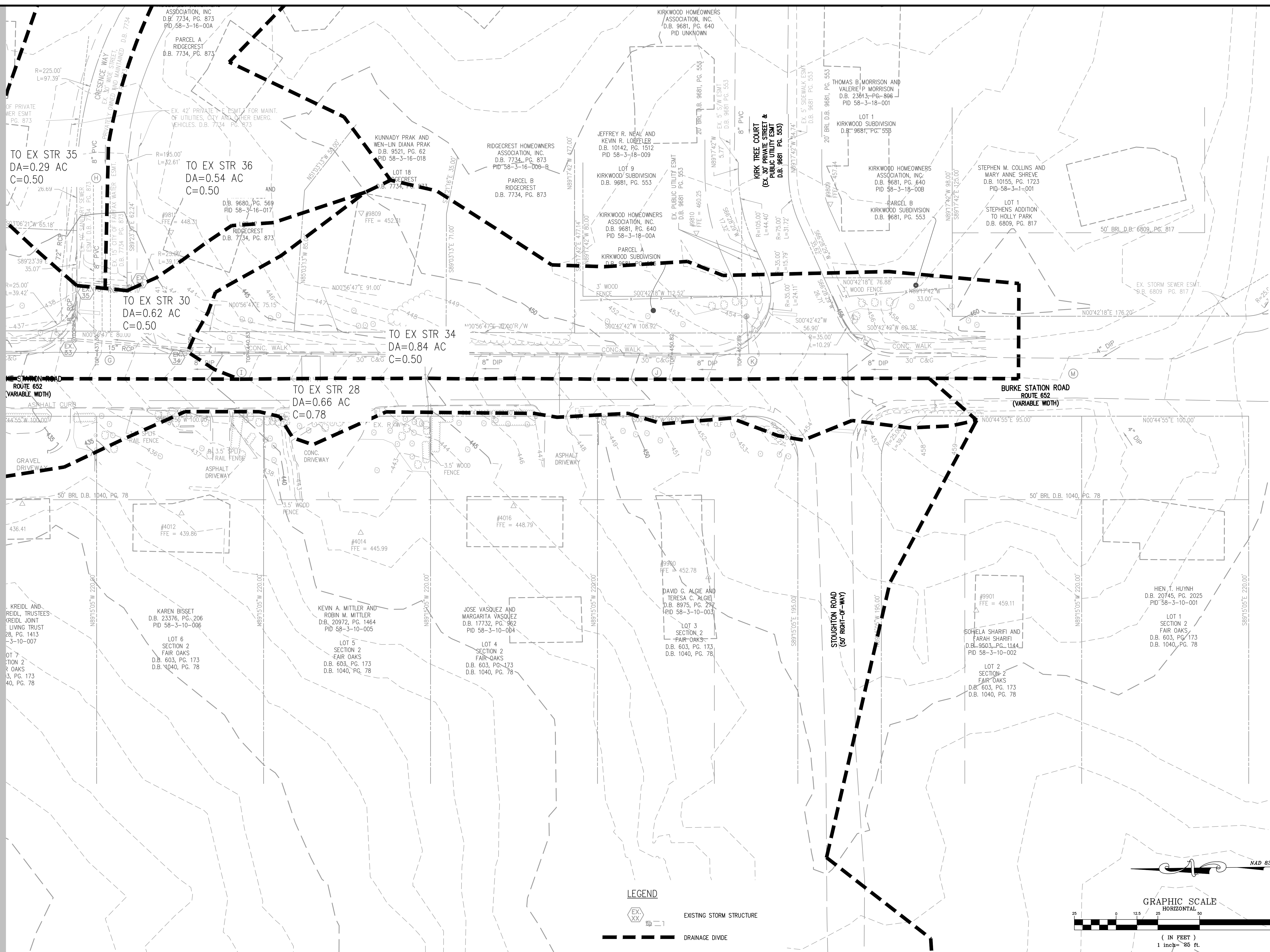
PROJECT NUMBER		
PLAN STATUS		
DATE	DESCRIPTION	
SB	SB	SD
DESIGN	DRAWN	CHKD
SCALE	H: 1"=25'	
	V: N/A	
JOB No.	6916-01-002	
DATE	DECEMBER, 2014	
FILE No.	6916-D-MP-002	
SHEET	40 OF 59	



MATCHLINE - SEE SHEET 41

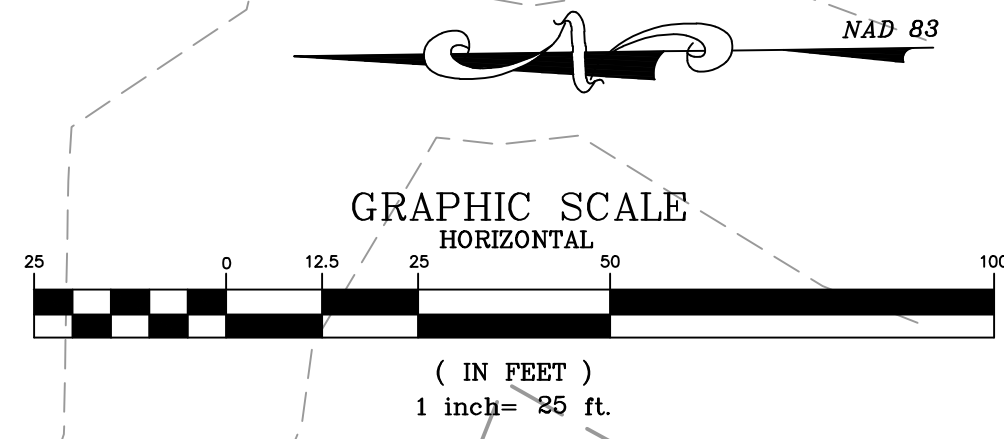
PROJECT NUMBER	
PLAN STATUS	
DATE	DESCRIPTION
SB	SB SD
DESIGN	DRAWN CHKD
SCALE	H: 1"=25'
	V: N/A
JOB No.	6916-01-002
DATE :	DECEMBER, 2014
FILE No.	6916-D-MP-002
SHEET 41 OF 59	

