V:\DesignAid\Plot-Drivers\default_rda.tbl

PROJECT MANAGER<u>.*Wendy_Block_Sanford.City_of_Fairfax_(703)_385-7889_</u></u>* SURVEYED BY Rinker Design Assoc., P.C. (703) 368-7373 (2011) DESIGNED BY _Adam_D.Welschenbach.P.E.Rinker_Design_Assoc..P.C.(703)_368-7373_ SUBSURFACE UTILITY PROVIDED BY_Accumark_(2011)_____

GRADING GENERAL NOTES

- G-I The grade line denotes top of finished pavement unless shown otherwise on typical sections or plans.
- G-2 Not Used.
- G-3 Earthwork quantities on this project are based on anticipated settlement and may require adjusting during construction. Payment will be made only for quantities actually moved.
- G-4 The cost of removal of all existing concrete items located in the area to be graded, including, but not limited to the following, shall be included in the price bid for regular excavation: concrete, drainage box, headwalls, parking stops, sidewalk, entrance aprons, <u>concrete pads, curb, asphalt curb, concrete curb/qutter etc.</u>
- G-5 The excavation of unsuitable material as specified on these plans is based on previously conducted subsurface soil investigation. If, during construction, it is deemed necessary to change the depth more than I foot (0.3 m) or the limits of such excavation, such change shall be made at the direction of the Engineer and measurement and payment shall be made in accordance with Section 303 of the applicable VDOT Road and Bridge Specifications.
- G-6 The borrow or embankment material for this project shall be a minimum CBR <u>4.2</u> or as approved by the Materials Engineer. All borrow materials shall have a liquid limit (LL) value of less than 45 and plasticity index (PI) value of less than 20 in their natural state.
- G-7 Not Used.
- G-8 All side slope fills shall be benched in accordance with section 303.04(h) of the VDOT Road and Bridge Specifications

DRAINAGE GENERAL NOTES

- D-I The horizontal location of all drainage structures shown on these plans is approximate only, with the exception of structures showing specific stations, special design bridges and storm sewer systems.
- D-2 The horizontal location and invert elevations shown for proposed culverts and storm sewer outfall pipes are based on existing survey data and required design criteria. If, during construction, it is found that the horizontal location or invert elevations shown on the plans differ significantly from the horizontal location or elevations of the stream or swale in which the culvert or storm sewer outfall pipe is to be placed, the Engineer shall confer with, and get approval from the applicable City Engineer before installing the culvert or storm sewer outfall pipe.
- D-3 The "H" dimensions shown on the plans for drop inlets and junction boxes and the "L.F.(m)" dimensions shown for manholes are for estimating purposes and are based on the proposed invert elevations shown for the structure and the anticipated top (rim) elevation based on existing or proposed finished grade. The actual "H" or "L.F.(m)" dimensions are to be determined by the contractor from field conditions.
- D-4 Not Used
- D-5 Not Used.
- D-6 Pipes shall conform to any of the allowable types shown on sheet number 2K(6) within the applicable height of cover limitations. For strength, sheet thickness, or class designation, available sizes, height of cover limitations and other restrictions for a particular pipe type or height of cover, see the VDOT Road and <u>Bridge Standard</u> PC-1.Structural plate pipe may be substituted for corrugated pipe of the same size and a structural plate pipe arch may be substituted for a corrugated pipe arch of the same size, provided the substitution complies with the applicable sections of the VDOT Road and Bridge Standard PC-I.
- D-7 Not Used.
- D-8 Not Used
- D-9 Not Used
- D-10 The proposed riprap may be omitted by the Engineer if the slope designated for placement of riprap is found to be comprised of solid rock or closely consolidated boulders with soundness, size and weight equal to, or exceeding, the specifications for the proposed riprap.
- D-II Not Used
- D-12 All existing drainage facilities labeled "To Be Abandoned" shall be left in place. backfilled and plugged in accordance with the VDOT Road and Bridge Standard PP-1. Basis of Payment will be C.Y. of Flowable Backfill.
- D-13 Existing drainage facilities being utilized as a part of the drainage system, and designated on the plans "To Be Cleaned Out", shall be cleaned as directed by the Engineer. The cost incidental to this shall be included in the contract price for other items.
- D-14 Proposed drop inlets with a height (H) less than the standard minimum shown in the VDOT <u>Road and Bridge Standards</u> shall be considered and paid for as Standard Drop Inlets for the type specified. Pipes with less than standard minimum finished height of cover shall be noted as such in the drainage description for the pipe. Specific pipe bedding and cover requiremtns are provided in the applicable PB-l and PC-l standard drawings of the VDOT Road and Bridge Standards.
- D-15 Not Used.
- D-16 When Standard CG-6 or CG-7 is specified on a radius (such as at a street intersection), the Engineer may approve a decrease in the cross slope of the gutter to facilitate proper drainage.
- D-17 St'd SL-I Safety slab locations are based on the assumed use of precast structures. If cast-in-place structures are utilized, and the interior chamber dimensions (length and width, or diameter) are less than 4 feet, the safety slabs shall not be installed.

Project General Notes and Legend

PAVEMENT GENERAL NOTES

- P-I Not Used.
- P-2 The pavement materials on this project will be paid for on a tonnage basis. The weight will vary in accordance with the specific gravity of the aggregates and the asphaltic content of the mix actually used to secure the design depth. The weight of the asphalt concrete is based on 95% of theoretical maximum density.(See IIM-LD-158)

INCIDENTAL GENERAL NOTES

- I-I Not Used.
- 1-2 Not Used.
- 1-3 Not Used.
- I-4 All trees located within the Clear Zone or within a minimum of 30 feet of the edge of pavement, within the limits of the right of way or construction easement, unless otherwise noted on plans or directed by the Engineer, shall be removed, as provided for in Section 301 of the applicable VDOT Road and Bridge Specifications.
- 1-5 Not Used.
- I-6 Certain trees shall be preserved as noted on plans or as directed by the Engineer.
- When Standard slope roundoffs would damage trees, bushes or other desirable vegetation, they shall be omitted when so ordered by the Engineer.
- I-8 Not Used.
- I-8A Not Used.
- 1-9 When no centerline alignment is shown for a proposed entrance, the entrance shall be constructed in the same location as the existing entrance.
- I-IO Not Used.
- I-II VOID
- I-I2 St'd.RM-2 Right of Way Monuments shall be set by the Contractor.
- 1-13 Not Used.
- I-I4 Salvaged guardrail materials not used in the new construction shall become the property of the Contractor and shall be disposed of at a licensed landfill, recycled or be retained by the Contractor.
- I-I5 Not Used.
- I-16 The "Underground Utilities" survey data on this project has been provided by consultant and copies are available from the Department/City.
- I-I7 For method of constructing Straight-Line Taper Lanes in Curb and/or Curb and gutter sections, see typical details on Sheet 2A(4).
- I-18 All pavement markings and traffic flow arrows shown on the roadway construction plans are schematic only. The actual location and application of pavement markings shall be in accordance with Section 704 of the applicable VDOT Road and Bridge Specifications, MUTCD, sequence of construction/traffic control plans, pavement marking plan sheets 10 thru 10(3) and as directed by the
- I-19 The following outside sources, under contract with VDOT, have provided information on this project.

Hydraulic Design: Rinker Design Associates, P.C. (RDA) Roadway Design: RDA (Water/Sanitary Only) Utility Design: Utility Designation: <u>Accumark</u> Utility Location: <u>Accumark</u> Survey: Bridge Design: Geotechnical Investigation: <u>DMY Engineering Consultants, L.L.C.</u> Traffic Design: <u>Vanasse Hangen Brustlin,Inc.,and RDA</u> Landscape Desian:

If questions or problems arise during construction, please contact the City

Engineer. DO NOT CONTACT THE OUTSIDE SOURCES. I-20 The Official Electronic .pdf Version of the plans will override the paper copies or prints of specific layers. Portions of this plan assembly have been CADD generated. To assist in the preparation of the bid and construction of the project, Microstation format (.dgn)

files will be made available to the prime contractor during bids and after award of the contract.

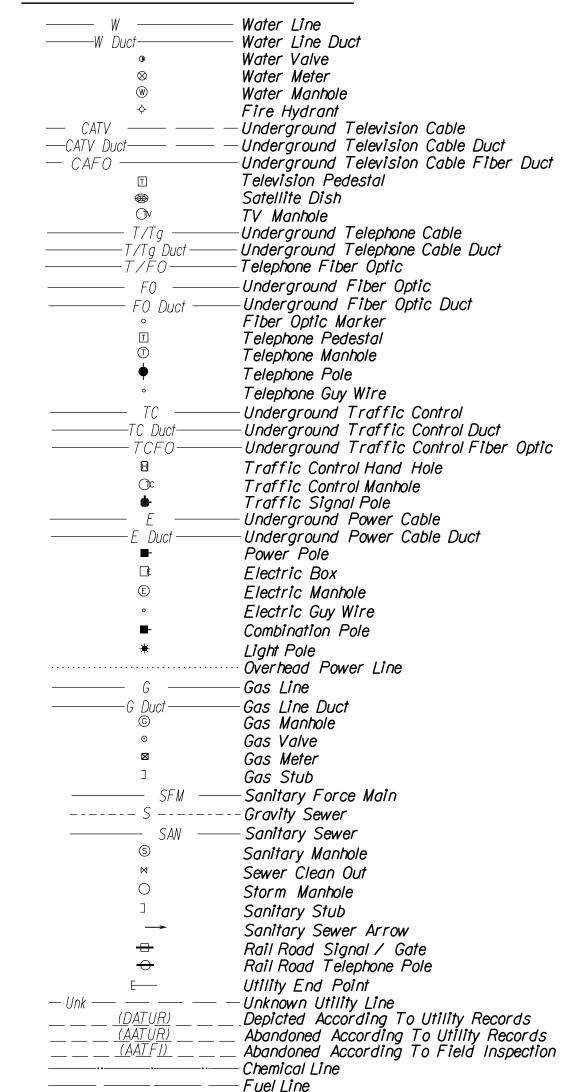
I-2I All electronic plan assemblies will include the construction plans in two formats: .pdf files and Microstation format (.dqn) files. Only the .pdf files will be considered as part of the official plan assembly.

The Microstation format (.dgn) files are furnished only as information for the contractor. These plans are developed in layers (levels) to aid in readability. However, the construction items may or may not be in the proper layering scheme as described in the VDOT CADD Manual. The Microstation files will only match the scanned files if all required levels are turned on A Microstation Software license is required to be able to read these files.

EROSION AND SEDIMENT CONTROL (ESC) GENERAL NOTES

E See Sheet IP(2) for the Erosion and Siltation Control General Notes.

Utility Legend



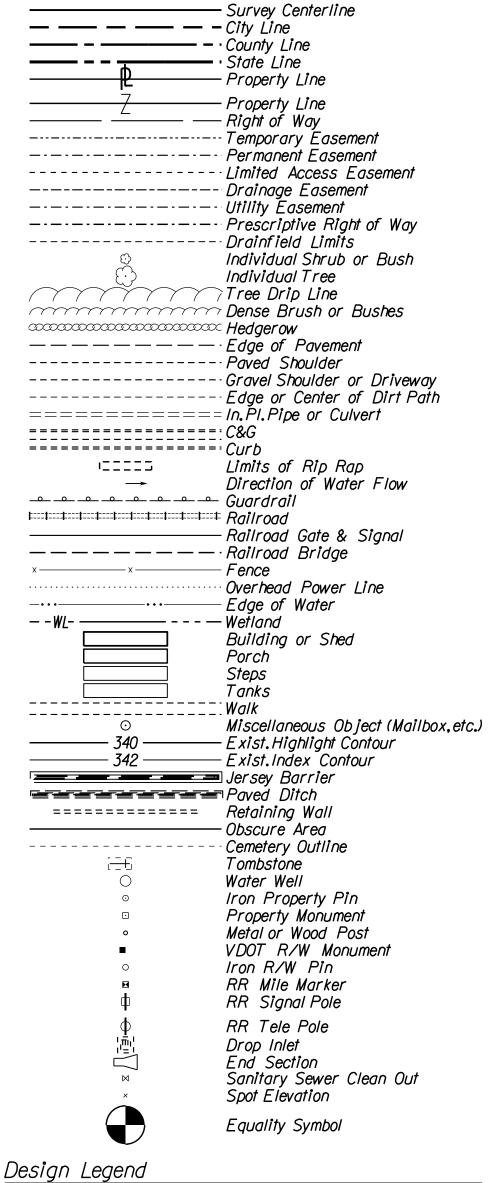
----Transmission Towers

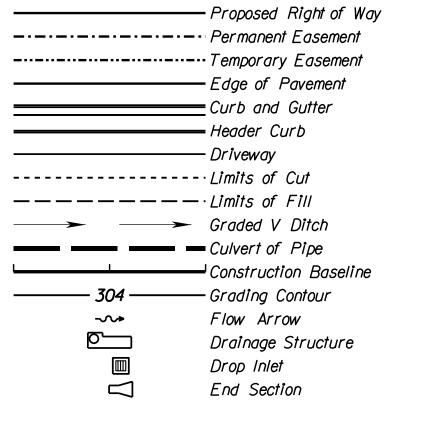
25.0000 FT / in.

REVISED STATE ROUTE PROJECT 0029-151-105 P101, P102, R201, C501 VA.

DESIGN FEATURES RELATING TO CONSTRUCTION OR TO REGULATION AND CONTROL OF TRAFFIC MAY BE SUBJECT TO CHANGE AS DEEMED NECESSARY BY THE DEPARTMENT

Survey Legend





PROJECT SHEET NO. 0029-151-105

Dulles, Virginia

DMY Engineering Consultants Inc. Rinker Design Associates, P.C.

GEOTECHNICAL ENGINEER | PROFESSIONAL ENGINEER

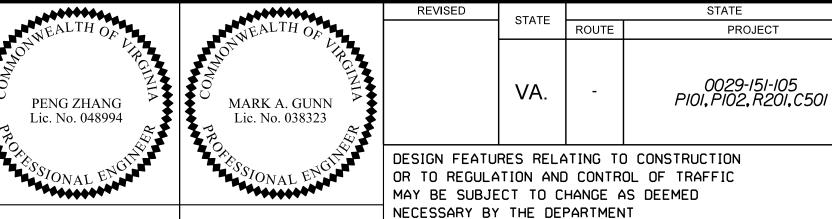
Manassas, Virginia

2A

Tie to – Exist.Ground

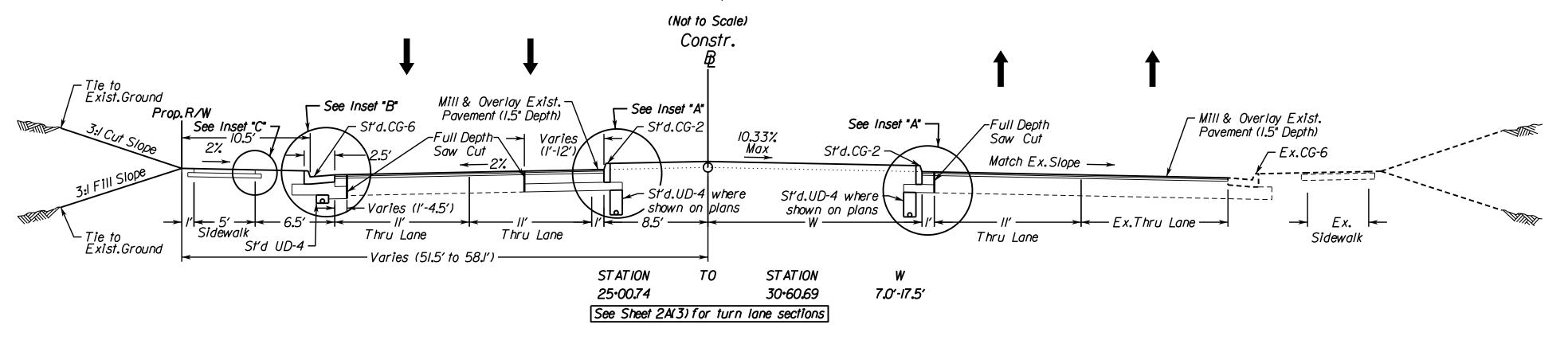
NOTE: PAVEMENT WIDTHS VARY AT TURN LANES, TAPERS AND CONNECTIONS. SEE PLAN SHEETS AND CROSS SECTIONS FOR PAVEMENT LENGTHS AND WIDTHS.

Typical Sections



Fairfax Blvd., U.S. Route 29/50

Normal Crown, Four-Lane Section VDOT Std.GS-5, V=35 MPH



Fairfax Blvd., U.S. Route 29/50 Normal Crown, Four-Lane Section

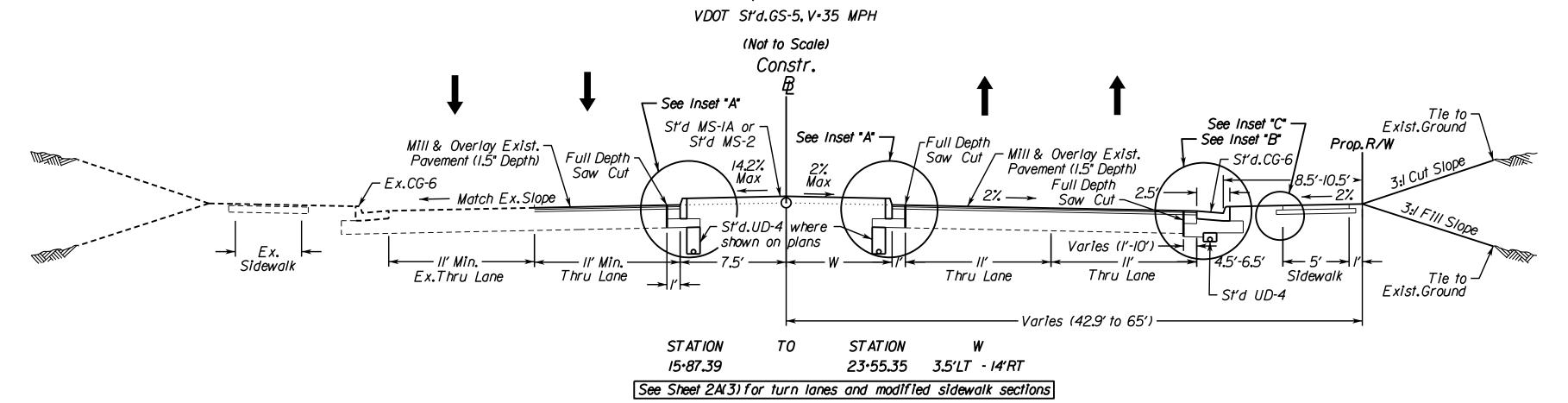
VDOT Std.GS-5, V=35 MPH (Not to Scale) Constr. Tie to ¬ Exist.Ground - See Inset "A" — Mill & Overlay Exist. — Mill & Overlay Exist. Pavement (1.5" Depth) Pavement (1.5" Depth) St'd.CG-6 - See Inset "A" ′ ← Match Ex.Slope| <u>Full Depth</u> Saw Cut Full Depth -Saw Cut Saw Cut Varies (l'-2') Varies (l'-2' Tie to Exist.Ground Tie to -Exist.Ground Thru Lane Ex.Thru Lane Thru Lane Thru Lane Turn Lane Turn Lane └ St'd UD-4 -*Varies (52.7' to 52.9')* -STATION STATION

25.00.74

Fairfax Blvd., U.S. Route 29/50 Normal Crown, Four-Lane Section

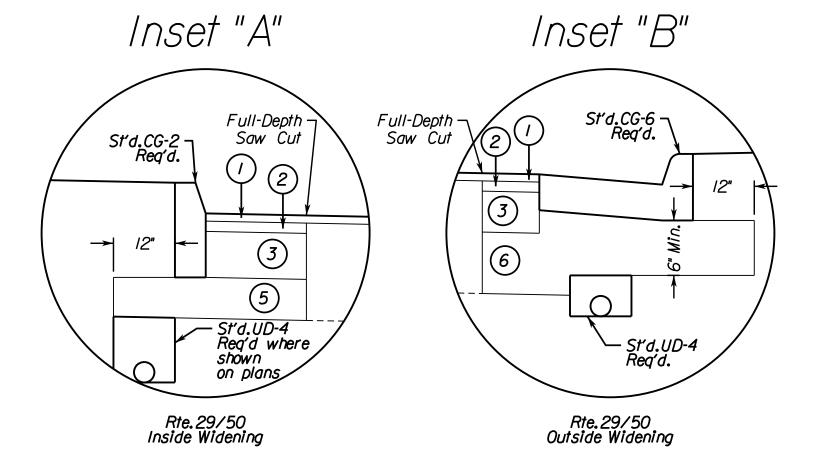
See Sheet 2A(3) for turn lane sections

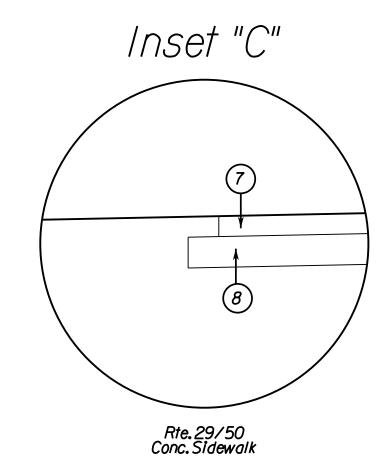
23•55**.**35



- Surface Course (1.5") Asph. Conc., Type SM-9.5D
- Intermediate Course (2") Asph.Conc., Type IM-19.0A
- Base Course (9") Asph. Conc., Type BM-25.0A
- Base Course (IO*) Asph.Conc.,Type BM-25.0A
- Sub-base Course (8") Aggregate Base Material, Type F, Size 2IA pugmill mixed with 4% hydraulic cement by weight (CTA) extended (I2") behind the curb
- Sub-base Course (12") Aggregate Base Material, Type I, Size No. 21B Connected to UD-4 underdrain.
- Sidewalk (4") Class A3 Hydraulic Cement Concrete
- (4") Aggr.Base Material,
 Type I,Size No.2I-B extended (4") either side of the Sidewalk

Note: All pavement widening shall be constructed in accordance with St'd WP-2. See Sheet 2A(4) for details.





PROJECT 0029-151-105

SHEET NO.

Rte.29/50 & Rte.123 Conc.Sidewalk

0029-151-105

SHEET NO.

- *16.5′-20.5*

-*Varies (43'-53.*8') —

Prop.Twin -

-8'x8' Sta. 30+60.69 to 36+42.56 -Transition Sta. 36+42.56 to 37+38.56 -12'x6' Sta. 37+38.56 to 40+88.48

Box Culvert

Dulles, Virginia

DMY Engineering Consultants Inc. Rinker Design Associates, P.C.