MATCHLINE-SEE SHEET 40

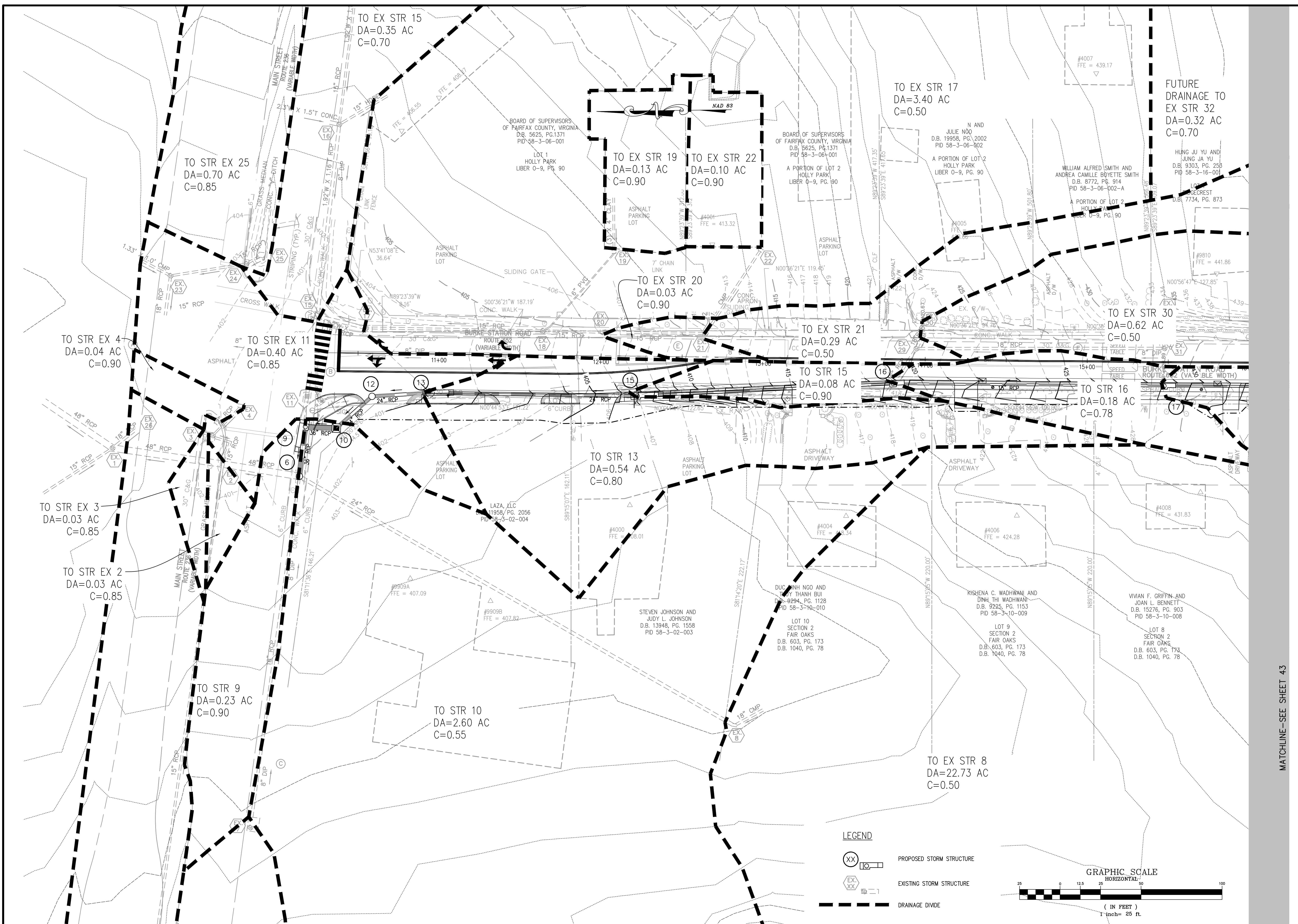


LEGEND

- EXISTING STORM STRUCTURE
- DRAINAGE DIVIDE

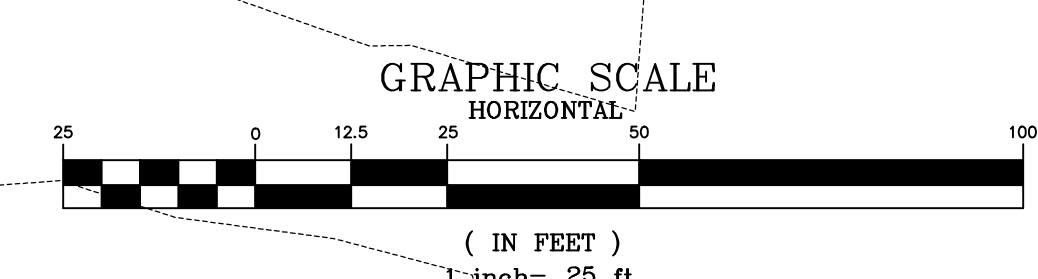


PROJECT NUMBER		
PLAN STATUS		
DATE	DESCRIPTION	
SB	SB	SD
DESIGN	DRAWN	CHKD
SCALE	H: 1"=25'	
	V: N/A	
JOB No. 6916-01-002		
DATE : DECEMBER, 2014		
FILE No. 6916-D-MP-002		
SHEET 42 OF 59		