GEOTECHNICAL ENGINEER | PROFESSIONAL ENGINEER

Manassas, Virginia

Inset "C"

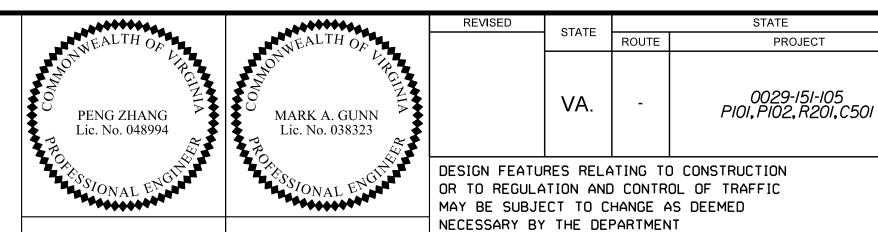
Rte.123 Conc.Sidewalk

Rte.123 Outside Widening

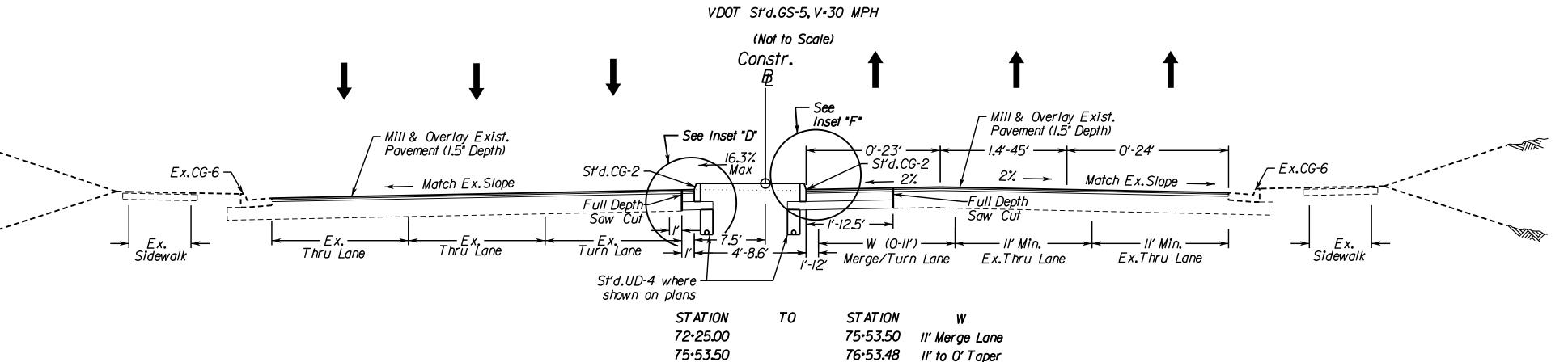
2A(2)



Typical Sections Cont.



Chain Bridge Road, Route 123 Normal Crown, Four-Lane Section



Surface Course - (1.5") Asph. Conc., Type SM-9.5D

Intermediate Course - (2") Asph.Conc., Type IM-19.0A

Base Course - (IO*) Asph. Conc., Type BM-25.0A

Sub-base Course - (8") Aggregate Base Material, Type F, Size 2IA pugmill mixed with 4% hydraulic cement by weight (CTA) extended (I2") behind the curb

Sub-base Course - (12") Aggregate Base Material, Type I, Size No. 21B Connected to UD-4 underdrain.

Sidewalk - (4") Class A3 Hydraulic Cement Concrete

(4") Aggr.Base Material, Type I.Size No.2I-B extended (4") either side of the Sidewalk

Note: All pavement widening shall be constructed in accordance with St'd WP-2. See Sheet 2A(4) for details.

Chain Bridge Road, Route 123 Normal Crown, Four-Lane Section VDOT Std.GS-5, V=30 MPH

77.03.32 O' to II' Taper

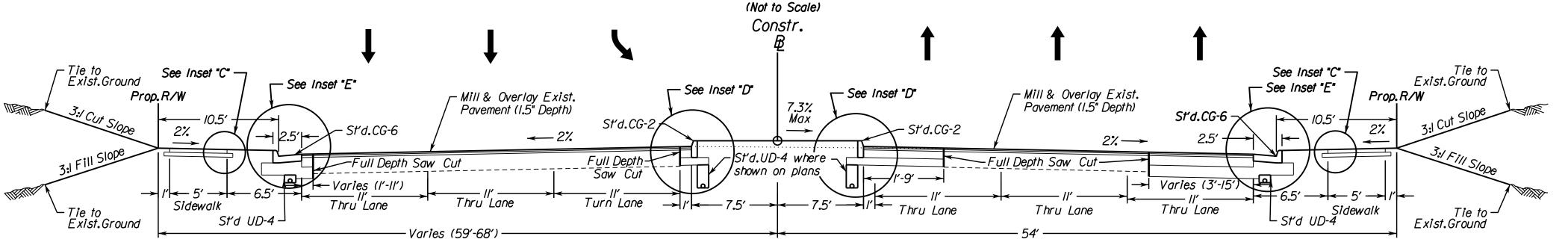
77•70.00 II' Turn Lane

76•53.48

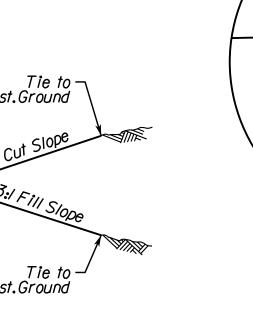
77•*03.32*

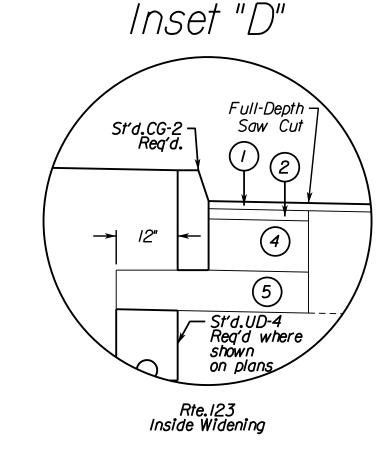
STATION

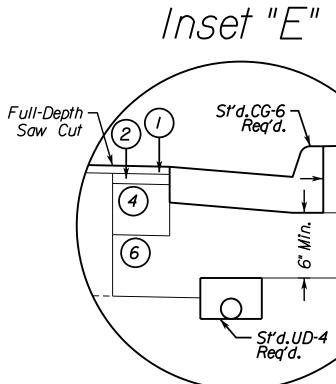
67•*38.4*5

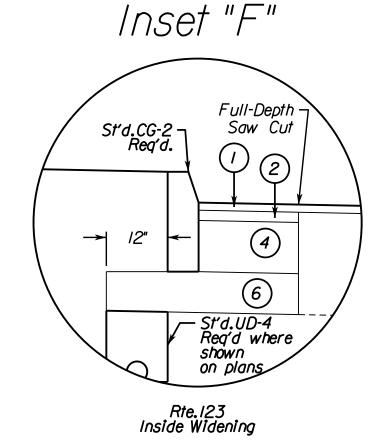


See Sheet 2A(3) for turn lanes and modified sidewalk sections





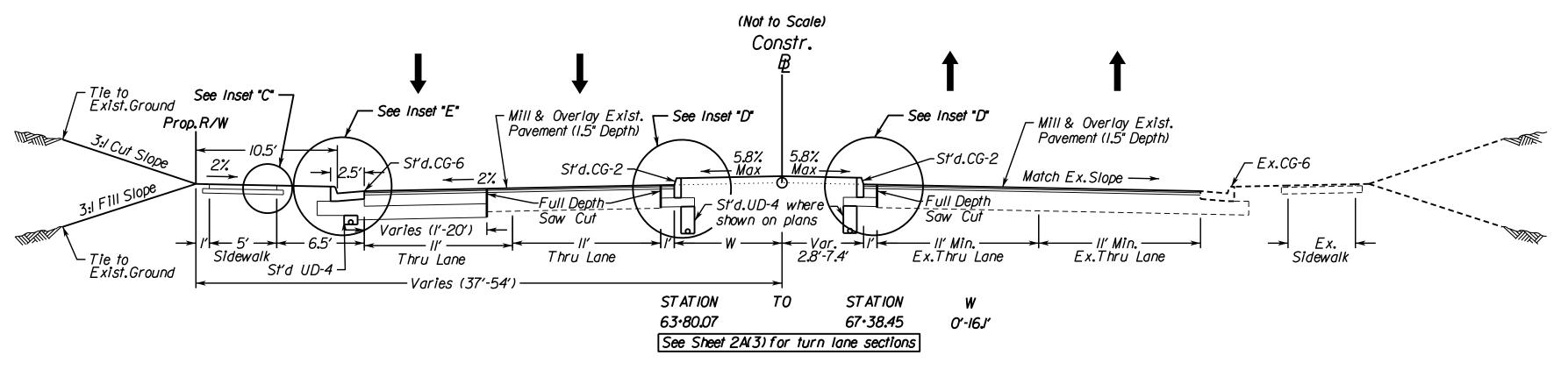




Chain Bridge Road, Route 123 Normal Crown, Four-Lane Section VDOT St'd.GS-5, V=30 MPH

STATION

72•25.00



PROJECT 0029-151-105 SHEET NO.

STATE

PROJECT

0029-151-105 P101, P102, R201, C501

2A(3)

ROJECT MANAGER<u>.W*endy_Block_Sanford,City_of_Fairfax_(703)_385-7889_</u></u>* SURVEYED BY *Rinker <u>Design Assoc.,</u>P.C.(703) 368-7373 (2011)* DESIGNED BY _Adam_D.Welschenbach, P.E., Rinker_Design_Assoc., P.C.(703)_368-7373_ SUBSURFACE UTILITY PROVIDED BY_*Accumark_(2011)*______

NOTE: PAVEMENT WIDTHS VARY AT TURN LANES, TAPERS AND CONNECTIONS. SEE PLAN SHEETS AND CROSS SECTIONS FOR PAVEMENT LENGTHS AND WIDTHS.

Lee Highway, U.S. Route 29/50

WB Left Turn Lane VDOT Std.GS-5, V=35 MPH

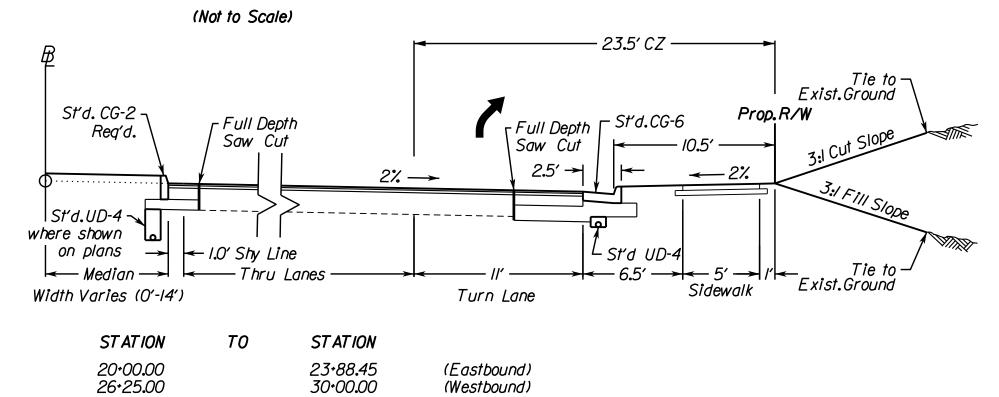
(Not to Scale) Match Ex.Slope → _Full Depth — _Saw_Cut -<u>Full Depth</u> Saw Cut — I.O' Shy Line shown on plans (3.5'-14') St'd.MS-I or

26+25.00

32+15.00

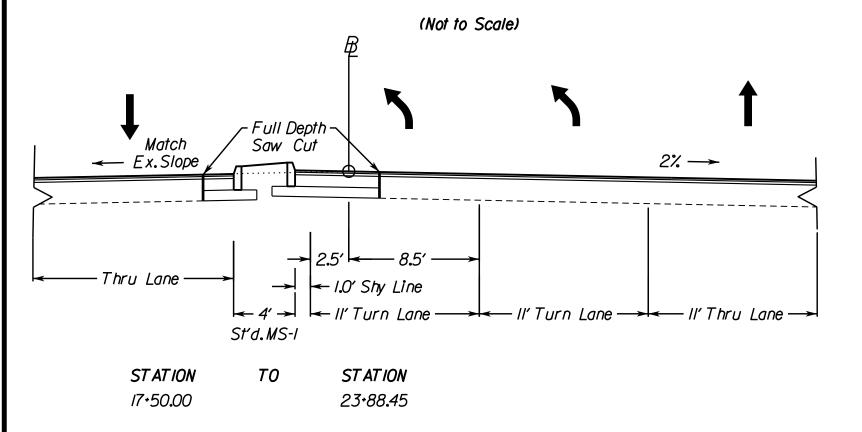
Lee Highway, U.S. Route 29/50

Right Turn Lane VDOT Std.GS-5, V=35 MPH



Lee Highway, U.S. Route 29/50

EB Dual Left Turn Lane VDOT St'd.GS-5, V=35 MPH

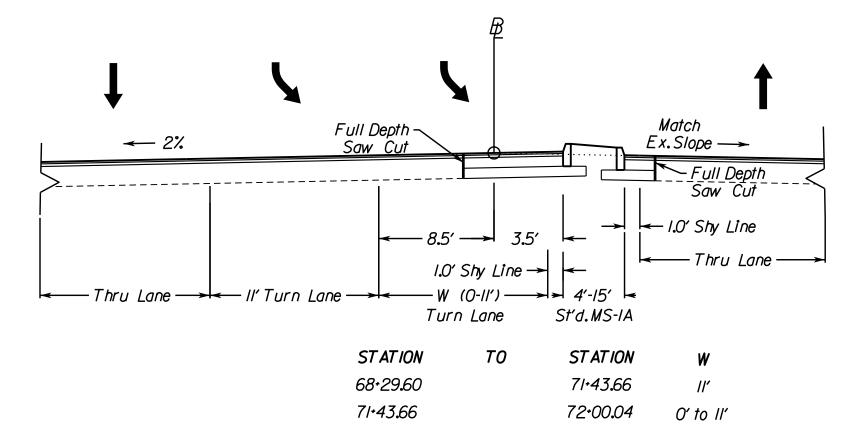


Typical Sections Cont.

Chain Bridge, Route 123

SB Dual Left Turn Lane VDOT St'd.GS-5, V=30 MPH

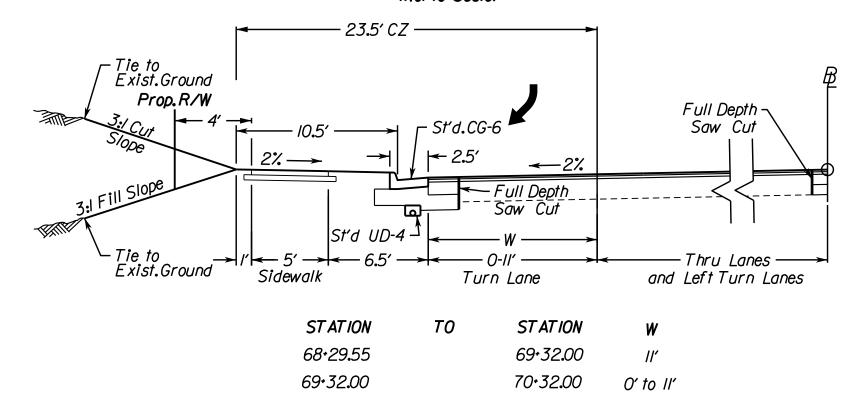
(Not to Scale)



Chain Bridge, Route 123

SB Right Turn Lane VDOT Std.GS-5, V=30 MPH

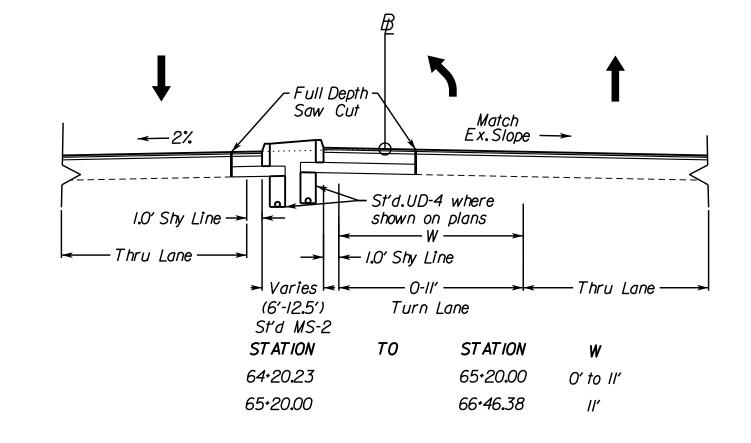
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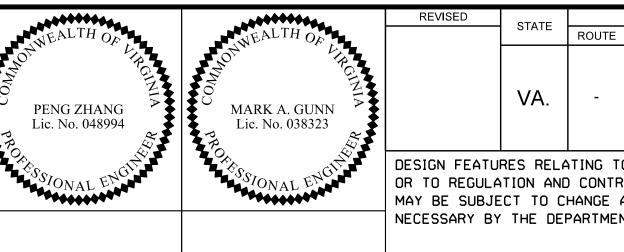


Chain Bridge, Route 123

(Not to Scale)

NB Left Turn Lane VDOT St'd.GS-5, V=30 MPH

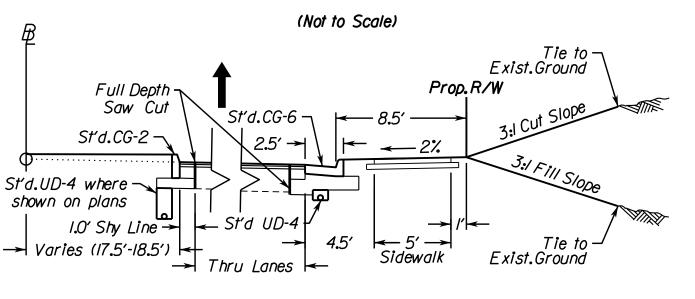




DESIGN FEATURES RELATING TO CONSTRUCTION OR TO REGULATION AND CONTROL OF TRAFFIC MAY BE SUBJECT TO CHANGE AS DEEMED NECESSARY BY THE DEPARTMENT DMY Engineering Consultants, Inc. Rinker Design Associates, P.C. Manassas, Virginia GEOTECHNICAL ENGINEER | PROFESSIONAL ENGINEER

Lee Highway, U.S. Route 29/50

EB Modified Bench Section VDOT St'd.GS-5, V=35 MPH

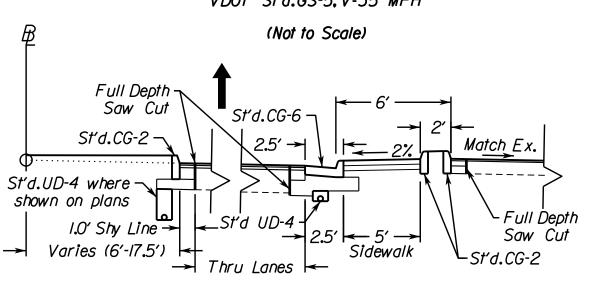


STATION 19+81.57

STATION 18+21**.**63

Lee Highway, U.S. Route 29/50

EB Curb Abutted Sidewalk Section VDOT St'd.GS-5, V=35 MPH

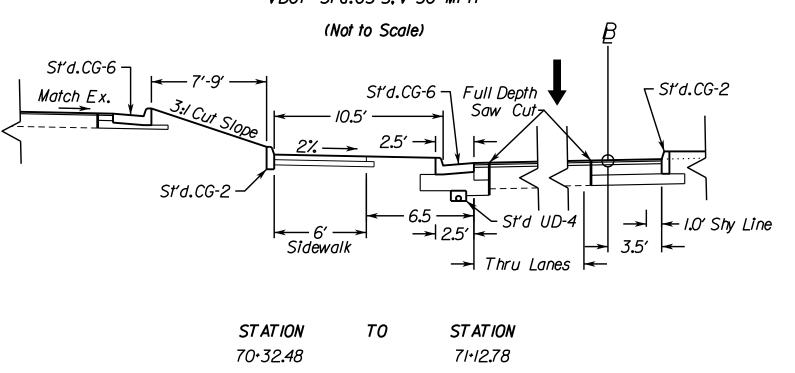


STATION *16+83.40*

ΤO STATION *18+21.63*

Chain Bridge, Route 123

SB 6' Sidewalk w/ CG-2 Section VDOT St'd.GS-5, V=30 MPH



PROJECT 0029-151-105 SHEET NO.

DMY Engineering Consultants Inc. Rinker Design Associates, P.C.

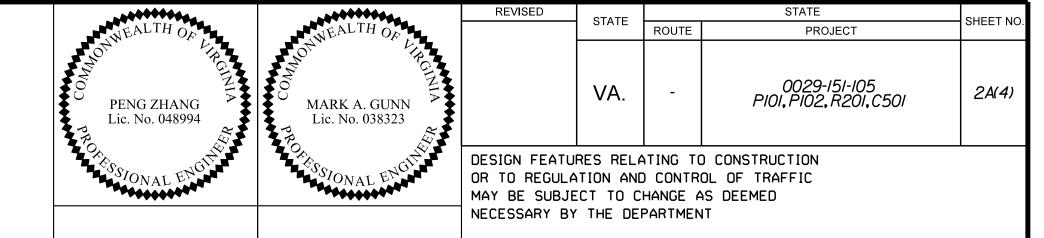
GEOTECHNICAL ENGINEER | PROFESSIONAL ENGINEER

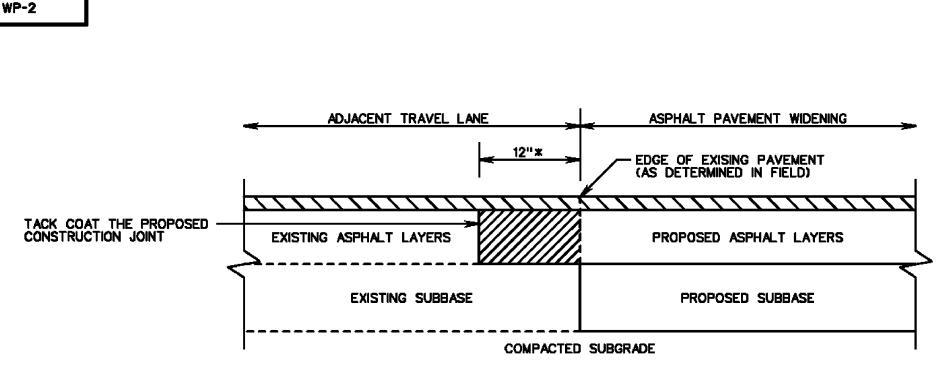
Dulles, Virginia

Manassas, Virginia

PROJECT MANAGER <u>Wendy Block Sanford City of Fairfax (703) 385-7889</u>
SURVEYED BY <u>Rinker Design Assoc. P.C.(703) 368-7373 (2011)</u>
DESIGNED BY <u>Adam D. Welschenbach P.E. Rinker Design Assoc. P.C.(703) 368-7373</u>
SUBSURFACE UTILITY PROVIDED BY <u>Accumark (2011)</u>

Typical Sections Cont.