LEGEND

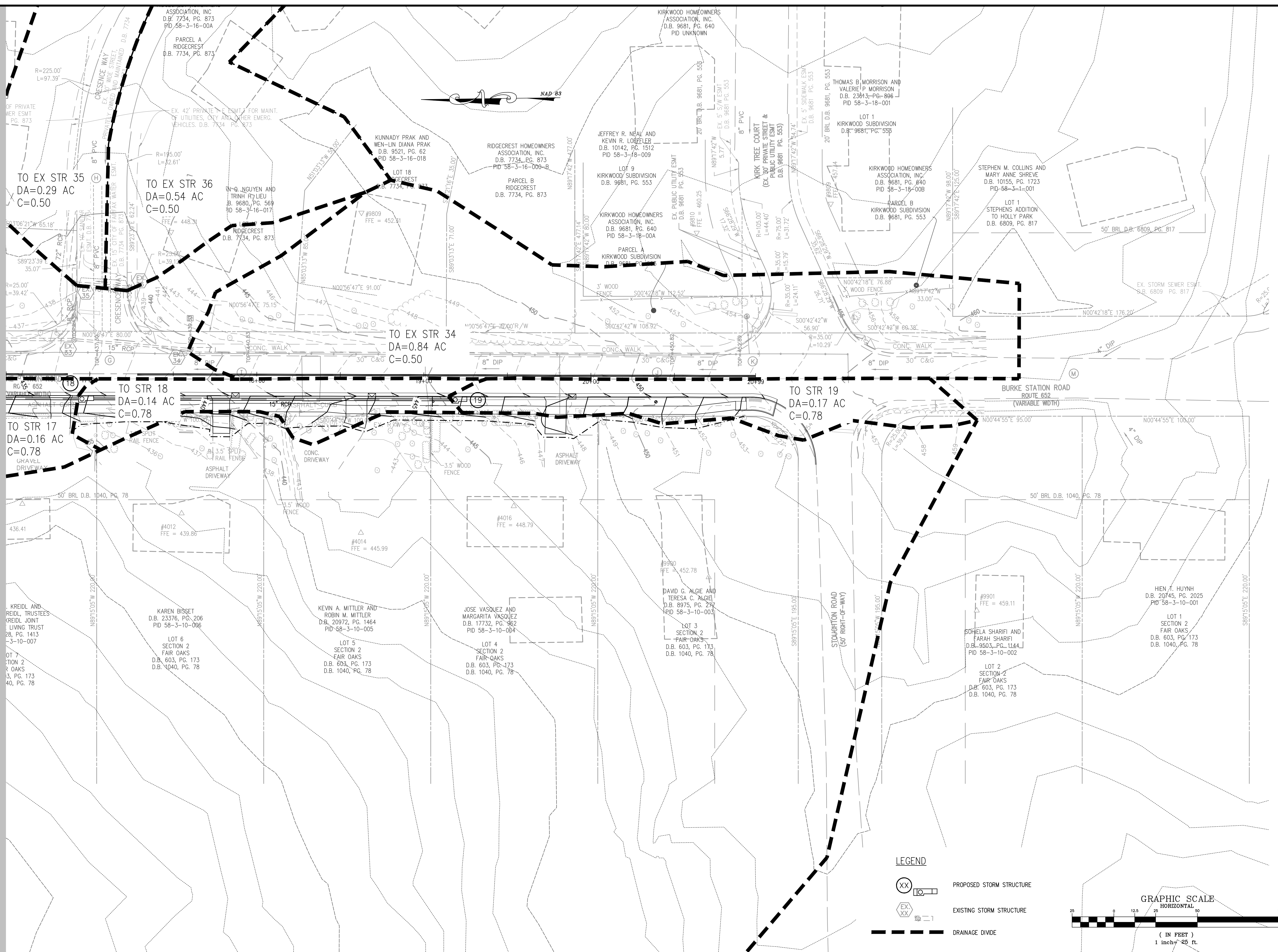
- XX PROPOSED STORM STRUCTURE
- EX EXISTING STORM STRUCTURE
- DRAINAGE DIVIDE



MATCHLINE—SEE SHEET 43

PROJECT NUMBER		
PLAN STATUS		
DATE	DESCRIPTION	
DESIGN	SB	SD
DRAWN		CHKD
SCALE	H: 1"=25'	V: N/A
JOB No.	6916-01-002	
DATE	DECEMBER, 2014	
FILE No.	6916-D-MP-002	
SHEET	43	OF 59

MATCHLINE-SEE SHEET 42



LEGEND

- PROPOSED STORM STRUCTURE
- EXISTING STORM STRUCTURE
- DRAINAGE DIVIDE

GRAPHIC SCALE
 HORIZONTAL

(IN FEET)
 1 inch = 25 ft.

EXISTING STORM SEWER COMPUTATIONS

STRUCTURE		DRAINAGE	RUN-OFF	"CA"		INLET	RAIN	RUNOFF	INVERT		LENGTH	SLOPE	MANNING'S	DIAMETER	CAPACITY	VELOCITY	FLOW	STRUCTURE TOP ELEVATION	REMARKS		
From	To	AREA (ACRES)	COEF. "A"	"C"	Increment	Accumulated	Min.	In./Hr.	"Q" INC C.F.S.	Upper End	Lower End	Feet	Feet/Feet	"n" VALUE	RCP	Inches	C.F.S.	%	F.P.S.	Seconds	
Ex 34	Ex 33	0.840	0.50	0.42	0.42	5	7.27	3.05	429.75	428.76	69.92	0.0142	0.013	15	7.68	39.77	6.26	11	439.55		
Ex 33	Ex 32	0.000	0.90	0.00	2.84	5	7.27	20.61	427.06	422.44	145.16	0.0318	0.013	18	18.73	110.05	10.60	14	436.36	Add Flow From Ex 35	
Ex 32	Ex 31	0.000	0.90	0.00	2.84	5	7.27	20.61	422.34	421.70	12.69	0.0504	0.013	18	23.58	87.42	13.34	1	429.24		
Ex 31	Ex 29	0.000	0.90	0.00	2.84	5	7.27	20.61	421.60	417.03	152.99	0.0299	0.013	18	18.14	113.59	10.27	15	428.90		
Ex 29	Ex 28	0.000	0.90	0.00	3.15	5	7.27	22.86	416.98	416.34	33.94	0.0189	0.013	18	14.42	158.60	8.16	4	420.13	Add Flow From Ex 30	
Ex 28	Ditch	0.660	0.78	0.51	3.66	5	7.27	26.61	417.64	416.24	100.00	0.0140	0.013	18	12.42	214.20	7.03	14	420.13		
Ex 30	Ex 29	0.620	0.50	0.31	0.31	5	7.27	2.25	417.35	417.08	9.14	0.0295	0.013	18	18.04	12.49	10.21	1	420.65		
Ex 36	Ex 35	0.540	0.50	0.27	0.27	5	7.27	1.96	433.64	433.27	33.36	0.0111	0.013	15	6.80	28.89	5.54	6	437.99		
Ex 35	Ex 33	0.290	0.50	0.15	2.42	5	7.27	17.56	428.07	426.96	34.09	0.0326	0.013	18	18.94	92.68	10.72	3	438.27	Add flow from upstream cul-de-sac, 14.6 cfs	
Ex 27	Ex 26	0.020	0.90	0.02	3.68	5	7.27	26.74	414.98	413.83	18.46	0.0623	0.013	12	8.88	301.26	11.30	2			
Ex 14	Ex 12	0.070	0.90	0.06	3.74	5	7.27	27.20	403.59	397.79	119.40	0.0486	0.013	15	14.22	191.24	11.59	10	0.00	Add Flow From Ex 27	
Ex 12	Ex 11	0.000	0.90	0.00	4.16	5	7.27	30.28	397.69	397.00	73.71	0.0094	0.013	15	6.24	485.03	5.09	14	401.04	Add Flow From Ex 13	
Ex 11	Ex 9	0.400	0.85	0.34	7.44	5	7.27	54.12	396.90	396.36	26.16	0.0206	0.013	24	32.51	166.44	10.35	3	399.90	Add Flow From Ex 15	
Ex 9	Ex 6	0.230	0.90	0.21	9.08	10	5.92	53.76	396.01	395.52	23.87	0.0205	0.013	24	32.42	165.79	10.32	2	400.71	Add Flow From Ex 10	
Ex 6	Ex 2	0.000	0.90	0.00	23.93	10	5.92	141.64	394.02	392.60	47.96	0.0296	0.013	48	247.83	57.15	19.72	2	401.02	Add Flow From Ex 8 and Ex 7	
Ex 2	Ex 1	0.030	0.85	0.03	24.51	10	5.92	145.09	392.30	387.18	68.65	0.0746	0.013	48	393.34	36.89	31.30	2	400.60	Add Flow From Ex 3 and Ex 5	
Ex 13	Ex 12	0.530	0.80	0.42	0.42	5	7.27	3.08	399.06	398.34	8.82	0.0816	0.013	3	0.25	1228.90	5.11	2	401.71	3.2"x0.6' channel	
Ex 4	Ex 3	0.040	0.90	0.04	0.04	5	7.27	0.26	397.35	397.14	24.92	0.0084	0.013	12	3.26	8.02	4.16	6	400.35		
Ex 3	Ex 2	0.030	0.85	0.03	0.06	5	7.27	0.45	396.44	395.80	26.00	0.0246	0.013	15	10.12	4.42	8.25	3	401.44		
Ex 5	Ex 2	0.550	0.90	0.50	0.50	5	7.27	3.60	408.70	397.30	383.25	0.0297	0.013	15	11.13	32.34	9.07	42	413.90		
Ex 7	Ex 6	6.960	0.50	3.48	3.48	5	7.27	25.30	406.78	396.22	218.82	0.0483	0.013	18	23.06	109.70	13.05	17	410.98		
Ex 8	Ex 6	22.730	0.50	11.37	11.37	5	7.27	82.62	405.36	396.62	309.46	0.0282	0.013	24	38.03	217.24	12.11	26	408.11		
Ex 10	Ex 9	2.600	0.55	1.43	1.43	10	5.92	8.47	396.97	396.81	7.32	0.0219	0.013	12	5.26	161.03	6.69	1	400.27		
Ex 22	Ex 21	0.100	0.90	0.09	0.09	5	7.27	0.65	412.08	406.68	61.69	0.0875	0.013	15	19.09	3.43	15.56	4	414.43		
Ex 21	Ex 20	0.290	0.50	0.15	0.24	5	7.27	1.71	405.68	401.54	61.79	0.0670	0.013	15	16.70	10.23	13.61	5	411.19		
Ex 20	Ex 18	0.030	0.90	0.03	0.26	5	7.27	1.90	401.34	401.05	38.53	0.0075	0.013	15	5.60	34.03	4.56	8	406.39		
Ex 18	Ex 17	0.000	0.90	0.00	0.38	5	7.27	2.76	400.90	398.64	109.14	0.0207	0.013	15	9.28	29.68	7.57	14	404.10	Add Flow From Ex 19	
Ex 17	Ex 15	3.720	0.50	1.86	2.24	5	7.27	16.28	398.54	397.48	31.62	0.0335	0.013	18	19.22	84.68	10.88	3	401.04	14"x23" ERCP	
Ex 31	Ex 30	0.370	0.70	0.26	0.26	5	7.27	1.88	420.00	408.00	150.00	0.0800	0.013	15	18.25	10.32	14.87	10	424.00		
Ex 30	Ex 16	0.280	0.70	0.20	0.46	5	7.27	3.31	400.00	400.14	120.00	0.0655	0.013	15	16.51	20.03	13.46	9			
Ex 16	Ex 15	0.000	0.90	0.00	0.46	5	7.27	3.31	400.04	397.78	118.62	0.0191	0.013	18	14.49	22.83	8.20	14	404.29	14"x23" ERCP	
Ex 15	Ex 11	0.350	0.70	0.25	2.94	5	7.27	21.37	397.23	397.00	50.24	0.0046	0.013	24	15.31	139.54	4.87	10	400.03	Add Flow from Ex 17	
Ex 19	Ex 18	0.130	0.90	0.12	0.12	5	7.27	0.85	403.44	401.80	65.96	0.0249	0.013	6	0.88	96.55	4.49	15	405.64		