CONSTRUCTION JOINT DETAIL

REMOVE EXISTING ASPHALT LAYERS TO EXISTING SUBBASE AND REPLACE WITH PROPOSED ASPHALT WIDENING LAYERS

PROPOSED MINIMUM 1 1/2 INCH THICK ASPHALT SURFACE COURSE (SEE NOTE 5)

* MINIMUM 12 INCHES, OR GREATER AS NECESSARY TO ABUT THE FULL THICKNESS OF EXISTING ASPHALT LAYERS AS DETERMINED BY CORES (SEE NOTE 3)

NOTES:

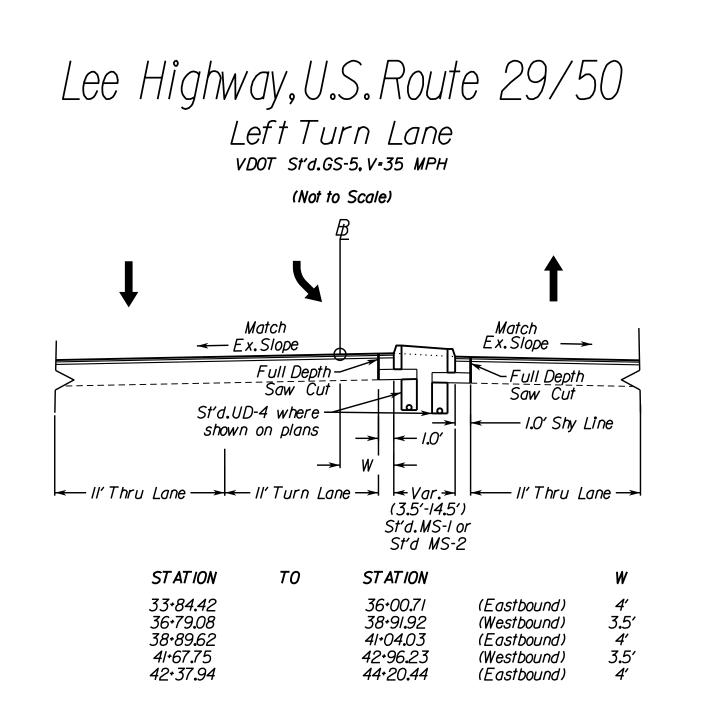
- 1. ASPHALT PAVEMENT WIDENING SHALL HAVE A PAVEMENT DESIGN IN ACCORDANCE WITH CURRENT VDOT PROCEDURES AND BE APPROVED BY THE ENGINEER.
- 2. THE PAVEMENT DESIGN FOR ASPHALT PAVEMENT WIDENING SHALL MEET OR EXCEED THE DEPTHS AND TYPES OF THE LAYERS OF EXISTING PAVEMENT. SUBSURFACE DRAINAGE OF THE EXISTING AND PROPOSED PAVEMENT SHALL BE ADDRESSED IN THE PAVEMENT DESIGN.
- 3. A MINIMUM OF THREE CORES SHALL BE TAKEN ALONG THE CENTER OF THE ADJACENT TRAVEL LANE TO DETERMINE THE TYPE AND THICKNESS OF EXISTING PAVEMENT LAYERS. THESE CORES SHALL BE SPACED NO MORE THAN 500 FEET APART.
- 4. THE ADJACENT TRAVEL LANE SHALL BE MILLED A MINIMUM DEPTH OF 1 1/2 INCHES AND REPLACED WITH AN ASPHALT SURFACE COURSE TO MATCH THE PROPOSED PAVEMENT WIDENING SURFACE COURSE, UNLESS WAIVED BY THE ENGINEER.
- 5. THE ENGINEER MAY REQUIRE THE MILLING DEPTH OF THE EXISTING PAVEMENT TO BE ADJUSTED TO ACHIEVE AN ACCEPTABLE PAVEMENT CROSS-SLOPE
 AND EFFECTIVE SURFACE DRAINAGE.
- 6. EXISTING PAVEMENT MARKINGS AND MARKERS WITHIN THE PROJECT LIMITS SHALL BE RESTORED SUBJECT TO THE APPROVAL OF THE ENGINEER.
- 7. FINAL TRANSVERSE PAVEMENT TIE-IN SHALL CONFORM TO THE REQUIREMENTS OF SECTION 315.05(c) OF THE SPECIFICATIONS EXCEPT THAT ALL JOINTS AT TIE-IN LOCATIONS SHALL BE TESTED USING A 10 FOOT STRAIGHTEDGE IN ACCORDANCE WITH THE REQUIREMENTS OF SECTION 315.07(d) OF THE SPECIFICATIONS.

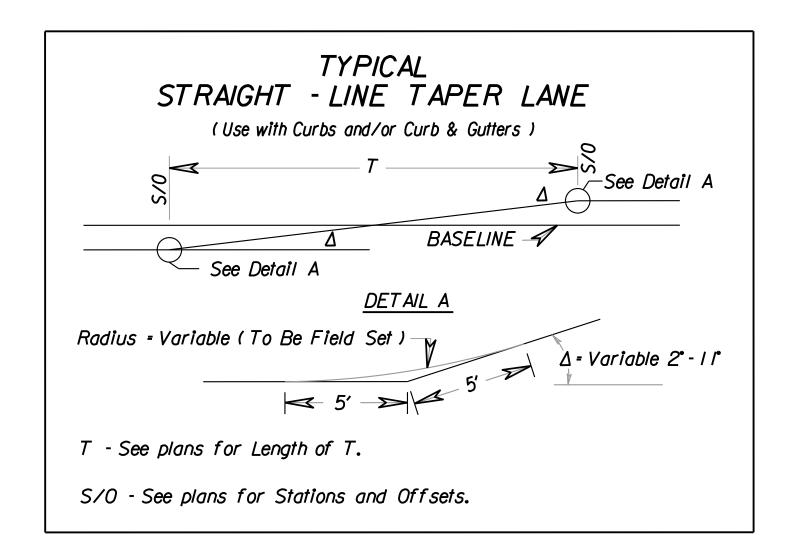
PAVEMENT WIDENING	SPECIFICATION REFERENCE
	315
ARDS N DATE	PAVEMENT WIDENING N DATE VIRGINIA DEPARTMENT OF TRANSPORTATION

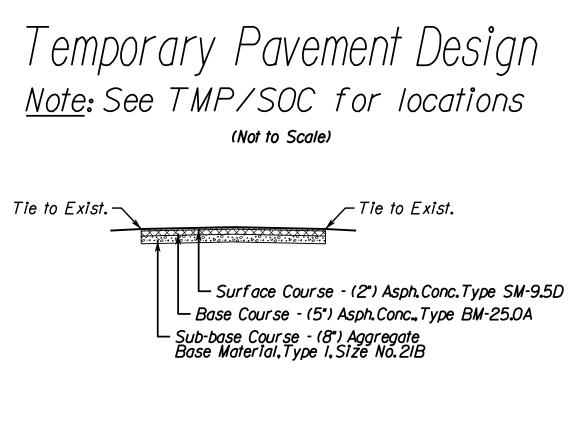
Std.CG-6 Prop.R/W Std.CG-6 Prop.R/W Std.CG-6 Std.CG-6 Std.CG-6 Std.CG-6 Saw Cut Std.UD-4 where shown on plans Std UD-4 Std UD-4 Std.UD-4 Std.UD

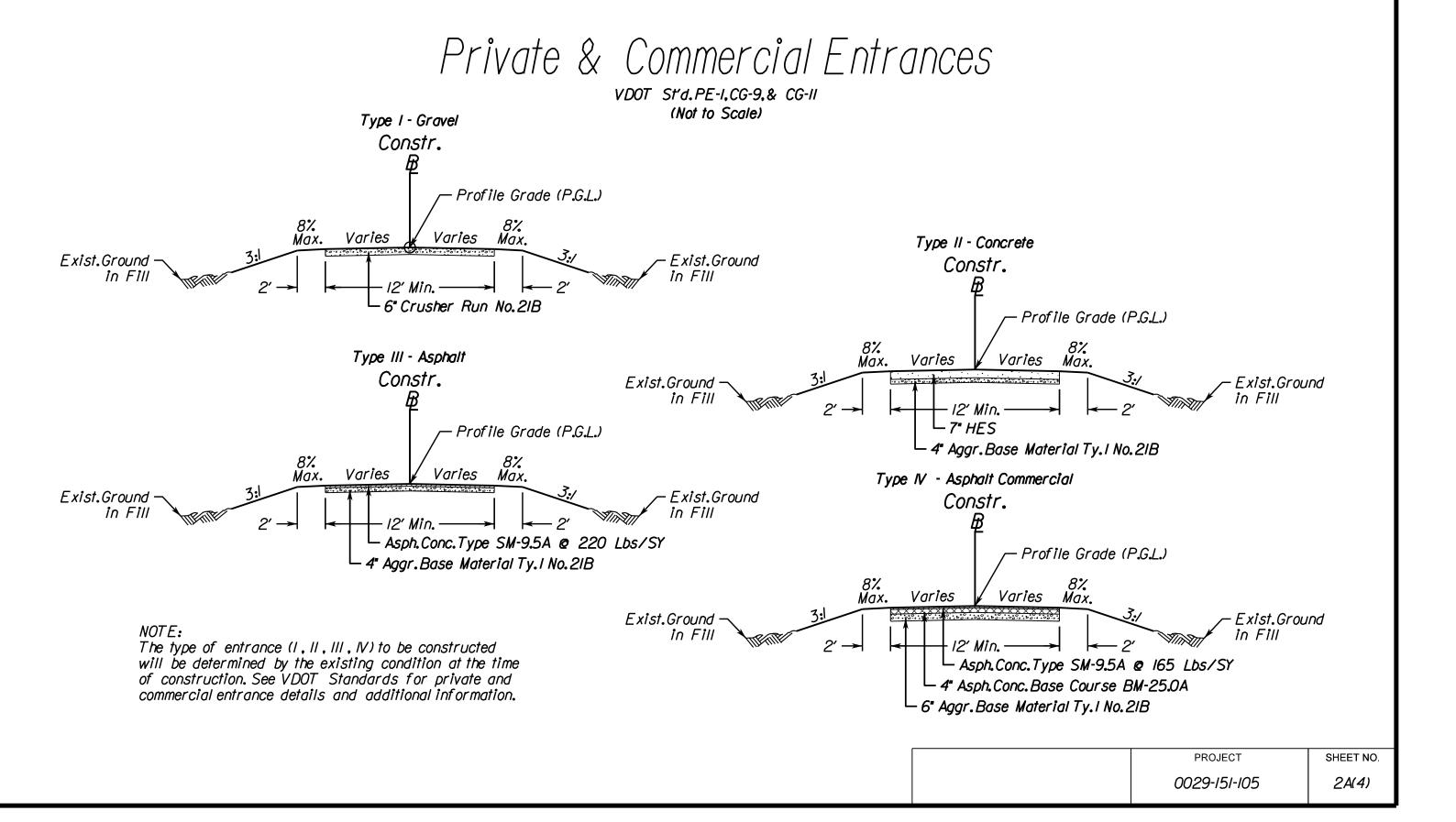
Lee Highway, U.S. Route 29/50

WB Modified Bench Section









PROJECT MANAGER<u>Wendy Block Sanford.City of Fairfax (703) 385-7889</u>_

DESIGNED BY _Adam_D.Welschenbach.P.E.Rinker_Design_Assoc..P.C.(703)_368-7373_

SURVEYED BY Rinker Design Assoc., P.C. (703) 368-7373 (2011)

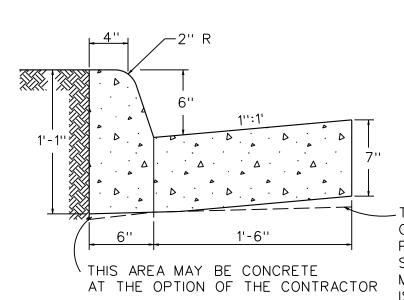
SUBSURFACE UTILITY PROVIDED BY_Accumark_(2011)______

Typical Sections Cont.