Existing Storm Sewer information was obtained via an As-Built survey conducted by Bowman Consulting.

NOTE: ALL INLETS TO EMPLOY IS-1 INLET SHAPING

EXISTING STORM CURB INLET COMPUTATIONS

NUMBER	TYPE	LENGTH ft.	DRAINAGE AREA Ac.	C	CA	INTENSITY In./Hr.	Q INCRE. C.F.S.	Q Carryover C.F.S.	Q gutterflow C.F.S.	S Gutterslope ft./ft.	Sx Cross Slope ft./ft.	T Spread ft.	W ft.	W/T	Sw ft./ft.	Sw/Sx	Eo (App 9C-8)	a (12W)	Se (Sx+SwEo) ft./ft.	REQ. LT Length ft.	L/LT	E (Chart 16)	Q Intercepted C.F.S.	Qb Carry Over C.F.S.	REMARKS	
Ex 13	DI-3B	15	4.63	0.90	4.167	4.0	16.668	0.00	16.668	0.0287	0.02	4.16	2	0.48	0.0833	4.2	1.00	2	0.083	0.103	45.11	0.33	0.52	8.610	8.05	
Ex 9	DI-3B	8	0.23	0.90	0.207	4.0	0.828	0.00	0.828	0.0830	0.02	1.75	2	1.14	0.0833	4.2	1.00	2	0.083	0.103	13.39	0.60	0.81	1.210	0.29	
Ex 41	DI-3B	15	0.37	0.70	0.259	4.0	1.036	0.00	1.036	0.0800	0.02	1.60	2	1.25	0.0833	4.2	1.00	2	0.083	0.103	13.12	1.14	1.00	1.036	0.00	
Ex 40	DI-3B	18	0.28	0.70	0.196	4.0	0.784	0.00	0.784	0.0714	0.02	1.62	2	1.23	0.0833	4.2	1.00	2	0.083	0.103	11.59	1.55	1.00	0.784	0.00	

GRATE INLET DESIGN COMPUTATIONS

Structure #	Area (Ac.)	"C"	"I"	FLOW	GRATE DESIGN			
					GRATE	PERM	AREA	DEPTH
EX 11	2.69	0.90	4	9.70	III	7.5	5.63	0.790
EX 15	0.27	0.90	4	0.97	III	7.5	5.63	0.200

EXISTING JUNCTION LOSS COMPUTATIONS

INLET NO.	OUTLET WS	Do	Qo	Lo	Sf %	Hf	Vo	Ho	Qi	Vi	QVI	Vp ² 2g	Hi	ANGLE	K	Hd	Ht	1.30 Ht	0.50 Ht	FINAL H	INLET WATER SURFACE ELEV.	RIM ELEV.	
Ex 2	390.38	48	145.09	68.65	1.02056	0.70062	31.3049	3.80434	145.09	19.7192	2861.058	6.03799	2.1133	0	0	0	5.91764	—	2.9588	394.04	394.04	400.60	
Ex 6	395.80	48	141.64	47.96	0.97261	0.46646	19.7192	1.5095	141.64	10.3141	1460.8852	1.65186	0.57815	101	0	0	1.15631	3.24396	—	1.622	397.89	397.89	401.02
Ex 9	397.89	24	53.76	23.87	5.64907	1.34843	10.3141	0.41297	54.12	10.3141	558.19758	1.65186	0.57815	14	0.178	0.29403	1.28515	—	0.6426	399.88	399.88	400.71	
Ex 11	399.88	24	54.12	26.16	5.72498	1.49766	10.6993	0.44439	51.65	10.6993	552.61964	1.77757	0.62215	0	0	0	1.06654	—	0.5333	401.91	401.91	399.90	
Ex 15	401.91	24	21.37	50.24	0.89262	0.44845	4.88576	0.09267	21.37	8.6858	185.61546	1.17148	0.41002	0	0	0	0.50268	—	0.2513	402.61	402.61	401.04	
Ex 16	402.61	18	3.31	118.62	0.09932	0.11782	8.21034	0.26168	3.31	11.8229	39.133749	2.17051	0.75968	0	0	0	1.02136	—	0.5107	403.24	403.24	403.59	
Ex 30	403.24	15	3.31	120	0.26264	0.31516	13.4559	0.70288	1.88	13.4559	25.297135	2.81152	0.98403	0	0	0	1.68691	2.193	—	405.75	405.75	412.00	
Ex 31	409.00	15	1.88	150	0.08473	0.12709	14.8709	0.85848	0	14.8709	0	3.43392	1.20187	0	0	0	2.06035	2.6785	—	411.81	411.81	424.00	
Ex 12	401.91	15	30.28	73.71	21.9791	16.2008	5.09749	0.10087	27.2	5.09749	138.65185	0.40349	0.14122	5	0.06	0.02421	0.2663	—					