MAY BE SUBJECT TO CHANGE AS DEEMED

NECESSARY BY THE DEPARTMENT

Rinker Design Associates, P.C. Manassas, Virginia PROFESSIONAL ENGINEER



105 & 502 THE BOTTOM OF THE CURB AND GUTTER MAY BE CONSTRUCTED PARALLEL TO THE SLOPE OF SUBBASE COURSES PROVIDED A MINIMUM DEPTH OF 7"

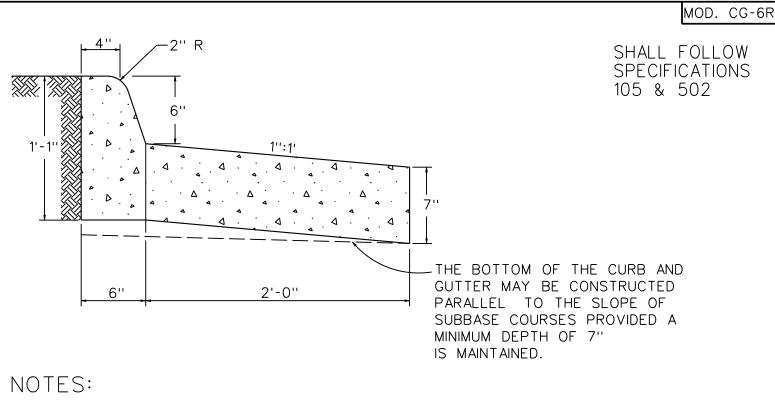
MOD. CG-6

SHALL FOLLOW SPECIFICATIONS

NOTES:

- 1. THIS ITEM MAY BE PRECAST OR CAST IN PLACE.
- 2. CONCRETE TO BE CLASS A3 IF CAST IN PLACE, 4000 PSI IF PRECAST.
- 3. COMBINATION CURB & GUTTER HAVING A RADIUS OF 300 FEET OR LESS (ALONG FACE OF CURB) SHALL BE PAID FOR AS RADIAL COMBINATION CURB & GUTTER.
- 4. FOR USE WITH STABILIZED OPEN-GRADED DRAINAGE LAYER, THE BOTTOM OF THE CURB AND GUTTER <u>SHALL</u> BE CONSTRUCTED PARALLEL TO THE SLOPE OF SUBBASE COURSES AND TO THE DEPTH OF THE PAVEMENT.
- 5. ALLOWABLE CRITERIA FOR THE USE OF CG-6 IS BASED ON ROADWAY CLASSIFICATION AND DESIGN SPEED AS SHOWN IN APPENDIX A OF THE VDOT ROAD DESIGN MANUAL IN THE SECTION ON GS URBAN STANDARDS.

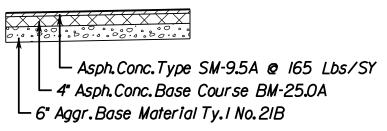
MODIFIED CG-6 COMBINATION 6" CURB & GUTTER



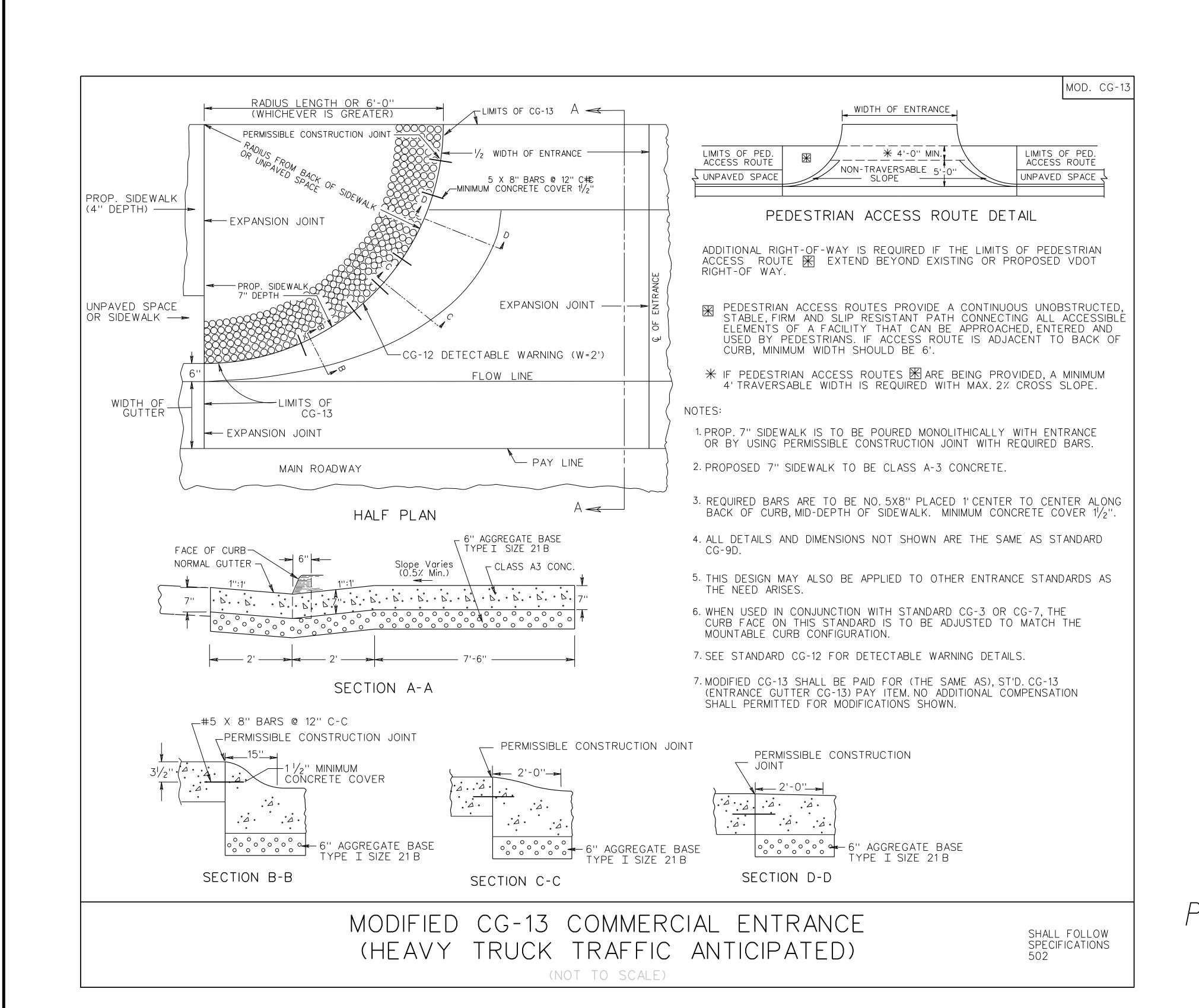
- 1. THIS ITEM MAY BE PRECAST OR CAST IN PLACE.
- 2. CONCRETE TO BE CLASS A3 IF CAST IN PLACE, 4000 PSI IF PRECAST.
- 3. COMBINATION CURB & GUTTER HAVING A RADIUS OF 300 FEET OR LESS (ALONG FACE OF CURB) SHALL BE PAID FOR AS RADIAL COMBINATION CURB & GUTTER.
- 4. FOR USE WITH STABILIZED OPEN-GRADED DRAINAGE LAYER, THE BOTTOM OF THE CURB AND GUTTER <u>SHALL</u> BE CONSTRUCTED PARALLEL TO THE SLOPE OF SUBBASE COURSES AND TO THE DEPTH OF THE PAVEMENT.
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MODIFIED CG-6R COMBINATION 6" CURB & REVERSE GUTTER

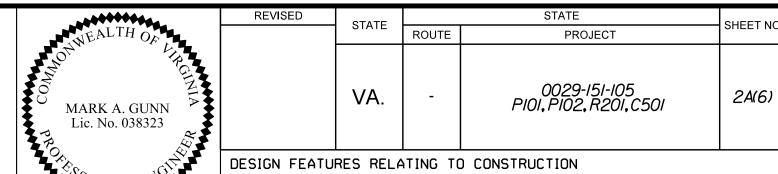
Parking Lot Pavement Design



PROJECT SHEET NO. 0029-151-105



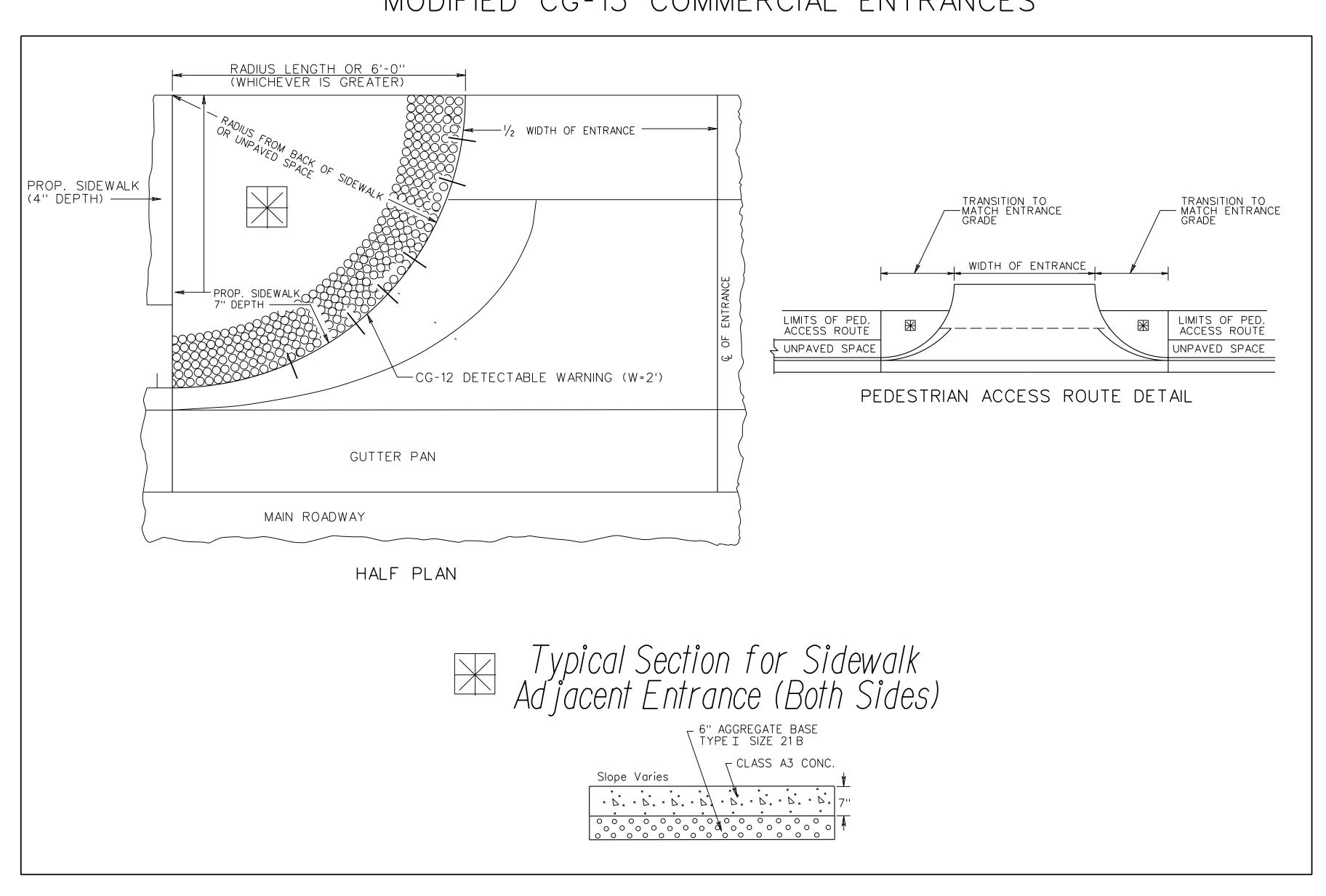
Typical Sections Cont.