PROPOSED STORM SEWER COMPUTATIONS

Main table for PROPOSED STORM SEWER COMPUTATIONS with columns: STRUCTURE, DRAINAGE, RUN-OFF, "CA", INLET, RAIN, RUNOFF, INVERT, LENGTH, SLOPE, MANNING'S, DIAMETER, CAPACITY, VELOCITY, FLOW, STRUCTURE TOP ELEVATION, REMARKS.

GRATE INLET DESIGN COMPUTATIONS table with columns: Structure #, Area (Ac.), "C", "I", FLOW, GRATE PERIM, AREA, DEPTH.

Existing Storm Sewer information was obtained via an As-Built survey conducted by Bowman Consulting.

NOTE: ALL INLETS TO EMPLOY IS-1 INLET SHAPING

PROPOSED STORM CURB INLET COMPUTATIONS

Table for PROPOSED STORM CURB INLET COMPUTATIONS with columns: NUMBER, TYPE, LENGTH, DRAINAGE AREA, C, CA, INTENSITY, Q, Carryover, Gutterflow, Gutterslope, Sx, T, W, W/T, Sw, Sw/Sx, Eo, a, Sw', Se, REQ LT, L/LI, E, Q, Qb, REMARKS.

STORMWATER NARRATIVE

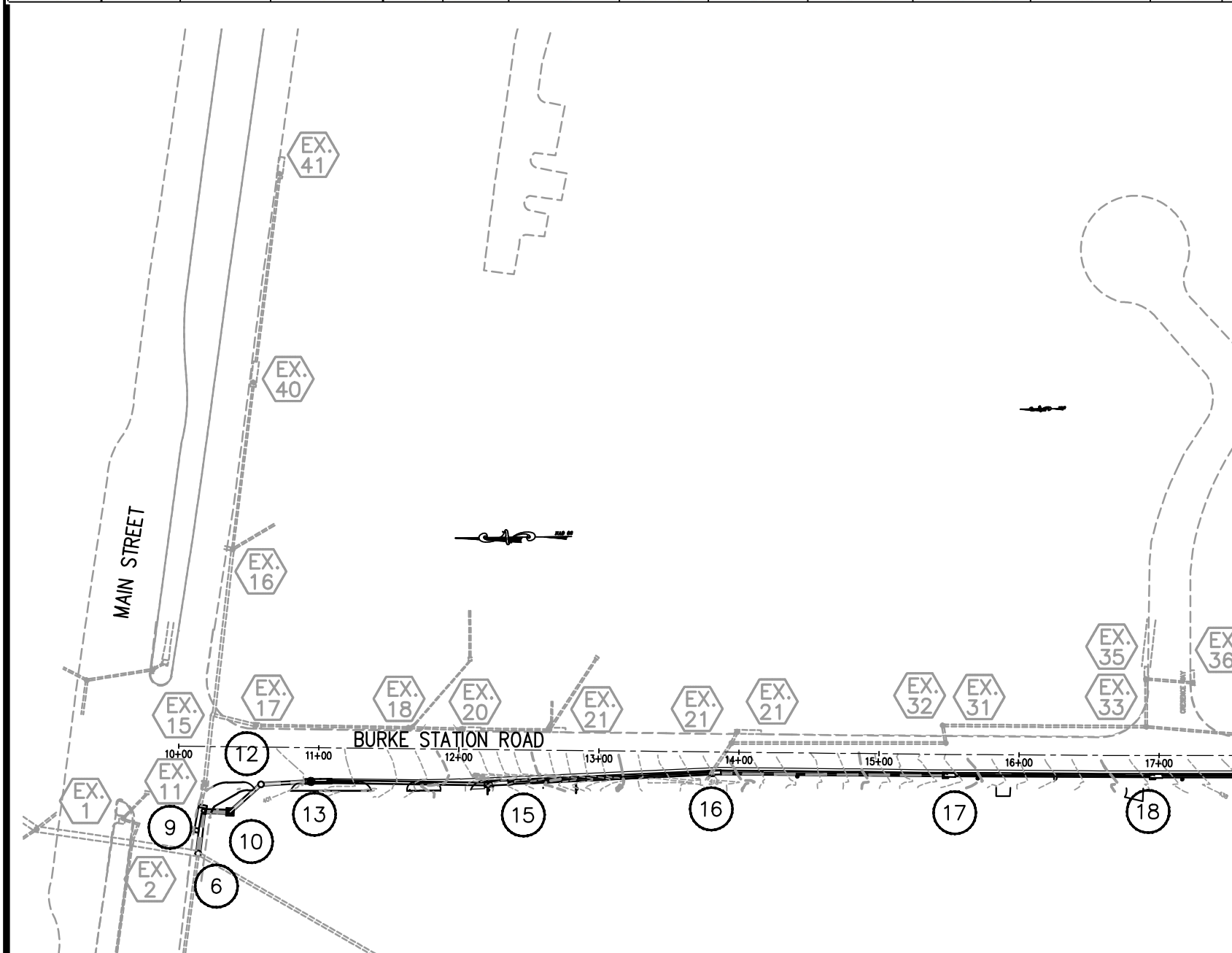
THIS PROJECT IS PROPOSING ROAD IMPROVEMENTS CONSISTING MAINLY OF CURB AND SIDEWALK INSTALLATION ALONG BURKE STATION ROAD. THE EXISTING AND PROPOSED STORM SEWER AFFECTING THE PROJECT AREA WERE ANALYZED. SEE THIS SHEET FOR PROPOSED COMPUTATIONS AND SHEET 44 FOR EXISTING COMPUTATIONS.

IN THE EXISTING CONDITION GRATE INLETS 2, 4, 11 AND 15 AND CURB INLET 9, 13, 30 AND 31 WERE ANALYZED TO DETERMINE THE INLET CAPACITY AND FLOODING AT EACH LOCATION. IT WAS ASSUMED THAT THE CULVERT AT EX 14 WAS CLOGGED, DEBRIS WAS OBSERVED AT THIS LOCATION WHICH WOULD PREVENT STORMWATER FROM ENTERING THE PIPE DURING A RAINFALL EVENT.

A HYDRAULIC GRADE LINE ANALYSIS WAS ALSO PERFORMED. A MAJORITY OF THE STRUCTURES INCLUDING EX 10, 11, 12, 14, AND 15 WILL CAUSE FLOODING PROBLEMS WHERE STORMWATER WILL EXIT THE SYSTEM THROUGH THE TOP OF THE STRUCTURE. THIS IS DUE TO THE PIPES BEING UNDERSIZED FOR THE AMOUNT OF RUNOFF THEY RECEIVE.

THE PROPOSED STORM SEWER IMPROVEMENTS WILL MOVE THE SYSTEM UNDERGROUND. THE PROPOSED STORM SEWER ARE SIZED TO ACCOMMODATE THE RUNOFF RECEIVED. DRAINAGE PROBLEMS DUE TO INADEQUATE PIPE SIZES HAVE BEEN ALLEVIATED ALONG THE WESTERN PORTION OF BURKE STATION ROAD WHERE THE STORM SEWERS HAVE BEEN UPSIZED.

STRUCTURES 11 AND 15 WILL NO LONGER CREATE FLOODING PROBLEMS DUE TO WATER LEAVING THE SYSTEM. THE PROPOSED INLETS WERE SIZED TO CAPTURE THE 10 YEAR STORM. EXISTING GRATE INLET 11 ANALYSIS WAS PERFORMED TO REFLECT THE PROPOSED IMPROVEMENTS, IT WILL NOW RECEIVE 1.36 CFS AND PRODUCE A PONDING DEPTH OF 0.24' WITH A SPREAD OF 11.96'.

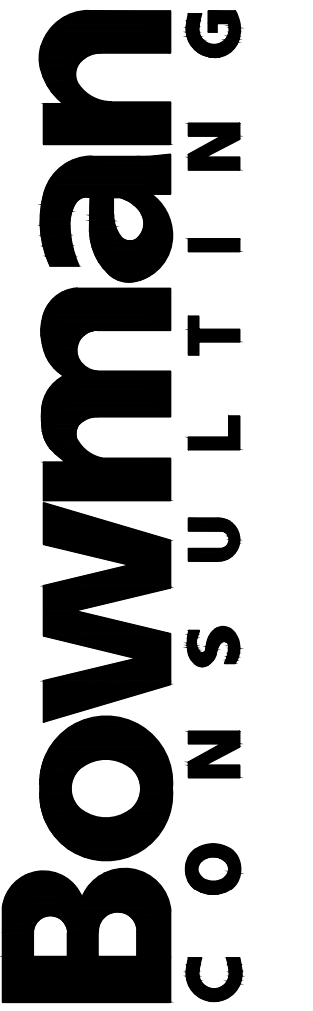


STORM LEGEND SCALE: 1"=100'

PROPOSED JUNCTION LOSS COMPUTATIONS

Table for PROPOSED JUNCTION LOSS COMPUTATIONS with columns: INLET NO., OUTLET WS, Do, Qo, Lo, Sf, Hf, Vo, Ho, Qi, Vi, QVi, Vp2, Hl, ANGLE, K, Hd, Hi, 1.30, 0.50, FINAL H, INLET WATER SURFACE ELEV, RIM ELEV.

Summary table for junction loss computations with columns: Vp2, Vo'2, Vi'2, Hl=0.35, Ho=0.25, H=K, Ht=Ho+Hi+H, 90 K=0.70, 50 K=0.50, 20 K=0.25, 80 K=0.86, 40 K=0.43, 15 K=0.19, 70 K=0.61, 30 K=0.35, 10 K=0.13, 60 K=0.56, 25 K=0.30, 5 K=0.06.



Bowman Consulting Group, Ltd. 14220 Thunderbolt Place Suite 300 Chantilly, Virginia 20151 Phone: (703) 464-1000 Fax: (703) 464-9720 www.bowmanconsulting.com

PROPOSED STORM DRAIN COMPUTATIONS BURKE STATION ROAD STREETScape IMPROVEMENTS CITY OF FAIRFAX VIRGINIA

PROJECT NUMBER

PLAN STATUS

DATE DESCRIPTION

SB DRAW SHD DESIGN DRAWN CHKD

SCALE H: 1"=100' V: NA

JOB No. 6916-01-002

DATE : DECEMBER, 2014

FILE No. 6916-DP-002

SHEET 45 OF 59

PROJECT NUMBER

PLAN STATUS

DATE	DESCRIPTION
SB DESIGN	SB DRAWN
	SD CHKD
SCALE	H: N/A V: N/A
JOB No.	6916-01-002
DATE	: DECEMBER, 2014
FILE No.	6916-D-MP-002

Virginia Runoff Reduction Method ReDevelopment Worksheet - v2.8 - June 2014

Site Data Summary

Total Rainfall = 43 inches

Site Land Cover Summary

	A Soils	B Soils	C Soils	D Soils	Total	% of Total
Forest (acres)	0.00	0.00	0.00	0.00	0.00	0.00
Turf (acres)	0.00	0.00	0.00	0.12	0.12	16.90
Impervious (acres)	0.00	0.00	0.00	0.59	0.59	83.10
					0.71	100.00

Site Rv	0.83
Post Development Treatment Volume (ft ³)	2144
Post Development TP Load (lb/yr)	1.35
Post Development TN Load (lb/yr)	9.63
Total TP Load Reduction Required (lb/yr)	0.12

Total Runoff Volume Reduction (ft ³)	0
Total TP Load Reduction Achieved (lb/yr)	0
Total TN Load Reduction Achieved (lb/yr)	0.00
Adjusted Post Development TP Load (lb/yr)	1.19
Remaining Phosphorous Load Reduction (Lb/yr) Required	0.00

Drainage Area Summary

	D.A. A	D.A. B	D.A. C	D.A. D	D.A. E	Total
Forest (acres)	0.00	0.00	0.00	0.00	0.00	0.00
Turf (acres)	0.12	0.00	0.00	0.00	0.00	0.12
Impervious (acres)	0.59	0.00	0.00	0.00	0.00	0.59
						0.71

Drainage Area Compliance Summary

	D.A. A	D.A. B	D.A. C	D.A. D	D.A. E	Total
TP Load Red. (lb/yr)	0.16	0.00	0.00	0.00	0.00	0.16
TN Load Red. (lb/yr)	0.00	0.00	0.00	0.00	0.00	0.00