OR TO REGULATION AND CONTROL OF TRAFFIC MAY BE SUBJECT TO CHANGE AS DEEMED NECESSARY BY THE DEPARTMENT

Rinker Design Associates, P.C. Manassas, Virginia PROFESSIONAL ENGINEER

ADDITIONAL DETAILS FOR ST'D CG-13 & MODIFIED CG-13 COMMERCIAL ENTRANCES



SHEET NO. 0029-151-105

PROJECT MANAGER<u>.*Wendy_Block_Sanford.City_of_Fairfax_(703)_385-7889_</u></u>*

DESIGNED BY _Adam_D.Welschenbach.P.E.,Rinker_Design_Assoc.,P.C.(703)_368-7373_

SURVEYED BY *Rinker Design Assoc., P.C.(703) 368-7373 (2011)*

SUBSURFACE UTILITY PROVIDED BY Accumark (2011)

6.0 GEOTECHNICAL RECOMMENDATIONS

6.1. CONCRETE BOX CULVERT

The subsurface conditions encountered in the soil test borings along the proposed culvert alignment indicate that the soils at the proposed culvert subgrade level should be able to provide a bearing capacity of 3,000 pounds per square foot (psf).

During construction, the culvert subgrade bearing capacity should be documented in the field by an authorized representative of the Geotechnical Engineer of Record to check that the in situ bearing capacity at the bottom of excavation is adequate for the design loads.

6.2. SLOPES

Based on our prior experience and the general subsurface conditions encountered at this site, we recommend that permanent slopes no steeper than 2H: 1V and temporary slopes no steeper than 1.5H: 1V be constructed for all cut and fill slopes on this project. For the construction of box culvert, a shoring system should be used in areas where there is not enough space to satisfy the slope requirements.

6.3. PAVEMENTS

The pavement design was performed in accordance with the Guidelines for 1993 AASHTO Pavement Design, which was published by VDOT and revised in July 2011. Detailed pavement design including the traffic load information and calculations are attached to this report in Appendix E. The traffic report prepared by VHB has been approved by VDOT. We have reviewed both the traffic data provided to us by VHB and the available VDOT historical traffic data, and the more conservative traffic data (VDOT historical traffic data) were used for the pavement design.

The design CBR value for the pavement design was derived from the fourteen CBR tests on the soil samples collected. The laboratory CBR values ranged from 1.2 to 6.4 with an average value of 3.6. The design CBR value was calculated as the mathematical average of the remaining laboratory CBR values multiplied by a factor of two-thirds. The design CBR value used for this project is 2.4.

We recommend the full depth pavement widening/reconstruction using the following sections:

Pavement Locations	Pavement Section
	Surface Course (SM-9.5D) = 1.5"
Rt. 29/50 (Fairfax Blvd)	Intermediate Course (IM-19.0A) = 2"
Inner Lane Widening	Base Course (BM-25.0A) = 9"
	Subbase Course (Cement Treated Aggregate) = 8"
Rt. 29/50 (Fairfax Blvd) Outer Lane Widening	Surface Course (SM-9.5D) = 1.5"
	Intermediate Course (IM-19.0A) = 2"
	Base Course (BM-25.0A) = 9"
	Subbase Course (21B) = 12"
Rt. 123 (Chain Bridge Rd) Inner Lane Widening	Surface Course (SM-9.5D) = 1.5"
	Intermediate Course (IM-19.0A) = 2"
	Base Course (BM-25.0A) = 10"
	Subbase Course (Cement Treated Aggregate) = 8"
	Surface Course (SM-9.5D) = 1.5"
Rt. 123 (Chain Bridge Rd)	Intermediate Course (IM-19.0A) = 2"
Outer Lane Widening	Base Course (BM-25.0A) = 10"
	Subbase Course (21B) = 12"

It should be noted that, based on the collected existing pavement data, the existing pavement is structurally deficient relative to the VDOT 30-year design standard. However, it is our understanding that, as an intersection improvement project, it is not intended to construct a pavement buildup to the existing pavement. Therefore, we recommend a 1.5-inch surface mill and inlay with 1.5 inches of Type SM-9.5D asphalt pavement surface course for the portions of existing pavement that are planned to be retained within the project limits, with the exception of the outer lane of eastbound US 29/50 from the beginning of construction to approximately 200 feet east of Farr Avenue. The existing asphalt pavement in this area shows severe distresses in the outermost portion (approximately 8 feet wide) of the outer lane, which is not underlain by concrete pavement. The distressed asphalt pavement in these areas should be demolished to the subgrade level and reconstructed in accordance with the full depth pavement recommendations.

The pavement widening/reconstruction should be performed in accordance with VDOT standard detail WP-2 for asphalt pavement widening. VDOT standard UD-4 edge drains should be

Geotechnical Recommendations

Note: See project's cross sections for details on unsuitable material locations, hard material locations, etc.

installed beneath the curb and gutter of all new pavements. The underdrain pipes should be either connected to existing underdrain or storm structures.

We recommend the following pavement section be used for temporary pavement during construction of the project. The temporary pavement section is designed for an 18 months life.

Pavement Location	Pavement Section
Temporary Pavement during Construction	Surface Course (SM-9.5D) = 2"
	Base Course (BM-25.0A) = 5"
	Subbase Course (21B) = 8"

Site specific traffic data for the parking lot along the north side of US 29/50 are not available for a detailed pavement design calculation. We recommend the following pavement section be used for the portion of parking lot that must be reconstructed due to the installation of the box

	Pavement Location	Pavement Section
	Parking Lot along the North Side of US 29/50	Surface Course (SM-9.5D) = 1.5"
		Intermediate Course (IM-19.0A) = 2"
		Base Course (BM-25.0A) = 3"
		Subbase Course (21B) = 8"

Northfax Intersection and Drainage Improvements VDOT Project No. 0029-151-105; DMY Project No. 1252.01 April 6, 2012 (Revised September 11, 2013 and September 5, 2014)

7.0 CONSTRUCTION RECOMMENDATIONS

7.1. SITE PREPARATION

Site preparation should consist of any proposed demolition, removing existing underground utilities, existing structures, topsoil and vegetation, and any other soft or unsuitable material from the proposed construction areas. Utilities such as pipes should be removed entirely or abandoned by filling the pipe with grout to prevent future migration of soils into the pipe. Voids resulting from the removal of tree stumps should be filled with compacted structural fill. Disposal of demolition debris should be performed in accordance with local, state and federal regulations. Additional requirements included in Section 301 – Clearing and Grubbing of the VDOT 2007 Road and Bridge Specifications should be followed.

7.2. EARTHWORK CONSTRUCTION

The earthwork for the proposed roadway construction should be performed in accordance with Section 303 - Earthwork of the VDOT 2007 Road and Bridge Specifications. Additional site specific recommendations are discussed in the following paragraphs.

Subgrade Preparation

Following the site preparation and any required excavation, the newly exposed roadway subgrade should be evaluated by an authorized representative of the Geotechnical Engineer of Record. During this evaluation, we recommend that all subgrade areas be proof-rolled using a fully loaded tandem axle dump truck (20-ton minimum) or similar rubber-tired vehicle. The proofrolling should be performed in such a pattern that the entire subgrade areas are loaded with at least one pass. Areas that are not accessible to proofrolling may be evaluated using a steel probe rod or other suitable devices.

If the subgrade exhibits excessive deflections or pumping when proof-rolled or soft subgrade is detected by probing, an appropriate remedial measure would be recommended by the Geotechnical Engineer of Record at that time. Potential problem subgrade areas as identified by this soil investigation and the recommended remedial measures are detailed in the following paragraphs. The stabilized subgrade areas should be again evaluated and approved by the Geotechnical Engineer of Record prior to fill placement or pavement installation.

The average optimum moisture content from the Proctor tests of soil samples collected from the proposed roadway widening areas is 14.9%. We have considered that soils with moisture content of greater than 17.9% (i.e., 120% of the average optimum moisture content) to be excessively moist. In general, excessive moisture may be more or less depending on the amount of recent precipitation at the time construction is performed. Along some parts of the project alignment, the soils at/near the proposed grade were found to have in-place moisture contents far above the corresponding optimum moisture contents. This condition has the potential to cause pumping problems during subgrade and base construction. We recommend that upon completion of any necessary excavation in these areas, 2 feet of subgrade soils from beneath the proposed pavement and shoulders be removed, and either dried out and replaced, or replaced with drier soils. The approximate locations where excessive in-place moisture contents will likely be encountered are:

Table 7-1: List of Areas with Excessively Moist Subgrade

Roadway Stations	Widening Locations
18+00 to 25+00 (US 29/50)	Right
64+75 to 67+00 (Rt. 123)	Left & Middle
67+50 to 69+50 (Rt. 123)	Left, Right & Middle
72+50 to 77+85 (Rt. 123)	Middle

The soils at the proposed grade were found to have insufficient load carrying capacity in some areas as indicated by the very low CBR values. We recommend that the top 2 feet of the in-situ subgrade soils in these areas be removed and replaced with VDOT Select Material Type I (minimum CBR 30). The removed soils may be used in the deeper part (i.e., 2 feet or more below the proposed subgrade) of the roadway embankment. The approximate locations of the low CBR subgrade soils are:

Table 7-2: List of Areas with Low CBR Subgrade

Roadway Stations	Widening Locations
16+00 to 18+00 (US 29/50)	Right

Soft/loose soils (SPT N-Values less than 5 bpf) were encountered at/near the proposed subgrade in some areas. We recommend that the soft/loose subgrade soils be densified in place if feasible. Otherwise, the soft/loose subgrade soils should be completely removed in these areas and replaced with VDOT Select Material Type I (minimum CBR 30). Alternatively, a minimum of 3 feet of the wet and soft/loose subgrade soils should be removed, a layer of VDOT approved woven subgrade stabilization geotextile be placed on the excavated surface, then fill the excavation with VDOT Select Material Type I (minimum CBR 30). The approximate locations where the soft/loose subgrade conditions will likely be encountered are:

Table 7-3: List of Areas with Soft/Loose Subgrade

Roadway Stations	Widening Locations
19+50 to 20+50 (US 29/50)	Right
23+50 to 25+00 (US 29/50)	Left

Engineered Fills and Placement

All engineered fills including roadway embankment and backfill around structures should have a Liquid Limit less than **45** and a Plasticity Index less than **20**. Additionally, any borrow material to be used within 3 feet of the pavement subgrade elevation should have a CBR value of 5.0 or greater. Before field operations begin, a representative sample of each proposed engineered fill should be collected and tested to determine its Atterberg Limits, gradation, maximum dry density, optimum moisture content, and natural moisture content. The test results will be used to evaluate the suitability of each proposed engineered fill for quality control purposes during fill

Based on the subsurface conditions observed in our exploration, the excavated onsite natural soils and clean fills can be re-used as engineered fill with the exception of the excavated soils from the proposed culvert at the following locations, which potentially have excessive organic

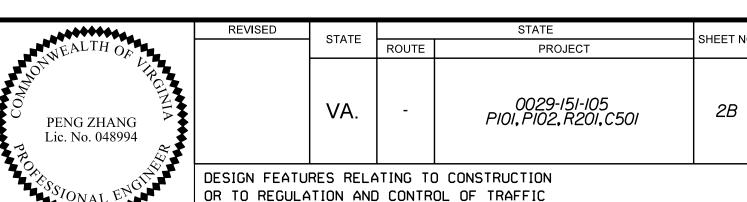
Table 7-4: List of Areas with Excessive Organic

Locations	Depth
18+00 \pm to 19+50 \pm (Box Culvert)	10± feet below surface
20+00 \pm to 22+70 \pm (Box Culvert)	10± feet below surface

Additionally, the soils excavated from the top 4 to 10 feet (average about 8 feet) of the proposed box culvert excavation are expected to have natural moisture contents greater than 120% of their respective optimum moisture contents. These excavated soils as well as the wet and soft/loose soils excavated from the roadway subgrade areas listed in the Subgrade Preparation section of this report are not suitable for direct use as structural fills. However, after moisture reconditioning within 20% of optimum moisture contents, they may be used as backfill around the culvert structure as well as roadway embankment.