Drainage Area A Summary

Land Cover Summary

	A Soils	B Soils	C Soils	D Soils	Total	% of Total
Forest (acres)	0.00	0.00	0.00	0.00	0.00	0.00
Turf (acres)	0.00	0.00	0.00	0.12	0.12	16.90
Impervious (acres)	0.00	0.00	0.00	0.59	0.59	83.10
					0.71	

BMP Selections

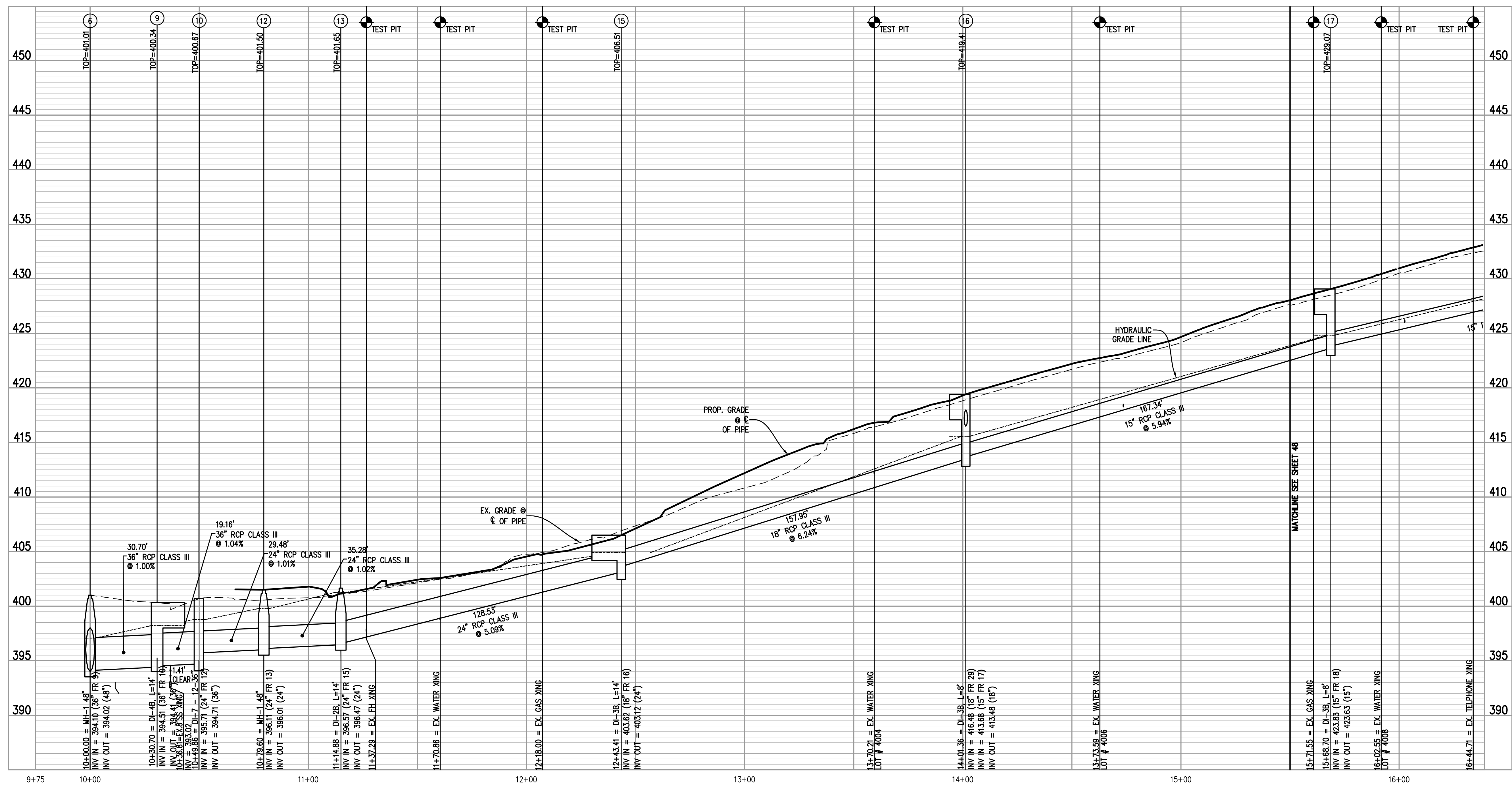
Practice	Credit Area (acres)		Downstream Practice
	Impervious:	Turf (Pervious):	
14. Insert Name of Device	0.14	0.03	

Total Impervious Cover Treated (acres)	0.14
Total Turf Area Treated (acres)	0.03
Total TP Load Reduction Achieved in D.A. A (lb/yr)	0.16
Total TN Load Reduction Achieved in D.A. A (lb/yr)	0.00

STORMWATER QUALITY NARRATIVE

THE BURKE STATION ROAD IMPROVEMENT IS LOCATED IN FAIRFAX VIRGINIA. THE LIMITS OF DISTURBANCE WERE DETERMINED TO BE 30,905 SF (0.71 ACRES). THE ROAD IMPROVEMENTS ARE BOUNDED BY LITTLE RIVER TURNPIKE (RTE 236) AND STOUGHEN ROAD. CURRENTLY THE SITE IS A PAVED ROAD WITH A ROADSIDE DITCH. THE SITE CONTAINS 4,753 SF (0.11 AC) OF MANAGED TURF AND 26,152 SF (0.60) OF IMPERVIOUS AREA. THE DEVELOPMENT WILL CONSIST OF INSTALLING CURB AND GUTTER AND A SIDEWALK ALONG THE WESTERN SIDE OF BURKE STATION ROAD. THE PROPOSED CONDITIONS WILL INCLUDE 5,100 SF (0.12 AC) OF MANAGED TURF AND 25,805 SF (0.59 AC) OF IMPERVIOUS AREA. SINCE THE SITE WAS PREVIOUSLY DEVELOPED THE VIRGINIA RUNOFF REDUCTION METHOD FOR REDEVELOPMENT WILL BE USED TO ANALYZE THIS PROJECT.

USING THE VIRGINIA RUNOFF REDUCTION METHOD REDEVELOPMENT WORKSHEET-V2.8-JUNE 2014 THE REQUIRED TOTAL PHOSPHORUS LOAD REDUCTION WAS DETERMINED TO BE 0.12 LB/YEAR. IN ORDER TO SATISFY THIS REDUCTION REQUIREMENT A BAYFILTER, (A MANUFACTURED DEVICE) IS BEING PROPOSED AT STRUCTURE 19. THE DRAINAGE AREA TO THIS STRUCTURE IS 0.17 ACRES THAT IS LOCATED OUTSIDE THE LIMITS OF DISTURBANCE. CURRENTLY NO BMP DEVICES SERVE THIS AREA.



STORM DRAIN PROFILES (1 OF 2)
BURKE STATION ROAD
STREETScape IMPROVEMENTS
 VIRGINIA
 CITY OF FAIRFAX

PROJECT NUMBER

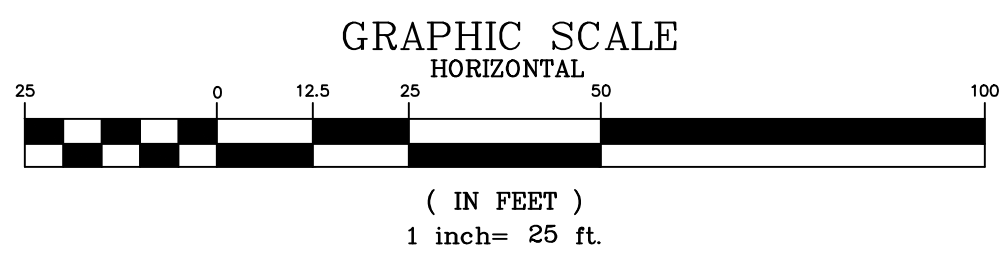
PLAN STATUS

DATE	DESCRIPTION

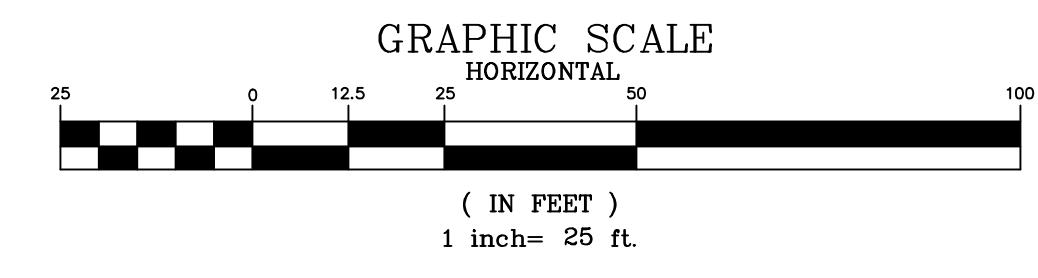
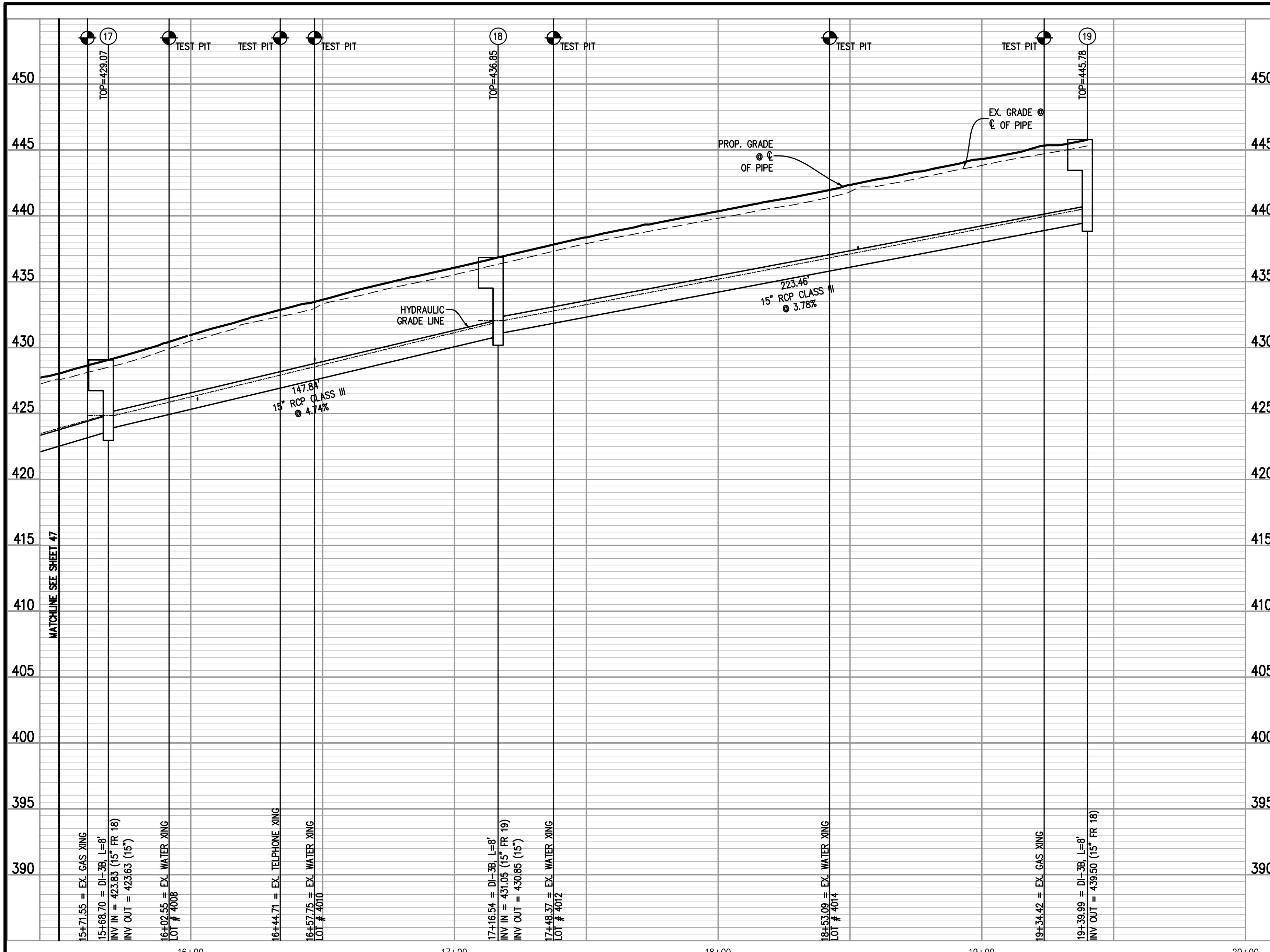
DESIGN	SB	SD

SCALE: H: 1"=25'
V: 1"=5'

JOB No. 6916-01-002
 DATE : DECEMBER, 2014
 FILE No. 6916-D-MP-002



Cad file name : P:\6916 - City of Fairfax On-Call\6916-01-002 (ENG) - Burke Station Road\Engineering\Engineering Plans\6916-D-CP-002-SDP.dwg



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STORM DRAIN PROFILES (2 OF 2)
BURKE STATION ROAD
STREETSCAPE IMPROVEMENTS
CITY OF FAIRFAX
VIRGINIA

PROJECT NUMBER		
PLAN STATUS		
DATE	DESCRIPTION	
SB	SB	SD
DESIGN	DRAWN	CHKD
SCALE	H: 1"=25'	
	V: 1"=5'	
JOB No.	6916-01-002	
DATE :	DECEMBER, 2014	
FILE No.	6916-D-MP-002	
SHEET	48	OF 59