Engineered fill materials should be placed in lifts not exceeding 8 inches in loose thickness for roadway embankments and not exceeding 6 inches in loose thickness for backfill around the box culvert structure. They should be moisture conditioned to within 20 percentage of the optimum moisture content. The engineered fill should be compacted to a minimum of 95% of the maximum dry density obtained in accordance with VTM-1, Standard Proctor Method. The top 1 foot of soil supporting pavements, sidewalks, or gutters should be compacted to a minimum of 100% of the maximum dry density in accordance with VTM-1, Standard Proctor Method.

All fill operations should be observed on a full-time basis by an authorized representative of the Geotechnical Engineer of Record to determine that compaction requirements are being met. All fill shall be periodically tested to confirm that compaction is being achieved. A sufficient number



DMY Engineering Consultants Inc. Dulles, Virginia GEOTECHNICAL ENGINEER

MAY BE SUBJECT TO CHANGE AS DEEMED

NECESSARY BY THE DEPARTMENT

per lift. The elevation and location of the tests should be clearly identified and recorded at the time of fill placement.

of tests shall be taken in each lift before the next lift is placed, on the order of at least three tests

7.3. CULVERT CONSTRUCTION

All culverts should be constructed in accordance with Section 302 – Drainage Structures of the VDOT 2007 Road and Bridge Specifications and the standard detail PB-1 of the VDOT 2008 Road and Bridge Standards. Additional site specific recommendations are provided in the following paragraphs.

<u>Bedding</u>

The stabilized groundwater table along the proposed box culvert alignment was above the proposed box culvert subgrade. After any dewatering measures, the groundwater table is anticipated to stay relatively close to the box culvert subgrade. To provide a reasonably dry and stable working platform, we recommend that the bedding material for the proposed box culvert consist of 6 inches of No. 57 aggregate, wrapped in a woven subgrade stabilization geotextile, capped with 4 inches of No. 25 or No. 26 aggregate. This bedding requirement also applies to other large diameter (≥ 36") pipe culverts on the project.

Excavation Support

All foundation excavations should be sloped or stepped back in accordance with Occupational Safety and Health Administration (OSHA) regulations for excavations. Loose/soft soil condition was encountered within the culvert excavation depth along some parts of the proposed box culvert alignment. Shoring, bracing or flattened slopes will likely be required to maintain a safe and stable work area during the excavation and construction of the proposed box culvert structure. These areas are anticipated to be approximately between Station 18+50± and 29+00± along the proposed culvert alignment.

<u>Difficult Excavation</u>

No rock or auger refusal material was encountered. However, subsoil with SPT N-values greater than 50 bpf was encountered within the anticipated excavation depth in some parts of the culvert alignment. Difficult excavation involving heavy construction equipment and possibly loosening the hard material using jack hammer in isolated areas should be anticipated in the following areas:

Table 7-5: List of Potential Difficult Excavation Areas

Locations	Depth to Hard Materials
12+00± to 13+50± (Box Culvert)	15± to 5± feet below surface
13+50± to 14+50± (Box Culvert)	5± to 12± feet below surface
14+50± to 16+00± (Box Culvert)	12± to 10± feet below surface
16+00± to 18+00± (Box Culvert)	10± to 15± feet below surface
19+50± to 20+50± (Box Culvert)	13± below surface
23+00± to 25+50± (Box Culvert)	12± below surface

Construction Monitoring

Settlement monitoring during construction should be provided for the buildings and structures in close vicinity of the culvert excavation. The purpose of this program is to monitor in real time the potential ground settlement induced by construction dewatering and other construction activities. Depending on the construction techniques selected by the contractor, vibration monitoring may also be necessary for the buildings and structures in the close vicinity of the culvert excavation.

7.4. GROUNDWATER CONTROL

Groundwater was not encountered within the boring termination depth in the roadway borings. It is not anticipated that the roadway excavations will be impacted by groundwater.

Stabilized groundwater table along the proposed culvert ranged from approximate elevation of 324.8 to 340.5 feet (about 3.5 to 7 feet below existing ground surface), which was about 2 to 12 feet above the proposed culvert subgrade. Significant dewatering will be required during construction. The groundwater table should be maintained at least 2 feet below the foundation subgrade. We recommend that dewatering be specified as a performance requirement and the contractor should select and use suitable equipment and means to lower the groundwater table. Temporary dewatering during construction may be achieved by drainage ditches, pumping from sumps, and a well point system.

Extreme cautions should be exercised by the contractor to avoid over-dewatering, which could induce excessive ground settlement around the dewatered areas and cause damages to the buildings, structures and underground utilities in the close vicinity. A proper construction monitoring program should be designed and implemented by the contractor.

PROJECT	SHEET NO.
0029-151-105	2B

Design Associates, P.C.

PROJECT MANAGER<u>Wendy Block Sanford,City of Fairfax (703) 385-7889</u>_

DESIGNED BY _Adam_D.Welschenbach, P.E.Rinker_Design_Assoc..P.C.(703)_368-7373_

SURVEYED BY *Rinker Design Assoc., P.C. (703) 368-7373 (2011)*

SUBSURFACE UTILITY PROVIDED BY_Accumark_(2011)______

DESIGN FEATURES RELATING TO CONSTRUCTION
OR TO REGULATION AND CONTROL OF TRAFFIC
MAY BE SUBJECT TO CHANGE AS DEEMED
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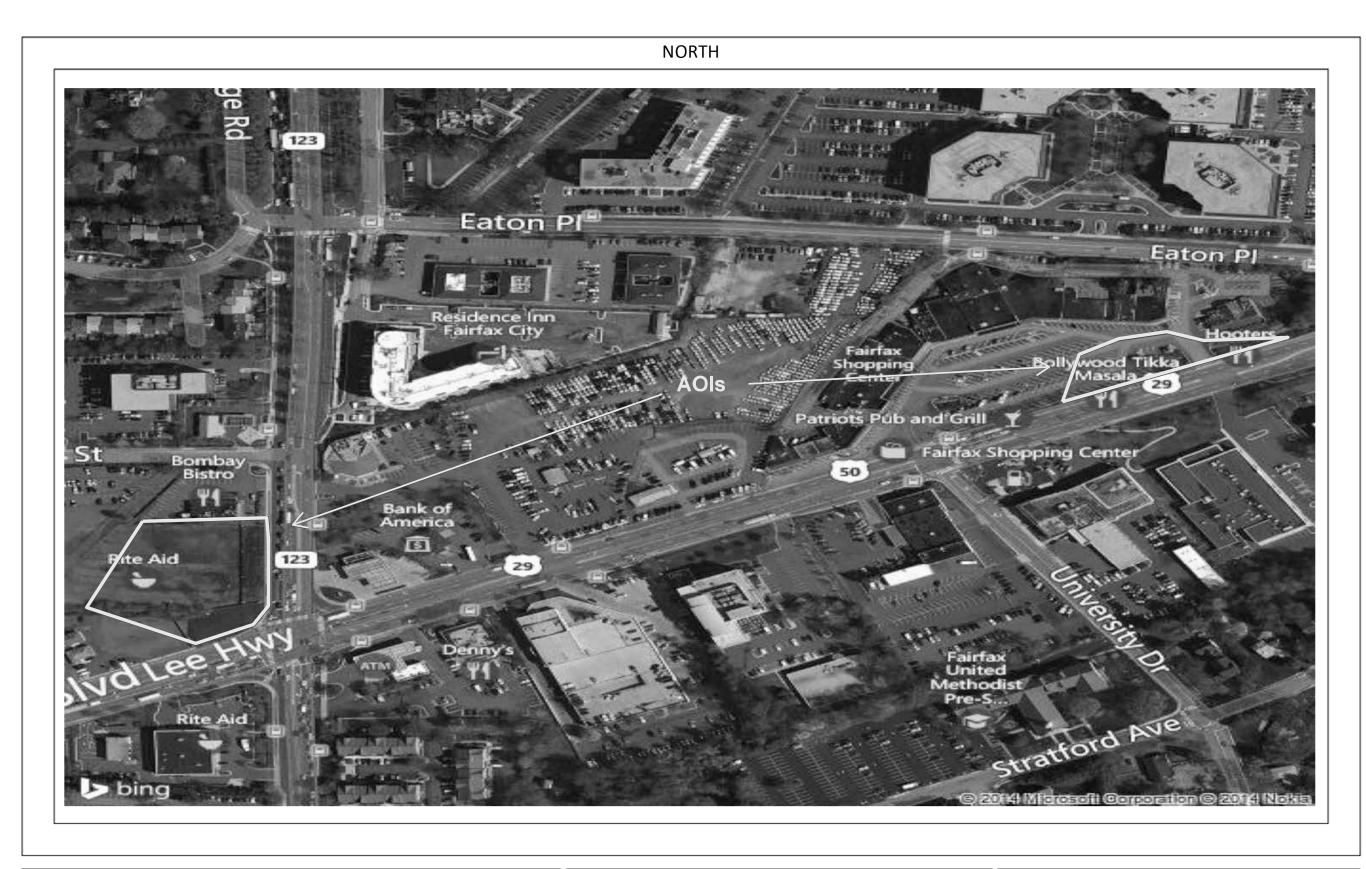
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Areas of Potential Contaminated Soils

The figure below is from the Soil and Groundwater Management Plan prepared by Meridian Environmental Co. and dated January 14,2015.

The Areas of Interest (AOIs) delineated in the figure are areas within the project limits that showed any anthropogenic impacts during investigation.

See the Contract Special Provisions for additional details on the steps for soil management required for this project.



Soil & Groundwater Management Plan
Aerial Map with Areas of Interest (AOI)
VDOT Project 0029-151-105

Fairfax Boulevard Drainage Improvements
Fairfax Boulevard (Rt. 29/50) & Chain Bridge Road (Rt. 123)
City of Fairfax, Virginia

Source: Bing Maps
Scale: NTS
Meridian Project No.: 14-001ESA

Drawn by: JBO

Checked by: JBO

Date: 12/30/2014

Figure 2

MERIDIAN ENVIRONMENTAL CO

a trusted partner in resolving environmental, geologic & hydrogeologic challenges